

**ADDENDUM #1**

FROM : ARCHITECTURICA  
555 W. BENJAMIN HOLT DRIVE  
SUITE 423  
STOCKTON, CA 95207  
(209) 952-5850 FAX (209) 952-2442

PROJECT : CODESTACK ACADEMY

LOCATION : 201 North California Street  
Stockton CA 95202

REF. # : ARCH PROJ. No. 2023-04

OWNER : San Joaquin County of Education  
2922 Transworld Drive  
Stockton, CA 95206

DATE : 7 November 2024

**NOTICE TO ALL BIDDERS**

**IT IS THE PURPOSE AND INTENT OF THIS ADDENDUM TO MODIFY AND/OR CLARIFY THE DRAWINGS AND SPECIFICATIONS FOR THIS PROJECT AND THIS ADDENDUM SHALL BECOME A PART OF THE CONTRACT DOCUMENTS. THESE CHANGES AND/OR INTERPRETATIONS SHALL BE INCORPORATED INTO YOUR BID.**

**REFER TO PROJECT PLANS AND SPECIFICATIONS PREPARED BY ARCHITECTURICA, COVER SHEET DATED 4 November 2024**

**STRUCTURAL**

ITEM 1 NEW STRUCTURAL DOCUMENT SET  
ALL PAGES REPLACED.

Attachments: Codestack Structural 2024 11-06(Bid Set)

END OF ADDENDUM #1

ARCHITECTURICA

By



Tim Dearborn, AIA  
Architect





## General

- Interpretation of drawings & specifications
  - For convenience, specifications have been prepared for this project and are arranged in several sections, but such separation shall not be considered as the limits of the work required by any separate trade. The terms and conditions of such limitations are wholly between the contractor and his subcontractors.
  - In general, the working details will indicate dimensions, positions and kind of construction, and the specifications will indicate qualities and methods. Any work indicated on the working details mentioned but not in the specifications, or vice versa, shall be furnished as though fully set forth in both. Work not particularly detailed, marked, or specified, shall be the same as similar parts that are detailed, marked, or specified. If conflicts occur between drawings and specifications, the most expensive materials or methods will prevail.
  - Should an error appear in the working details or specifications or in work done by others affecting this work, the contractor shall notify the architect at once and in writing. If the Contractor proceeds with the work so affected without having given such written notice and without receiving the necessary approval, decision or instruction in writing from the owner, then he shall have no valid claim against the owner, for the cost of so proceeding and shall make good any resulting damage or defect. No verbal approval, decision, or instruction shall be valid or be the basis for any claim against the owner, its officers, employees or agents. The foregoing includes typical errors in the specifications or notational errors in the working details where the interpretation is doubtful or where the error is sufficiently apparent as to place a reasonably prudent contractor on notice that, should he elect to proceed, he is doing so at his own risk.
- Construction shall conform to all applicable codes and regulations.
- Shop Drawing Note:
  - Shop drawings shall be submitted in the form of one reproducible and two copies of each sheet.
  - The purpose of shop drawing submittals by the Contractor is to demonstrate to the Structural Engineer that he understands the design concept by indicating which materials he intends to furnish and install, and by detailing the fabrication and installation methods he intends to use.
  - Prior to fabrication, shop drawings shall be submitted for review to the Structural Engineer. Shop drawing submittals shall include, but are not necessarily limited to structural steel, reinforcing steel, girded laminated beams, and pre-fabricated wood roof framing items such as I-joists and trusses.
  - Prior to submission the contractor shall review all submittals for conformance with the contract documents and shall stamp submittals as being "Reviewed for Conformance".
  - Shop drawing submittals processed by the Structural Engineer are not change orders.
  - Any detail on the shop drawing that deviates from the contract documents shall clearly be marked with the note "This is a Change".
  - Shop drawings or calculations submitted for review that require resubmission for review shall be billed hourly for such time to the General Contractor. Re-review will not proceed without written approval from the General Contractor for additional engineering review services.
- Safety Note:
  - It is the Contractor's responsibility to comply with the pertinent sections, as they apply to this project, of the "Construction Safety Orders" issued by the State of California latest edition, and all OSHA requirements.
  - The contractor and Structural Engineer do not accept any responsibility for the Contractor's failure to comply with these requirements.
  - The Contractor shall be responsible for adequate design and construction of all forms and shoring required.

- The Contractor shall notify the Architect and Structural Engineer where a conflict or a discrepancy occurs between the structural drawings and any other portion of the contract documents or existing field conditions. Such notification shall be given in due time to allow the contractor to correct the construction schedule. In case of a conflict between structural drawings and specifications, the more restrictive condition shall take precedence unless written approval has been given for the least restrictive. Contractor shall verify all dimensions with architectural and structural drawings prior to commencing any work.
- Where no specific detail is shown, the construction shall be identical or similar to that indicated for like cases of construction on this project. Should there be any question, contact the Architect and Structural Engineer prior to proceeding.
- When construction attaches to an existing building, a complete set of drawings of the existing building shall be kept on the job site. Contractor to obtain these drawings from the owner.
- Contractor shall provide an allowance equal to 2% of the bid for structural steel, misc. iron, light gauge framing, and reinforcing steel to be used at the discretion of the structural engineer. Unused amount to revert to the owner upon completion of the job.
- Any substitutions for structural members, hardware, or details shall be reviewed by the Architect and Structural Engineer. Such review will be billed on a time and materials basis to the General Contractor with no guarantee that the substitution will be allowed.
- Do not scale drawings. Contact the Architect or Structural Engineer for any dimensions not shown.
- These drawings are not complete until reviewed and accepted by the local building official and signed by the owner and the Structural Engineer.
- All drawings and written material appearing herein constitutes the original and unpublished work of the Structural Engineer and the same may not be duplicated, used or disclosed without written consent of the Structural Engineer.
- The structure shown on these drawings is structurally sound only in its completed form. The stability of this structure depends on the diaphragms and the bracing members shown. The Contractor is to provide for the design and construction of shoring for all earth, forms, concrete, steel, wood, and masonry to resist gravity, earth, wind, seismic, and construction loads. Shoring shall remain in place until all diaphragms and lateral resisting elements are in place in their entirety. Construction materials shall be spread out if placed on framed floors or roofs. Load shall not exceed the design live load per square foot.

## Lightgauge Metal Framing

- All metal framing shall be formed from corrosion resistant steel conforming to ASTM A653 or ASTM A101 with minimum yield strength of 33 ksi for 18 ga and lighter and 50 ksi for 16 ga and heavier.
- Metal framing shown on the structural drawings shall have channel type sections with stiffened flanges having the minimum properties as shown in the light gauge metal framing schedule.
- Metal tracks shall be the same gauge as framing which it supports, unless noted otherwise, with minimum flange width of 1/4" and minimum properties as shown in the light gauge metal framing schedule.
- Galvanized coating must meet the ASTM G55 specification.
- Factory punch-outs to be located along the centerline of the webs of the members and have a minimum center-to-center spacing of 24". Punch-outs to have a maximum width=1/2", and a maximum length=4". Lightgauge framing members shall be cut such that the minimum distance between the end of the member and the near edge of the web punch-out=10".
- All header members shall be un-punched.
- Design properties of metal framing studs, channels & tracks shall conform to (or exceed) the Steel Stud Manufacturer's Association (SSMA) Product Technical Information catalog # ICC Report # ESR-3064P.

## Design Criteria

- Code: 2022 California Building Code (CBC)
- Design Live Loads:

Area	Live Load	Remarks
Roof		
A) Flat to < 4:12	Lr = 20 psf	Reducible per code
B) 4:12 to < 12:12	Lr = 12-20 psf	Reducible per code
Floor	L = 0 psf	Reducible per code
- Snow Design Parameters: N/A
- Wind Design Parameters:

Basic Design Wind Speed (3-sec gust)	V = 95 mph
Nominal Design Wind Speed (3-sec gust)	Vnom = 74 mph
Risk Category	II
Exposure Category	C
Internal Pressure Coefficient	+0.18
Analysis Method	Directional Procedure
- Wall Pressures for Components & Cladding:

Zone 4:	20.8 psf
Zone 5:	23.9 psf
Parapet:	59.4 psf
- Wind Uplift loads (zones defined per ASCE 7-16 fig. 30.3-2 thru 30.3-6):

Zone 1:	12 psf
Zone 2:	34 psf
Zone 3:	50 psf
- Discontinuity Distance: a = 7.2 ft
- Wall Pressures for Components & Cladding:

Zone 4:	20.8 psf
Zone 5:	23.9 psf
Parapet:	59.4 psf
- Earthquake Design Parameters:

5.1 Seismic Importance Factor	Ia = 1.0
5.2 Risk Category	II
5.3 Soil Site Classification	D'
5.4 Seismic Design Category	D'
5.5 Mapped Spectral Response Accel	
A) Short period	Sa = 0.725g
B) 1-sec period	S1 = 0.283g
5.6 Design Spectral Response Accel	
A) Short Period	Sas = 0.590g
B) 1-sec period	S1s = 0.516g
5.7 Seismic Force Resisting System	Steel Special Concentric Braced Frames, Semi Rigid Diaphragm at roof, Rigid Diaphragm at floors.
5.8 Seismic Base Shear	V = xxx kips
5.9 Seismic Response Coefficient	Cs = 0.048g
5.10 Response Modification Factor	R = 6.0
5.11 Component Amplification Factor	
A) Condenser	ap = 2.5
B) Generator	ap = 1.0
5.12 Component Response Modification Factor	
A) Condenser	Rp = 6.0
B) Generator	Rp = 2.5
5.13 Analysis Procedure	Equivalent Lateral Force

## Foundations

- Foundation design is based on the Geotechnical Engineering Report, MFE No. 06357-01 by Mid Pacific Engineering, Inc, dated April 18, 2024.
- All building pad preparation and foundation work shall be done in accordance with the requirements of the geotechnical report. Copies of the report may be obtained from the engineer upon request.
- The geotechnical engineer shall observe all footing excavations prior to placement of reinforcing steel and concrete.
- Foundation depths indicated on plans are for estimating purposes only. Actual depths are to be determined by the geotechnical engineer on the jobsite.
- When structural observation is required, structural engineer shall observe footing reinforcing steel prior to concrete placement. Provide 48 hours notice to structural engineer prior to concrete placement.
- The contractor shall be solely responsible for all excavation procedures including, but not limited to, lagging, shoring and protection of adjacent property, structures, streets, and utilities in accordance with the local building department.
- Foundation type: mat slab supported by helical anchors (designed by others) coefficient of friction = 0.20 passive lateral earth pressure = 150 pcf New basement retaining walls shall be designed to resist "at-rest" lateral earth pressures equal to an equivalent fluid pressure of 75 psf per foot of wall backfill for conditions of walls fixed at the top.

## Metal Deck

- Provide metal decking manufactured by Verco, ASC Profiles, or approved equivalent of type and gauge as shown on plans. Decking shall have the following minimum effective section properties u.n.o.:

Location	Type	Gauge	I (in <sup>4</sup> /ft)	I (in <sup>4</sup> /ft)	Sx (in <sup>3</sup> /ft)	Sy (in <sup>3</sup> /ft)
Roof	35D	18	2.415	2.272	0.480	1.070
Floor	35D-FormLok	16	3.133	2.968	1.317	1.377
Stair Landing	B-36	16	0.381	0.381	0.349	0.410
- Metal floor deck shall be composite type, conforming to ASTM A-653, Grade 40 min, or equal and shall have a galvanized, G-60 grade coating. If contractor chooses to substitute decking by alternative manufacturer, contractor shall submit complete supporting data showing comparison of structural properties, gravity & shear values to that of the specified deck.
- Metal roof deck shall conform to ASTM A-653, Grade 40 min, or equal and shall have a galvanized, G-60 grade coating.
- Prior to fabrication, the Contractor shall submit shop drawings and ICC/AFMO evaluation report for product used, to the Architect and Structural Engineer for review. Shop drawings should indicate deck gauge, size, and layout as well as closure conditions, welds to supports and side lap details.
- Connection and welding of decking to structural supports and at deck side seams shall be as specified in the structural drawings.
- No conduit or non-structural items may be placed in concrete fill over metal deck.
- When placing concrete over metal deck, concrete should first be placed over beams and girders rather than at mid-span of the metal deck and concrete should not be piled beyond the finished depth of the slab.
- All metal deck receiving concrete fill shall have factory-punched vent tabs to provide 1% ventilation typical.

## Elevator Guide Rails & Supports

- Guide rails, guide rail supports and brackets for elevator cab or counterweight shall be designed to meet the force and displacement requirements of ASCE 7-16 Section 13.6.11.
- Provide Shop Drawings and engineering calculations for the guide rails, guide rails supports and supporting brackets signed by a Civil Engineer, registered in the state which the projects located, for review by the Architect and/or Structural Engineer, and for approval by the enforcement agency prior to fabrication.
- All loading conditions resulting in eccentricities or torsion to beams and/or columns shall be resolved by the installation of stiffeners and diagonal struts designed and installed by the supplier.
- Seismic forces shall be accounted for and braced back to the structure. For additional requirements see California code of Regulations Title 24 part 2 and 7.
- See Architectural drawings for opening/pit dimensions, locations and details.

## Concrete

- Structural concrete shall attain 28-day compressive strength as required in note #30. Maximum slump shall not exceed 4".
- Concrete mix designs shall be prepared by a registered Civil Engineer, reviewed by Owner's testing laboratory and submitted to the Structural Engineer for review. Selection of concrete mix proportions shall be per ACI 318-19 Section 26.4.3, & 26.4.4.
- Cementitious materials:
  - Cement shall conform to ASTM C-150 type I or II.
  - Fly ash shall conform to ASTM C-618. Max quantity of fly ash shall be as given in specs (5% max u.n.o.)
- Concrete aggregates shall conform to ASTM C-33 for normal weight concrete and ASTM C-330 for light weight concrete.
- Water shall be clean and free from injurious amounts of oils, acids, alkalis, salts, organic materials or other substances deleterious to concrete or reinforcement.
- Non-shrink grout or groutack shall consist of a premixed nonmetallic formula. See note #27 for additional information.
- Reinforcing steel shall conform to ASTM A615-grade 60 for #4 and larger, and ASTM A615-grade 40 for #3 and smaller, except reinforcing steel to be welded shall conform to ASTM A706. Contractor shall submit rebar mill certificates.
- Welded wire fabric shall conform to ASTM A-1064.
- All preheating and welding of reinforcing bars shall be done in accordance with AWS D1.4 latest edition and shall be continuously inspected by a qualified laboratory. Contractor shall furnish I/P5 for all rebar welding to the laboratory. Reinforcing steel shall be fabricated according to "Manual of Standard Practice for Reinforced Concrete Construction".
- Dimensions shown for location of reinforcing are to the face of bars listed and denote clear coverage. Non-prestressed, cast-in-place concrete coverage shall be as follows, u.n.o.:

Cast against earth (except slabs)	3"
Cast in forms and exposed to earth or weather	
#6 & larger	2"
#5 & smaller	1 1/2"
Beams & columns (ties)	1 1/2"
Beams & columns (main reinf)	2"
Cast-in-place walls	
(exterior face & soil side)	see above
Cast-in-place walls	
(interior face - #11 & smaller)	3/4"
Tilt-up walls	see details
Slabs (on forms)	3/4"
Slabs (on ground)	2" c/c from top u.n.o.
- Splices in continuous reinforcement shall be lapped u.n.o., lap bars per note 31 u.n.o., splices in adjacent bars shall be greater than 5'-0" apart. Splice continuous bars in soil-bearing grade beams, structural slabs on grade and mat foundations as follows u.n.o.: lap bars at centerline of support; bottom bars at mid-span. Splice continuous bars in elevated slabs and beams, etc., as follows u.n.o.: top bars at mid-span; bottom bars at centerline of support. All bars size #14 and larger shall be continuous for full length shown or spliced with mechanical couplers as noted in details. Splices in WWF shall overlap 2 squares minimum.
- The minimum clear spacing between parallel bars in a layer shall not be less than the larger of bar diameter, 1", or 33% greater than the maximum aggregate size (nominal), whichever is greatest. This requirement also applies to the clear spacing between different layers of parallel bars and to the clear distance between a contact lap splice and adjacent splices or bars.
- All hooks shall be standard hooks unless otherwise shown or noted. At walls, provide hooks at ends of all reinforcing ends, corners and intersections, u.n.o. Provide construction/control joints @ all slabs on grade as noted on plan. Proposed joint plan shall be submitted to the Structural Engineer for approval prior to construction. Concrete surface at construction joints shall be thoroughly cleaned and laitance removed. Where indicated on drawings, roughen concrete surface to 1/4" amplitude. Concrete may be roughened by chipping the entire surface, sand blasting, or raking the surface to provide 1/4" deep deformations. Remove all debris from forms before casting dry concrete.
- Reinforcing, dowels, bolts, anchors, sleeves, etc., to be embedded in concrete shall be securely positioned in forms before placing concrete.
- Pipes and electrical conduits shall not be embedded in structural concrete or concrete fill over metal decking except where specifically approved by the Structural Engineer.
- Anchor bolts (AB's) cast in concrete or masonry for wall sill and ledge/ applications shall be headed bolts with cut threads conforming to ASTM A307 or F1554 u.n.o. Refer to "Wood notes" for additional requirements for bolts in contact with pressure treated or fire retardant material. Refer to "Structural steel" note for requirements for anchor rods cast in concrete for column base plate and steel embed applications.
- Walls shall be cast in horizontal layers of 2'-0" maximum depth.
- Concrete in walls, piers or columns shall set at least 2 hours before placing concrete in beams, spandrels, or slabs supported thereon.
- Consolidate concrete placed in forms by mechanical vibrating equipment supplemented by hand-spading, rodding or tamping. Use equipment and procedures for consolidation of concrete in accordance with the recommended practices of ACI 304 to suit the type of concrete and project conditions. Concrete shall not be dropped through reinforcing steel (as in walls) so as to cause segregation of aggregates. In such cases hoppers and chutes or trunks of variable lengths shall be used so that the free unconfined fall of concrete shall not exceed 6 feet.
- Drill through steel columns, beams and plates to pass continuous reinforcing, u.n.o.
- No wood spreaders allowed. No wood stakes allowed in areas to be concreted.
- Additional reinforcing in precast or tilt-up panels required for lifting stresses shall be supplied by contractor.
- Provide #5x4'-0" diagonal reinforcing at mid-depth of slab at all re-entrant corners typical. This applies to slab on grade, concrete over metal deck, and elevated structural slab conditions.
- Place non-shrink grout under base plates, sill plates, etc as indicated on the drawings. Non-shrink grout shall be Masterflex 420 Grout by Master Builders Technologies or approved equal with a minimum f'c of 7500 psi @ 28 days.
- All saw cutting shall be done after initial set has occurred to avoid tearing or damage by the saw blade, but before initial shrinkage has occurred.
- Notify Structural Engineer a minimum of 48 hours before placing any concrete.
- Concrete strength: (max slump = 4")

Use	f'c @ 28 days	Max Aggregate Size	Density (lbs/ft <sup>3</sup> )	Max W/C Ratio
Foundations	3000 psi	1/2"	145	0.58
Slab-on-grade	3500 psi	1"	145	0.45
Concrete fill o/ metal deck	3500 psi	3/8"	145	0.52
Exterior flatwork	2500 psi	1"	145	0.60

- Development lengths shall be provided per the table below unless noted otherwise.

Bar	Straight Bars		With Standard Hooks		
	f'c		f'c		
	3000 psi	4000 psi	3000 psi	4000 psi	
#3	15"	21"	#3	6"	6"
#4	24"	25"	#4	11"	10"
#5	36"	31"	#5	14"	12"
#6	43"	37"	#6	17"	15"
#7	63"	54"	#7	20"	17"
#8	72"	62"	#8	22"	19"
#9	80"	70"	#9	25"	22"

## Test and Inspections

- Tests and Inspections shall be provided as indicated below and shall conform to the requirements of the 2022 CBC, Chapter 17.
- All Test and Inspections shall be performed by a certified special inspector from an established Testing & Inspection Company, unless noted otherwise. Jobsite visits by the Structural Engineer do not constitute inspections and are not a substitute for special inspection.
- The special inspector shall observe the work indicated for conformance with the approved construction documents.
- The special inspector shall furnish inspection reports to the building department, the engineer or architect of record, and other designated persons. All discrepancies shall be brought to the immediate attention of the contractor for correction; then, if uncorrected, to the proper design authority and to the building department.
- The special inspector shall submit a final signed report stating whether the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved construction documents and the applicable workmanship provisions of the 2022 CBC.
- It is the contractor's sole responsibility to see that these tests and inspections are performed.
- Required Tests and Inspections are indicated below with a solid filled rectangle "■".
- Continuous notation indicates the full-time observation of work requiring special inspection by an approved special inspector who is present at the work area. Periodic notation indicates the intermittent observation of work.

- Note: Coordinate with building department, Test & Inspection firm.
- A. Compact fill
  - B. Concrete mix design, cement, aggregates & admixtures
  - C. Concrete strength f'c test
  - D. Reinforcing steel mill certification
  - E. Structural steel mill certification
  - F. Structural steel, cold formed steel, and anchor bolt sampling & testing (if not properly identified)
  - G. Masonry strength f'm
  - H. Masonry mortar, grout proportion, aggregates, additives
  - I. Post installed anchors: Expansion / Epoxy Anchors
  - J. High strength bolts, nuts and washers
  - K. End-welded studs
  - L. Beam to column moment connection
  - M. Veneer bond strength test
  - N. Shotcrete preconstruction test
  - O. Shotcrete strength & core test
  - P. Prefabricated items

	Verification and Inspection	Continuous	Periodic
A. STEEL			
1. Material verification of high-strength bolts, nuts & washers	○		●
2. Inspection of high-strength bolting, bearing & typical connections	○		●
3. Inspection of Welding Structural Steel: (field/shop) Complete & partial penetration groove welds	●		○
1. Multi-pass fillet welds	●		○
1. Single-pass fillet welds > 3/8"	●		○
1. Single-pass fillet welds < 3/8"	○		●
1. Floor and roof deck welds	○		●
4. Inspection of Steel Frame Joint Details for Compliance with Approved Construction Documents	○		●
5. Automatic end-weld stud shear connectors	○		●
B. CONCRETE			
1. Concrete Placement	●		○
2. Inspection of reinforcing steel & ledger/	○		●
3. Inspection of anchors cast in concrete	○		●
4. Precast concrete attachments & inserts	○		●
5. Erection of precast concrete members	○		●
C. WOOD			
1. Verify grade and thickness of sheathing	○		●
2. Verify nominal size of framing members at adjoining panel edges	○		●
3. Verify nail diameter and length, number of fastener lines, spacing between fasteners in each line and at edge margins	○		●
4. Verify positive connection of wood members supporting balcony or deck connections to exterior walls prior to concealment	○		●
D. MASONRY PLACEMENT & GROUTING	○		●
Note: refer to TMS 602-16 Tables 3 & 4			
1. Level 2 masonry inspection (Risk Categories I, II, III)	○		●
2. Level 3 masonry inspection (Category IV, DSA, OSHPD)	○		●
E. SOIL (by Geotechnical Engineer)			
1. Footing excavation	○		●
2. Pile/Pier foundation	○		●
3. Material verification below footing	○		●
4. Excavation verification to proper depth	○		●
5. Placement and compaction of controlled fill	○		●
6. Site preparation prior to placement of controlled fill	○		●
F. POST-INSTALLED ANCHORS			
1. Expansion anchor installation	○		●
2. Epoxy anchor installation	○		●

## Demolition

- Safety Notes:
  - It is the Contractor's responsibility to comply with the pertinent sections, as they apply to this project, of the "Construction Safety Orders" issued by the State of California, latest edition, and all OSHA requirements.
  - The Structural Engineer and Owner do not accept any responsibility for the Contractor's failure to comply with these requirements.
- Shore or brace trusses, beams, columns, and walls as required to maintain the stable integrity of the existing structure prior to demolition. It is the Contractor's sole responsibility to design and provide competent shoring and bracing for all loads imposed during and after demolition through completion of new construction.
- All dimensions given to and of the existing structure are approximate. Verify by field measurements and drawings the dimensions of the existing structure. Where actual conditions deviate from the details shown on the drawings, notify the Structural Engineer for instructions prior to proceeding with work.
- Demolition and removal of existing construction shall be made in such a manner as to avoid or minimize damage to adjacent construction.
- Extent of demolition is to be as indicated on plans, sections and details. Demolition is to include removal and disposal construction.

## Structural Observation

- This structure requires "Structural Observation" per 2022 CBC section 1709. The Architect or Engineer of record responsible for the structural design shall perform the structural observation. Observed deficiencies shall be reported in writing. In addition, final written documentation shall be issued stating that the necessary site visits have been made and identifying any reported deficiencies that, to the best of the structural observer's knowledge, have not been resolved.
- At a minimum, structural observations are required at the following stages of construction:
  - Prior to concrete placement of first footing pour when all reinforcing steel is in place.
  - When 75% percent of the rough framing is in place.
  - When 100% of the roof framing is in place.
- It is the Contractor's responsibility to notify the Architect and Structural Engineer at least 48 hours in advance of these Structural Observations.
- Structural Observations do not constitute special inspections of any type.



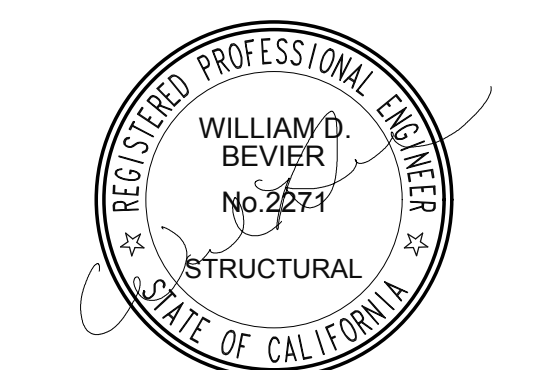
555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net

www.architechnica.net

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2479 Sunrise Blvd.  
Gold River, CA 95670  
Tel: (916) 631-3030  
Fax: (916) 631-8996  
Web: www.bevier.net  
Bevier Job No. 23011



Electronically Signed On: 11/6/24

CONSULTANT



CODESTACK  
ACADEMY

201 N CALIFORNIA ST.  
STOCKTON, CA 95202

SAN JOAQUIN COUNTY  
OFFICE OF EDUCATION

REVISIONS
Bid Set 11/06/2024

PROJECT NO: 2023-04  
ISSUE SET: SUBMITTAL SET  
ISSUE DATE: 09.05.24  
DRAWN BY: TB, JRW, MC

## GENERAL NOTES

S1.1



## Structural Steel

- Fabrication, erection and materials shall conform to the specifications and standards of the AISC, as contained in the "AISC 360-16 Specifications of Structural Steel Buildings" & the "AISC Manual of Steel Construction", 15th edition and California Building Code latest edition.
- Structural steel shall conform to the following specifications, u.n.o.:

Shapes	
Wide Flanges (W, WT)	ASTM A992
Wide Flanges (S, M, Angles (L)	ASTM A572
Channels (C), Misc Channels (MC)	ASTM A36 (B'), ASTM A992 (D')
Hollow Structural Steel (HSS)	ASTM A500, Gr. C (F <sub>y</sub> = 50 ksi)
Steel Circular Pipes (P)	ASTM A53, Type E or S, Gr. B
Plates & Bars	
Column Base Plates	ASTM A36
Brace Gusset Plates	ASTM A36
Beam Shear Connection Plates	ASTM A36
Column Continuity Plates	ASTM A572, Gr. 50
Beam Stiffener Plates	ASTM A36
Deck Closure Plates	ASTM A36
Stainless Steel Plates & Bars	ASTM A276
Other	ASTM A36
Nuts, Bolts, Rods, & Washers	
General Bolts	ASTM F3125, Gr A325-N
Slip Critical Bolts (see note #4 below)	ASTM F3125, Gr A325-SC
High Strength Bolts	ASTM F3125, Gr A325-N or Gr A490
Machine Bolts (General use)	ASTM A307
Bent & Headed Anchor Bolts	ASTM F1554, Gr. 36, 55, or 105
Partial & Fully Threaded Anchor Rods	ASTM F1554, Gr. 36, 55, or 105
Fully Threaded Rod (general use)	ASTM A36 (A307 Gr. A for 3/8"φ)
Welded Shear Connectors	ASTM A108, Gr. 1015 thru 1020
Welded Threaded Studs	ASTM A108, Gr. 1015 thru 1020
Nuts for Bolts & Machine Bolts	ASTM A563
Hardened Washers	ASTM F436
Unhardened Washers	ASTM F844
Plain Washers	ASTM B18, 22.1
Beveled Washers	ASTM B18, 23.1

- Butter connections shall consist of unfinished bolts per the table above unless noted otherwise. Anchor bolts cast in concrete or masonry shall be headed bolts with cut thread, full diameter body style conforming to ASTM F1554 u.n.o. Unless noted otherwise, anchor bolts/rods shall be grade 36 except that welded anchor bolts shall be grade 55 per 51 Supplementary requirements. All bolted connections and base plates shall have standard cut washers unless noted otherwise. Washers at base plates shall be placed at top and bottom of plate.
- "Slip-critical" bolted connections:
  - "Slip-critical" connections (A325-SC design values with special inspection) are required at all braced frame connections, at all connections along chord lines and drag lines (as noted on plans), and u.n.o., at all bolts in oversized or slotted holes.
  - The special Inspector must be present during installation and tightening operation of "slip-critical" connections.
- All structural steel shall receive minimum of one shop coat of red primer with a minimum dry film thickness of 2.0 mils. Do not shop prime or paint areas to be field welded, fireproofed, galvanized, to receive slip-critical high strength bolts, or to be embedded in concrete. Prior to priming or painting, clean structural steel in accordance with Steel Structures Painting Council (SSPC) recommendations & as required by the primer & paint manufacturer. Provide additional painting as noted in the specifications.
- All structural steel shall be erected plumb and true to line. Temporary bracing shall be installed and shall be left in place until other means are provided to adequately brace the structure. Contractor responsible for reviewing all base plate and support conditions during erection and bracing as required. See AISC and OSHA requirements.
- Place non-shrink grout under all base plates before adding vertical load. See Concrete Notes for non-shrink grout requirements.
- Structural steel below grade shall have 3" minimum of concrete cover.
- Provide 1/2"φ stitch bolts and ring fills, space at not more than 24" cc for all double angle members.
- At wood to steel parallel contact, attach with 1/2"φ welded threaded studs at maximum 32"cc, & 6" from ends of wood member, typical unless noted otherwise.
- Holes for unfinished bolts shall be of the same nominal diameter of the bolt plus 1/16". Use standard AISC gage and pitch for bolts except as noted otherwise. Holes for anchor bolts embedded in concrete shall be of the same nominal bolt diameter plus 3/16" unless noted otherwise.
- Welding shall be done by the electric arc process in accordance with American Welding Society standards, using only certified welders. All groove welds shall have complete penetration unless noted otherwise. All exposed welds shall be ground smooth. All welding to be done using E70xx electrodes. In addition, welding of ASTM A572 grade 50 steel and ASTM A992 steel shall be done with electrodes capable of depositing weld metal with a maximum diffusible hydrogen content of 16ml/100g (H16). Weld lengths called for on plans are the net effective lengths required.
- Minimum fillet welds:
  - 3/16" @ t < 1/2"
  - 1/4" @ t < 3/4"
  - 3/8" @ t > 3/4"
- Welding Procedure Specifications (WPS) for shop and field pre-qualified field joints and weld joints qualified by test shall be prepared for review prior to fabrication. All welding procedures that meet these requirements of AWS D1.1 Sec. 5.1 shall be considered as pre-qualified. Qualification testing is required when the depth of a partial penetration or complete penetration weld is 2" or greater.
- Structural steel & fasteners that are permanently exposed to weather shall be either primed and painted or hot dipped galvanized in accordance with ASTM A123 & A153. Repair galvanizing after welding in accordance with ASTM A130.
- When structural steel connections will be exposed to view in the completed building, they shall be fabricated, erected & finished in compliance with Architecturally Exposed Structural Steel (AESS) guidelines & Section 10 of the AISC 303-22 "Code of Standard Practice for Steel Buildings and Bridges".

## Powder Activated Fasteners (Shot Pins) - Hilti

- These notes govern all conditions called out on the plans as "PAF" or "shot pins", unless noted otherwise.
- Installation, testing & inspection of all PAF's shall be in accordance with the applicable evaluation report, these plans, and any project specifications.
- PAF's specified in these notes shall be used for dry, interior applications only.
- All PAF's shall be manufactured by Hilti, Inc. Tulsa, Oklahoma in accordance with the ICC evaluation report referenced below.

Connected Material	Base Material	Base Material Thickness, 't'	Minimum Penetration into Base Material (w)	Hilti Fastener	Evaluation Report (Issue Date)
Metal Decking	Steel (all grades)	1/8" ≤ t' < 3/8"	Full	X-HSN24 (w)	ESR-2147 (11/2022)
		1/4" ≤ t' < 1/2"	Full	X-ENP-19 L15	
		1/2" & thicker	1/2"		
Cold Formed Steel (Light Gauge) & Non-Preservative Treated Wood	Steel (all grades)	3/16" ≤ t' < 1/2"	Full	X-U w/ PB washer	ESR-2264 (06/2022)
		1/2" & thicker	1/2"		
		Normal Weight Concrete (Including concrete fill over metal decking) (a)	3x penetration min		
2x Preservative Treated Wood	Concrete (a)	CMU (grouted) (b) (c)		X-CP T2	ESR-2319 (08/2021)
		4 1/2" min	1 3/8"		

### Table Footnotes:

- 3" minimum edge distance & 4" minimum spacing required. Installations in concrete over metal deck may be installed either from underneath through the metal deck or from above directly into the concrete. For fasteners into the bottom of metal deck, spacing parallel to the deck flutes shall be 51" minimum.
- 4" minimum edge distance, and no more than one fastener shall be located in any given cell.
- Fasteners installed in the face of CMU shall be installed 1" minimum away from vertical mortar joints. At bed joints, fasteners shall not be spaced closer than 8"cc and must be installed a minimum of 8" from the end of the wall.
- 1 1/2" minimum concrete edge distance required. Locate fastener 6" from ends of sill plates.
- Full penetration means the entire length of the tapered tip shall penetrate completely through the base material.

## Expansion Anchors-Concrete:

- Use Hilti Knik Bolt-T22 Expansion Anchors as manufactured by Hilti Inc., Tulsa Oklahoma. ICC-ES Report No. ESR-4266 reissued December 2021.
- Installation of anchors shall be in accordance with the manufacturer's recommendations, ICC-ES Report, and these notes.
- Special inspection is required in accordance with the 2022 CBC Sections 1705A.11.3 and 1910A.5. Special Inspector must verify product, expiration date, concrete type and strength, anchor diameter and steel grade, compliance of drill bit, hole diameter and location, cleanliness of hole and anchor, and anchor embedment.
- Each anchor type (loaded in either pullout or shear) shall be torque tested in accordance with CBC Section 1910A.5 to the appropriate test load shown in the table. If any anchor fails testing, all anchors of the same type not previously tested shall be tested until 20 consecutive anchors pass, then initial testing frequency may be resumed.
- When installing anchors in existing concrete do not cut or damage existing reinforcing bars. Locate existing reinforcing bars with pachometer or x-ray if required.
- The testing of the anchors shall be done by the Testing Laboratory and a report of the test results shall be submitted to the Building Dept. and Architect/Structural Engineer.
- Anchors installed up into the bottom of metal deck with concrete fill shall be installed in the center of the low flute of the decking. The decking shall have a minimum thickness of 20 gauge. The minimum depth of embedment above the top of the deck shall be 1 1/2". The effective depth of embedment is considered to be one-third of the metal deck height plus the depth of embedment above the top of the deck. There shall be a minimum concrete cover of 1" between the top surface of the concrete and the end of the bolt.

Normal Weight Concrete Anchors			
Hilti Knik Bolt-T22 Expansion Anchors			
Anchor Diameter	Embedment Effective/Nominal u.n.o.	Carbon Steel Anchors Installation Torque Torque Test Load (ft-lbs)	Stainless Steel Anchors Installation Torque Torque Test Load (ft-lbs)
3/8"	2" / 2 1/2"	30	30
1/2"	3 1/4" / 3 3/4"	50	40
3/4"	4" / 4 1/2"	40	60
3/4"	4 3/4" / 5 1/2"	110	125

## Automatic End Welded Studs

- Automatic end welded studs shall be Tru-Weld Shear Connector Studs or approved equivalent. Studs shall be manufactured of C-1010 through C-1020 cold rolled steel which conforms to ASTM specifications A-108 or A-24.
- The studs shall be automatically end welded in accordance with the manufacturer's recommendations in such a manner as to provide complete fusion between the end of the stud and the plate. There should be no porosity or evidence of lack of fusion between the welded end of the stud and the plate. The stud shall decrease in length during welding approximately 1/8" for 3/8" diameter studs and under, and 3/16" for studs over 3/8" diameter. Welding shall be done only by qualified welders approved by a qualified welding inspector.
- Inspection of all the shop and field welding operations for automatic end welded studs shall be made by a qualified welding inspector, in accordance with the Structural Welding Code-Steel, latest edition by the American Welding Society. The type and capacity of the welding equipment shall be in accordance with manufacturer's recommendations and shall be checked and approved by the welding inspector. At the beginning of each days work, a minimum of two test stud welds shall be made, with the equipment to be used, to metal which is the same as the actual work piece. The test studs shall be subjected to a 90° bend test by striking with a heavy hammer. After the above test, the weld section shall not exhibit any tearing out or cracking.

## Adhesive Anchors-Concrete

- Where "Hilti" or "Simpson" post-installed adhesive anchors or post-installed reinforcing bars in concrete are called out on plan, the following Hilti or Simpson adhesive products shall be used, respectively. Substitutions between or for other products shall be approved by the engineer prior to use.
  - Hilti HIT-HY 200 V3 Adhesive as manufactured by Hilti, Inc. ICC Report No. ESR-4868 revised June 2023.
  - Simpson "SET-36" Adhesive as manufactured by Simpson Strong-Tie, Inc. ICC-ES Report No. ESR-4051 reissued April 2023.
- Installation, inspection & testing of anchors shall be in accordance with the manufacturer's recommendations, ICC-ES report and these notes.
- Threaded rod anchors shall be F1554, Grade 36 u.n.o.
- Continuous special inspection is required in accordance with CBC Section 1701. Special Inspector must verify product, expiration date, concrete type and strength, anchor diameter and steel grade, compliance of drill bit, hole diameter and location, cleanliness of hole and anchor, adhesive application, and anchor embedment. See "Test and Inspections" section of plans for additional information. Where pull-test loads are designated on plan, each anchor type (loaded in either pullout or shear) shall have 50% of the anchors (alternate in each group arrangement) tested in tension to the tension load shown. If any anchor fails testing, all anchors of the same type not previously tested shall be tested until 20 consecutive anchors pass, then initial testing frequency may be resumed. Where pull-test loads are not shown, pull-testing is not required.
- The testing of the anchors shall be done by the Testing Laboratory and a report of the test results shall be submitted to the Building Dept. and the Architect/Structural Engineer. Testing shall occur after full epoxy cure time has elapsed (24 hours min). Where the number of anchors of a specific size and type exceed 100, the following testing procedure may be used. The first 40 anchors shall be tested as specified in note 5 above, then 10% of the additional anchors shall be tested. Any failure shall be handled in the same manner as specified in note 5 above.
- When installing anchors in existing concrete do not cut or damage existing reinforcing bars. Locate existing reinforcing bars with pachometer or x-ray if required.

## Concrete Masonry

- 28-day compressive strength of concrete masonry (fm) shall be fm = 2000psi for all uses. Full masonry stresses are used in design.
- Concrete block units shall conform to ASTM C-90. Units shall be lightweight with a maximum unit weight of 105 pcf.
- Mortar shall be Type S.
- Grout shall comply with ASTM C476. All cells to fully-grouted unless specified otherwise on plan.
- Compliance with the requirements for the specified compressive strength of masonry, fm shall be in accordance with section 51.4B of the TM5402/602-16. For unit strength method see table below for required 28-day compressive strength of the concrete block units, grout, and mortar.

specified fm	required 28-day compressive strength		
	conc. block units (psi)	grout (psi)	mortar (psi)
2000	2000	2000 min	1800
2250	2600	2250 min	1800
2500	3250	2500 min	1800

- Unit strength method shall not be used for specified compressive strengths in excess of 2500 psi.
- Reinforcing steel shall conform to ASTM A615-grade 60 for #4 and larger, grade 40 for #3 and smaller.
  - All reinforcement shall be continuous. Stagger splices where possible. Lap bars 4B diameters minimum, unless noted otherwise.
  - Vertical reinforcing shall be held in position at top and bottom and at intervals not to exceed 20B diameters.
  - Each vertical bar in walls shall lap 4B diameters with a dowel of the same size extending into the foundation. Carry each dowel to within 3" of the bottom of the foundation and terminate with 90° hook. Dowels shall be straight and plumb.
  - Place all horizontal bars in bond beam units. When 2 bars are used, stagger laps a minimum of 5'-0".
  - Provide 2-#5 bars (full height of wall at jamb and extending a minimum of 2'-6" past edges of openings at head and sill) each side of all openings and each end of all walls, unless noted otherwise on drawings.
  - Before block is placed on concrete, thoroughly clean concrete and remove all laitance and loose material. Rough concrete surface to 1/8" amplitude.
  - Concrete block masonry shall be built to preserve the unobstructed vertical continuity of the cells. All head and bed joints shall be solidly filled with mortar for a distance in from the face of the unit not less than the thickness of the face shell. Bond shall be provided by lapping successive courses or by equivalent mechanical anchorage.
  - Vertical cells shall have vertical alignment sufficient to maintain a clear unobstructed continuous vertical cell measuring not less than 2"x3".
  - Low Lift (lift height up to 5'-4")
    - All cells shall be filled solidly with grout. Grout shall be placed in a continuous pour in lifts not exceeding 5'-4" where cleanouts are not provided. All grouting shall be done under the continuous observation of the owner's testing laboratory.
  - High Lift (lift height greater than 5'-4" and up to 12'-8")
    - Cleanout openings shall be provided in the bottom course of wall to be filled at each lift or pair of grout where such lift or pair of grout is in excess of 5'-4" in height. Maximum lift or pour height shall not exceed 12'-8". Cleanouts shall be provided at each cell. However, if the course at the bottom of the pour is constructed entirely of inverted open-end bond beam units, cleanout openings need only be provided at reinforced cells. Maximum cleanout spacing shall not exceed 32"cc. The cleanouts shall be sealed after inspection and before grouting.
  - Thoroughly clean all cells and bond beams of mortar projections, mortar droppings, or other foreign material before grouting.
  - All grout shall be thoroughly consolidated by mechanical vibration during placement in a manner to provide solidly grouted spaces.
  - When grouting is stopped for one hour or longer, horizontal construction joints shall be formed by stopping the pour of grout 1/2" below the top of the uppermost unit.
  - All embedded items (bolts, etc.) shall be securely positioned prior to grouting. Provide a minimum of 1" grout around all bolts in masonry. See Typical Details Sheet.
  - Pipes and electrical conduits shall not be embedded in concrete masonry except where specifically approved by the structural engineer.
  - Use open end block for all stack bond construction.

## Window System Design Criteria

- 2022 CBC Wind Speed 95 mph, exposure C.
- All mullions and their connections shall be designed to span between structural supports as shown on drawings. Verify ceiling heights with architectural drawings.
  - All mullions and their connections shall allow for a relative movement between supports of not less than 1/2" due to seismic loads.
  - Submit complete shop drawings and calculations signed by a Civil Engineer registered in the state in which the project is located, prior to fabrication. Details provided in these drawings are for reference only. Window system manufacturer shall design and supply all connection materials (including embedded items, diagonal bracing angles, brackets, outriggers, etc.) as required for the support of the window system. Embedded items shall be installed by the Contractor.

## Abbreviations

add..... Additional	LLH.....Long leg horizontal
alt..... Alternate	LLV.....Long leg vertical
AISC..... American Institute of Steel Construction	LVL.....Laminated Veneer Lumber
APA..... American Plywood Association	MB.....Machine bolt
ASTM..... American Society for Testing and Materials	mfr.....Manufacturer
AWS..... American Welding Society	max.....Maximum
AB..... Anchor bolt	mech.....Mechanical
∅..... And	min.....Minimum
arch..... Architect/Architectural	misc.....Miscellaneous
∅..... At	mm.....Metric
b.o..... Bottom of	N.I.C.....Not in contract
bm..... Beam	(N).....New
brg..... Bearing	nts.....Not to scale
btr..... Better	#.....Number or pounds
btwn..... Between	o'.....Over
bldg..... Blocking	oc.....On center
B.S..... Both sides	ONJ.....Open web joist
bolt..... Bolt	ops.....Opening
BN..... Boundary nail	opp.....Opposite
clg..... Ceiling	O.H.....Opposite Hand
cc..... Center to center	o.d.....Outside diameter
CF..... Center line	pen.....Partial penetration
clr..... Clear	pc.....piece
col..... Column	pl.....Plate
CP..... Complete Penetration	ply, plywd...Plywood
CR..... Concrete	psf.....Pounds per cubic foot
CMU..... Concrete masonry unit	psf.....Pounds per square foot
conn..... Connection	psi.....Pounds per square inch
CJ..... Construction Joint	PAF..... Powder Activated Fasteners
cont..... Continuous	PTDF..... Pressure Treated Douglas Fir
csk..... Countersink	r, rad..... Radius
CTJ..... Control Joint	RDVD..... Redwood
DL..... Dead Load	rein..... Reinforcing
dst..... Detail	req'd..... Required
diag..... Diagonal	rt..... Roof
dia..... Diameter	R.O..... Rough opening
do..... Ditto	∅..... Round or diameter
D.F..... Douglas Fir	sched..... Schedule
dbl..... Double	S.A.D..... See architectural drawings
dn..... Down	S.E.D..... See electrical drawings
dwg..... Drawing	S.M.D..... See mechanical drawings
ea..... Each	SMS..... Sheet Metal Screws
E.F..... Each Face	SDS..... Simpson Strong-Drive Screw
embed..... Embedment	SDSTS..... Self drilling self tapping screw
EN..... Edge Nail	SC..... Shear connector 3/4"φ u.n.o.)
eq..... Equal	shtg..... Sheathing
eq..... Equipment	sht.....Sheet
est..... Estimate	SHS.....Sheet metal screw
elav, el...Elevation	sim..... Similar
eq..... Equal	s.o.g.....Slab on grade
∅..... Expansion Joint	∅.....square
FC..... Face of Concrete	stagg.....Staggered
FB..... Face of Block	std.....Standard
FF..... Face of Masonry	stl.....Steel
FF..... Face of Plywood/Sheathing	ssl.....Stainless Steel
FS..... Face of Stud	stfr.....Stiffener
fin..... Finish	str.....Structural
FL..... Finish Floor	SP..... structural plywood
F.G..... Finish grade	SPEN..... structural plywood edge nailing
flr..... Floor	symm..... Symmetrical
ftg..... Footing	T.N..... Toe nail
ftd..... Foundation	t4b..... Top & bottom
f.o..... Face of	t.o.c..... Top of concrete
frmg..... Framing	t.o.f..... Top of framing
galv..... Galvanized	t.o.p..... Top of plate
ga..... Gage	t.o.S..... Top of Steel
glb..... Glued-laminated beam	t.o.w..... Top of Wall
g.l..... Grid Line	t4g..... Tongue & Groove
hgr..... Hanger	T5..... Tube Steel
hd..... Header	typ..... Typical
ht..... Height	u.n.o..... Unless noted otherwise
H5B..... High strength bolt	v..... Vertical
H55..... Hollow Steel Section	v.i.f..... Verify in field
hook..... Hook	w.....Width
horiz..... Horizontal	w/n.....With
i.d..... Inside diameter	w/o.....Without
int..... Interior	WS..... Wood screw
inv..... Inverted	WP..... Working point
ist..... Joist	WFS..... Welded headed studs
Jh..... Joist hanger	WLF..... Welded wire fabric
L.S..... Lag screw	WCLB..... West Coast Lumber
lt. wt..... Light weight	Inspection Bureau
LL..... Live Load	

## Adhesive Anchors-Concrete Masonry

- Use Hilti HIT-HY 210 adhesive as manufactured by Hilti, Inc., Tulsa, Oklahoma. ICC-ES Report No. ESR-4143 issued January, 2022, u.n.o.
- Installation of anchors shall be in accordance with the manufacturer's recommendations, ESR report, and these notes.
- Special inspection is required in accordance with 2022 CBC Section 1704 (DSA/OSFPD) shall be per 1704(A) & the ESR report. Special Inspector must verify:
  - Anchor type, diameter & length, and adhesive product type & expiration.
  - Installation description, including verification of masonry compressive strength, anchor installation & location (spacing & edge distance) in accordance with the manufacturer's published instructions and ESR Report.
- Do not cut or damage existing reinforcing bars. Locate with pachometer or x-ray if necessary.
- Base material temperature @ time of installation shall be between 23°-104° F.
- Anchors shall be A36 threaded rod, u.n.o. Anchors exposed to exterior weathering conditions shall be Type 304 or 316 stainless steel or hot-dipped galvanized.
- Each anchor type (loaded in either pullout or shear) shall have the following percentage of anchors pull-tested to the test load shown in the table below. If any anchor fails testing, all anchors of the same type not previously tested shall be tested until 20 consecutive anchors pass, then initial testing frequency may resume.
  - Structural anchorage: 100% u.n.o.
  - Anchorage of non-structural elements: 50%
  - Sill E bolts: 10%
- Pull-testing shall be done by the Testing Laboratory in the presence of the Special Inspector and a report of the test results shall be submitted to the Building Dept. and the Architect/Structural Engineer. Testing shall occur 24 hrs. minimum after anchor installation.

Rod Diam.	Min Embed	Face of CMU Test Load (lbs)		Top of CMU Test Load (lbs)
		4" Edge Dist	20" Edge Dist	
3/8"	3 3/8"	2000	2500	n/a
1/2"	4 1/2"	3100	4100	2400
5/8"	5 5/8"	4100	5100	2400
3/4"	6 3/8"	5100	7600	n/a



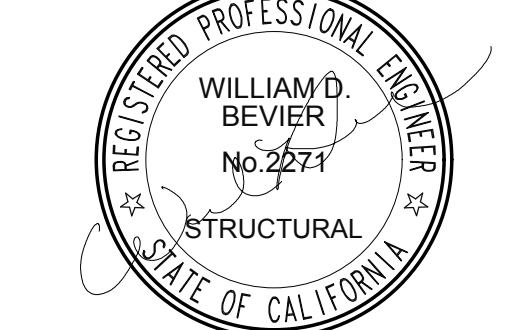
555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net

www.architechnica.net

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2479 Sunrise Blvd.  
Gold River, CA 95670  
Tel: (916) 631-3030  
Fax: (916) 631-8996  
Web: www.bevier.net  
Bevier Job No.23011



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CODESTACK  
ACADEMY

201 N CALIFORNIA ST.  
STOCKTON, CA 95202

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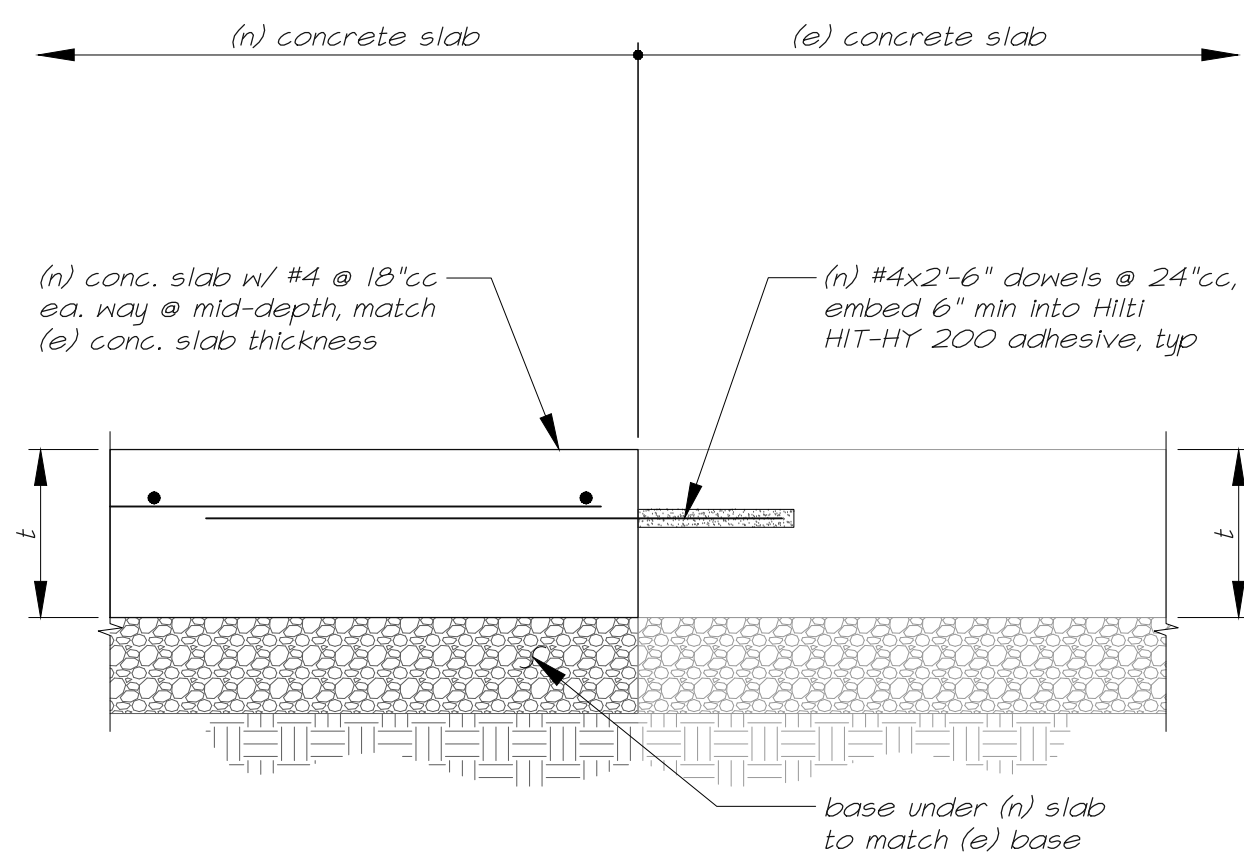
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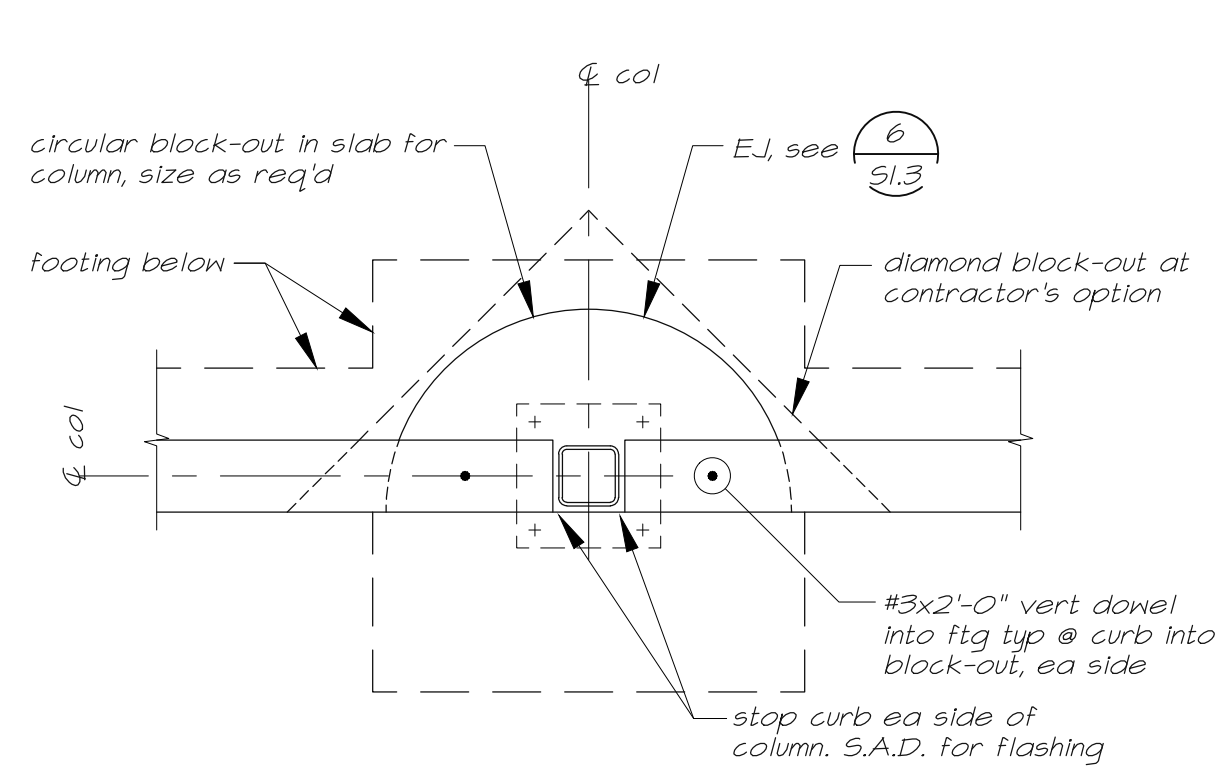
GENERAL NOTES

S1.2

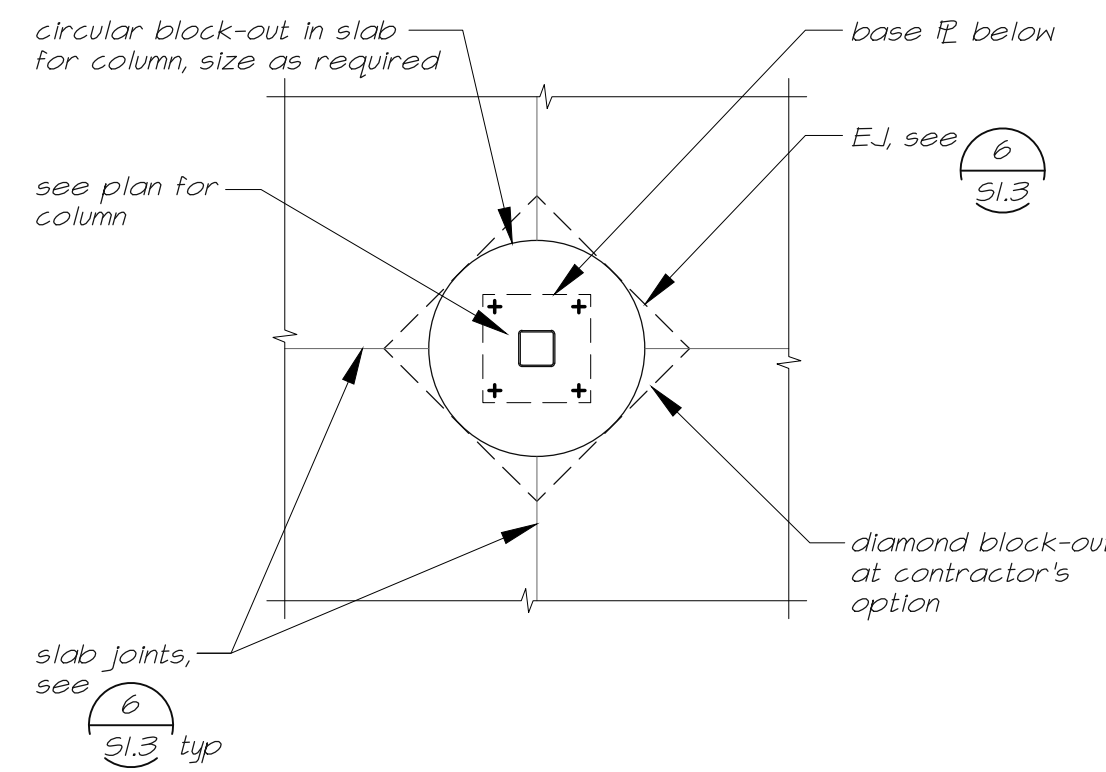




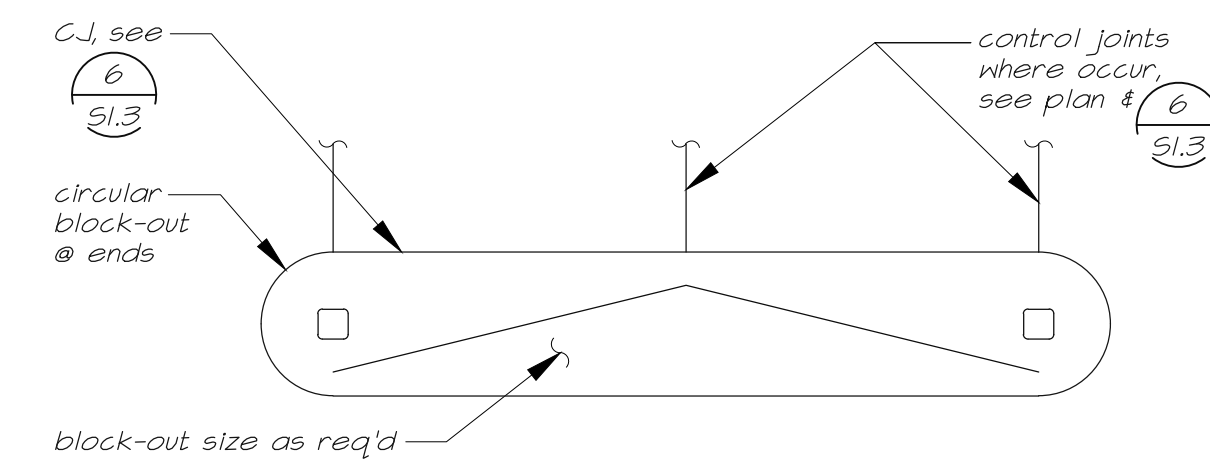
9 Typical (n) to (e) Slab  
S1.3



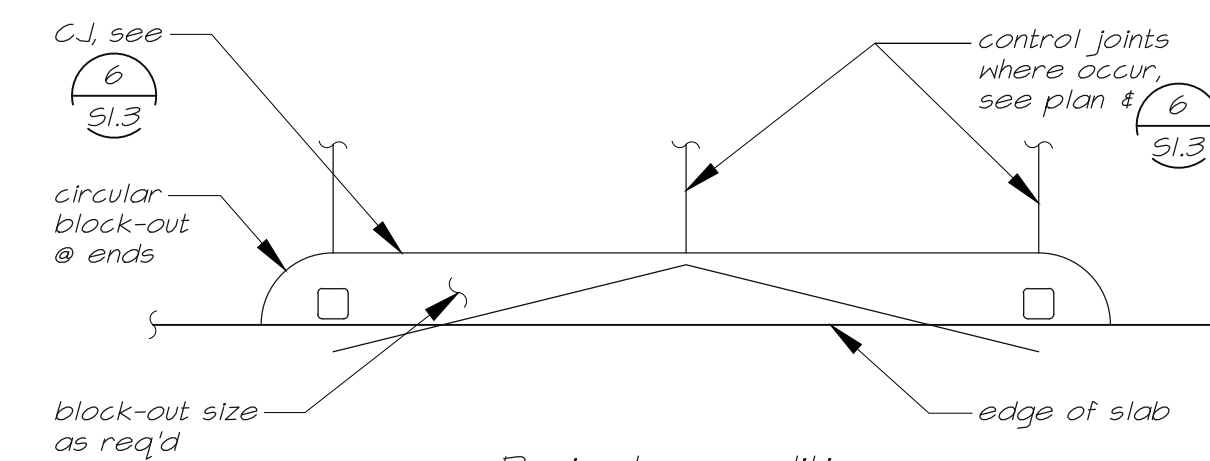
10 Typical Slab Block-out  
perimeter  
S1.3



11 Typical Slab Block-out  
interior  
S1.3

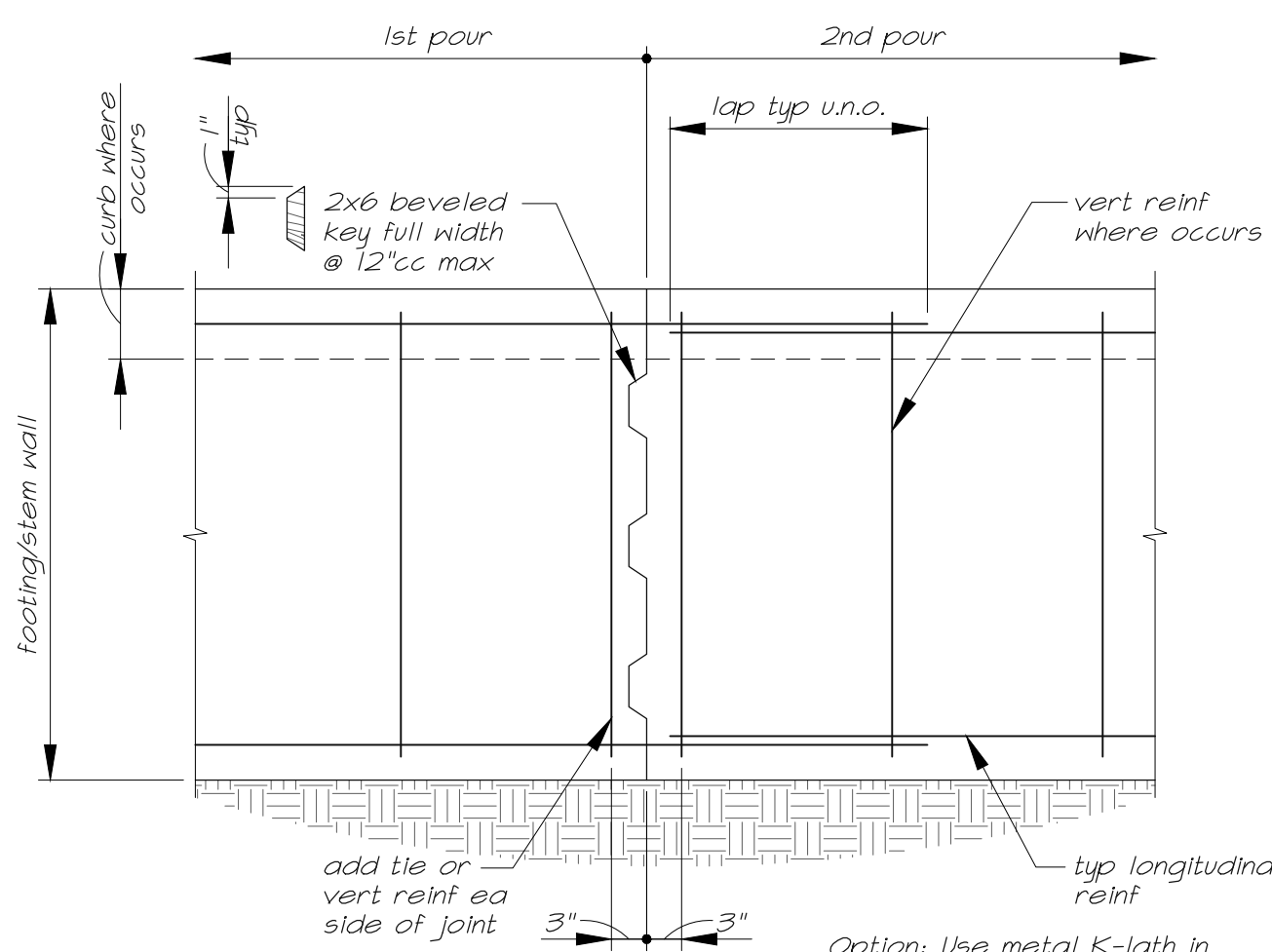


Interior condition

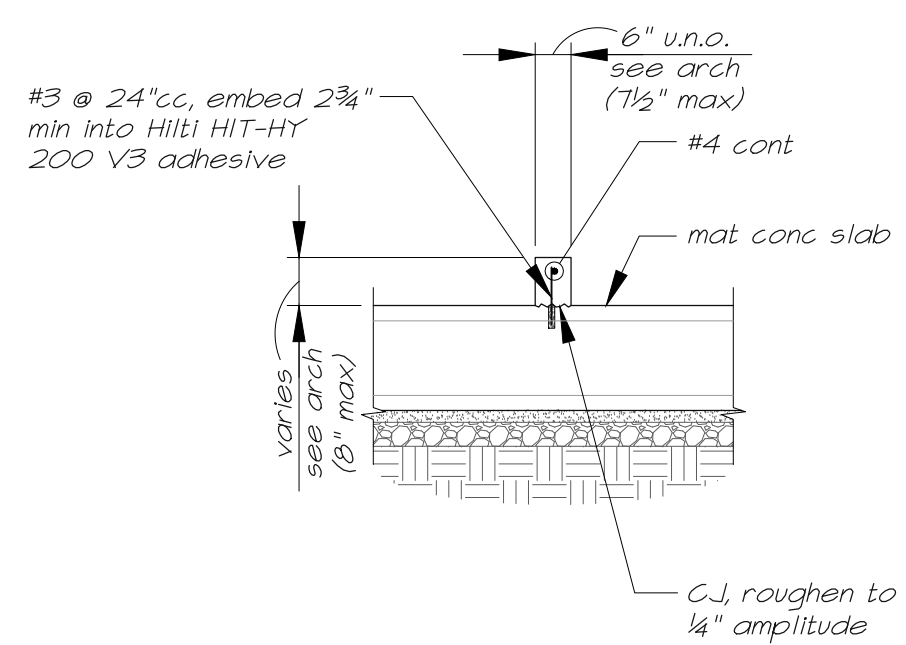


Perimeter condition

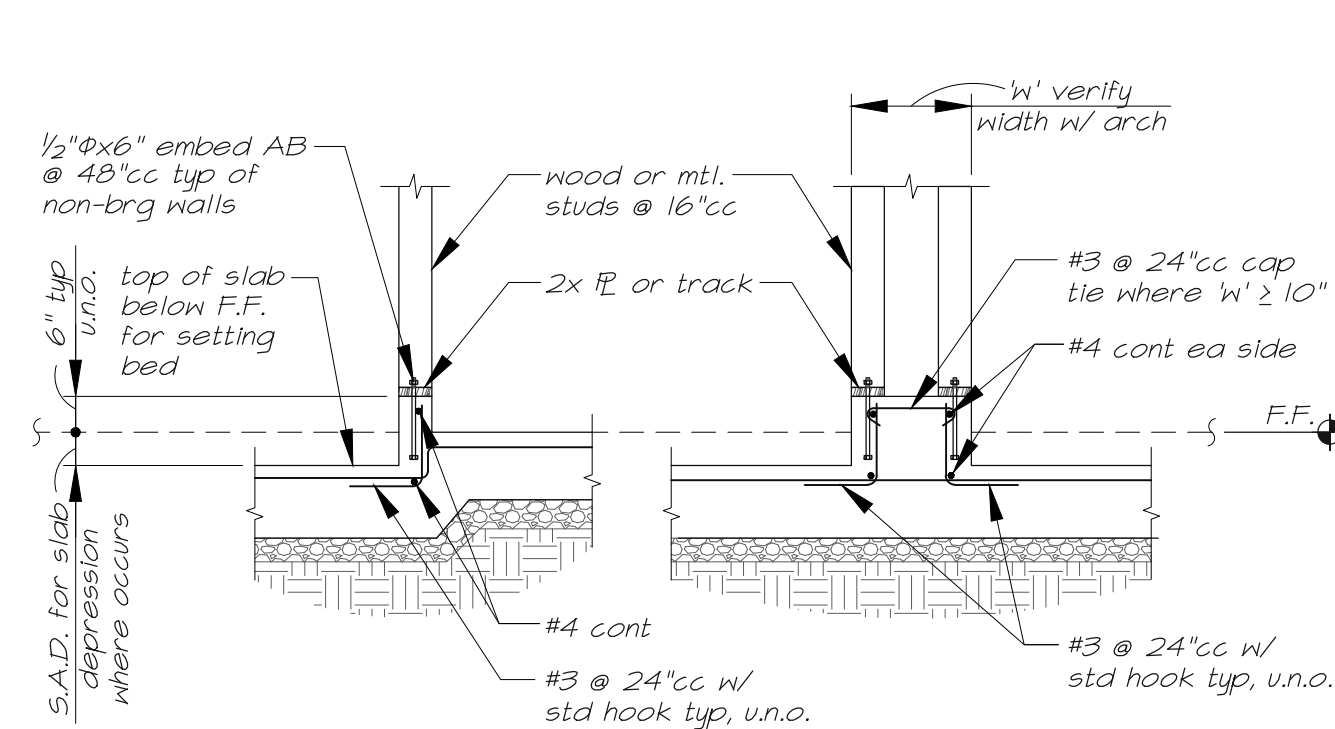
12 Typical Block-out  
@ braced frame  
S1.3



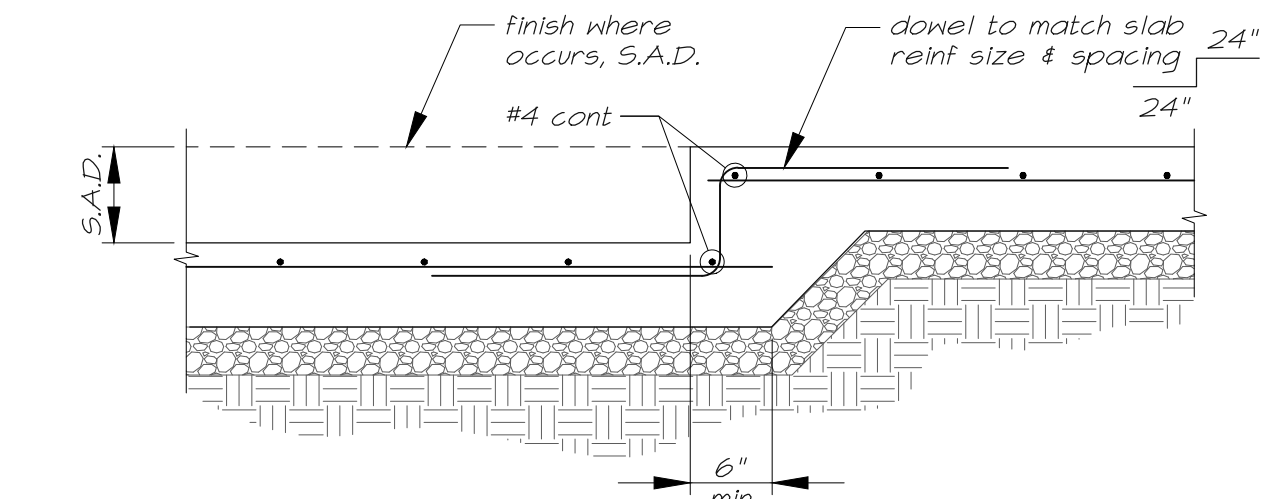
5 Typical Foundation Construction Joint  
S1.3



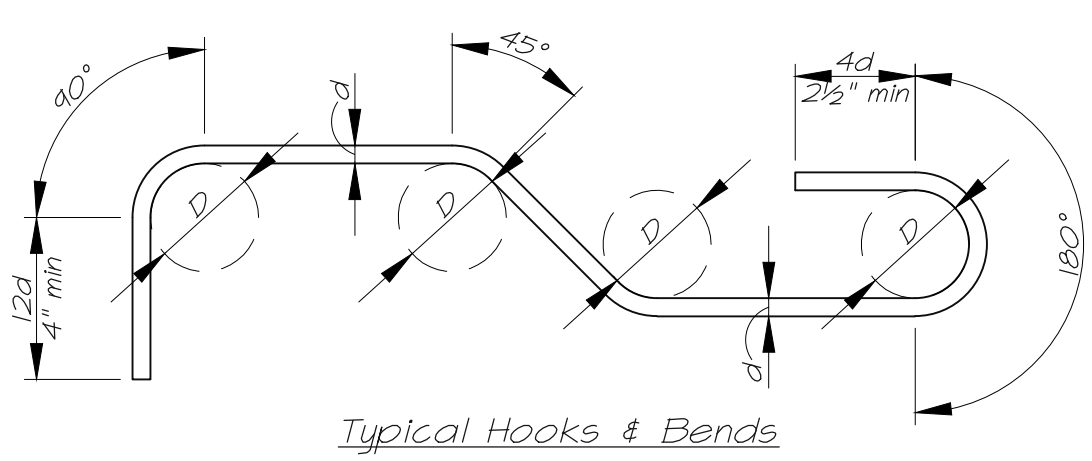
6 Typical Concrete Curb  
S1.3



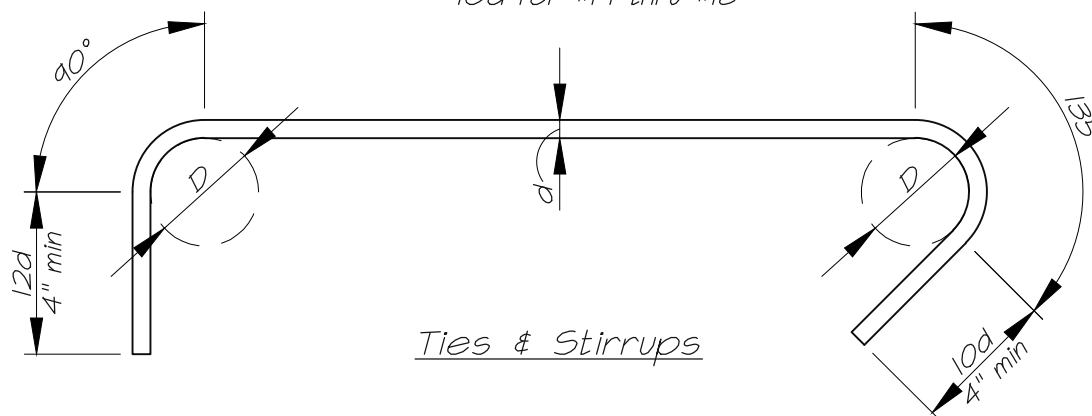
7 Typical Concrete Curb  
(at slab depression & double wall)  
S1.3



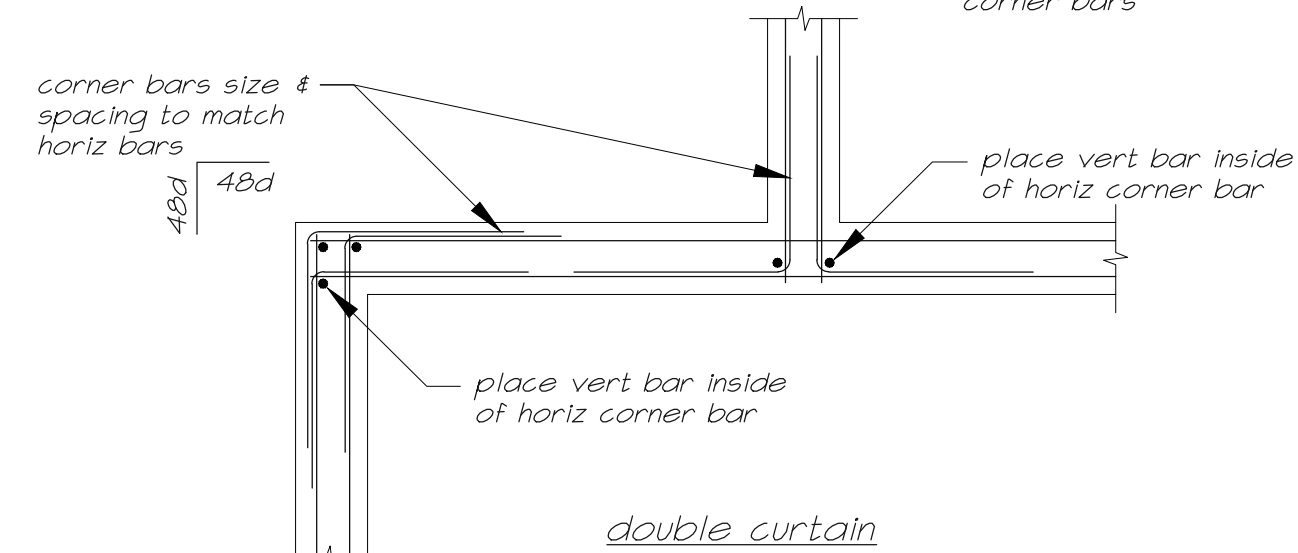
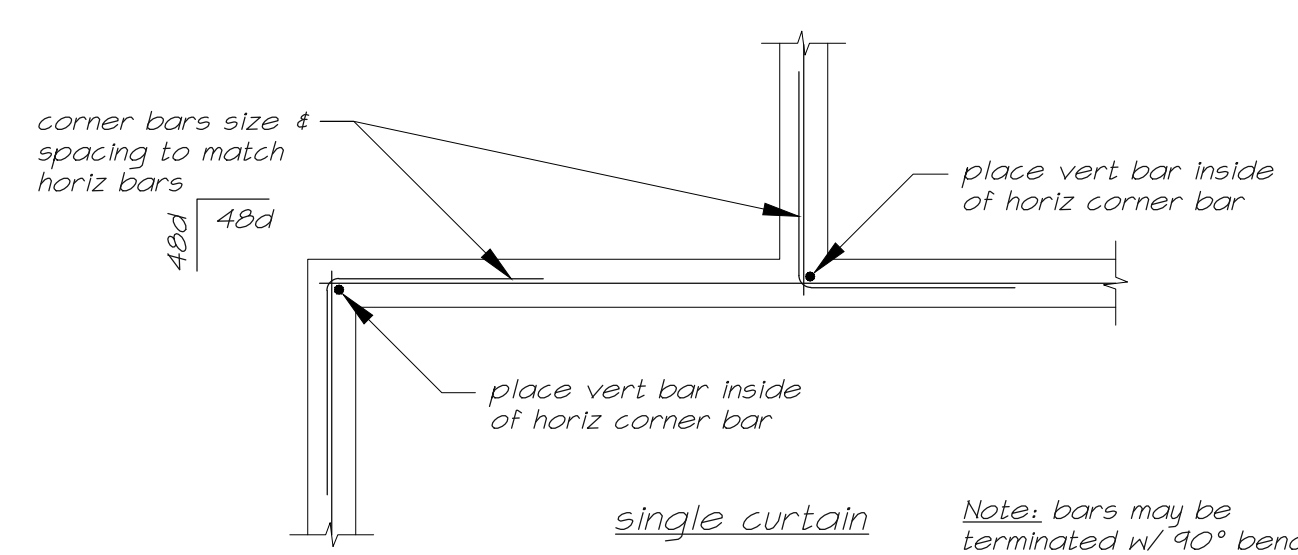
8 Typical Slab Depression  
S1.3



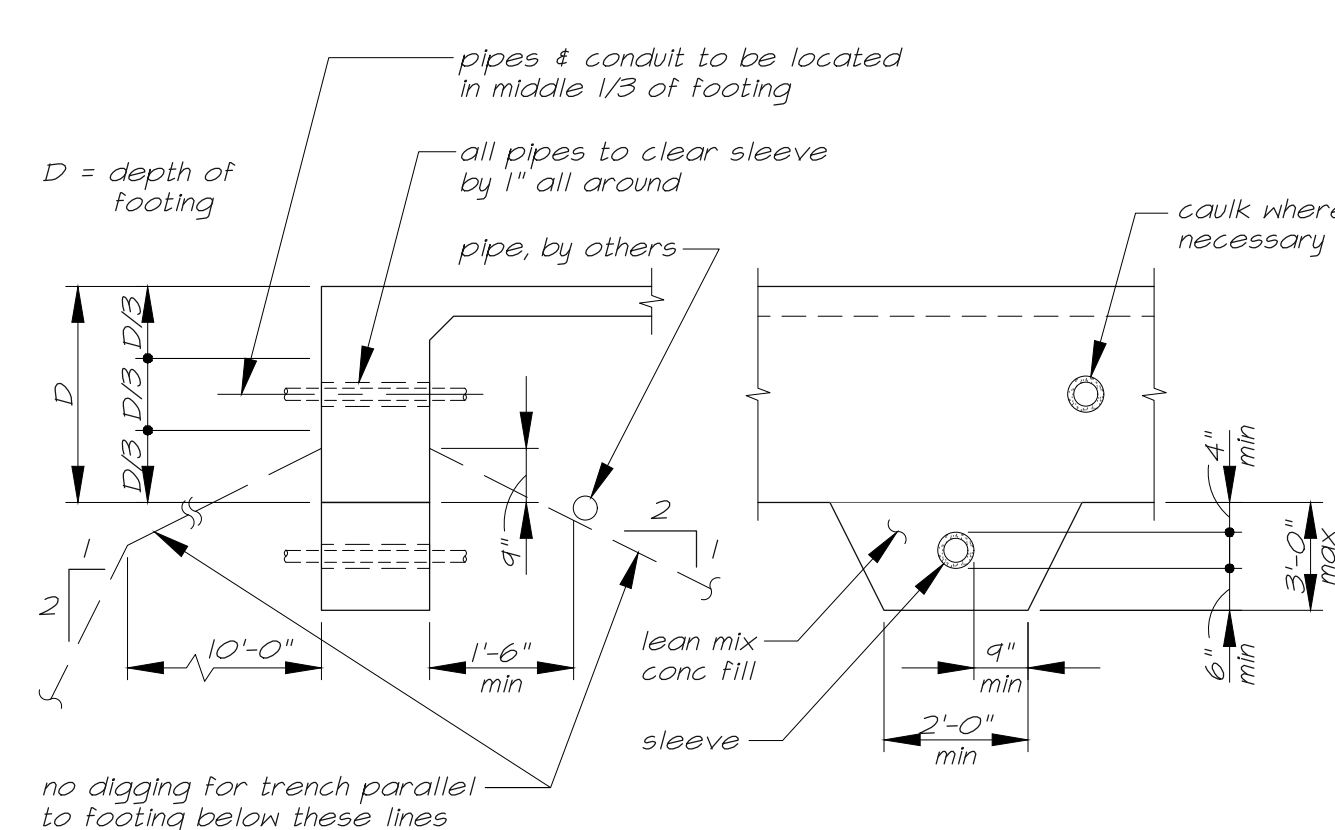
d = Rebar Diameter  
D = 6d for #3 thru #8  
8d for #9 thru #11  
10d for #14 thru #18



1 Rebar Hooks & Bends  
S1.3



2 Typical Corner Reinforcing  
S1.3



3 Concrete Footing @ Pipes & Conduits  
S1.3



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@archtechnica.net  
www.archtechnica.net

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2479 Sunrise Blvd.  
Gold River, CA 95670  
Tel: (916) 631-3030  
Fax: (916) 631-8996  
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TYPICAL DETAILS

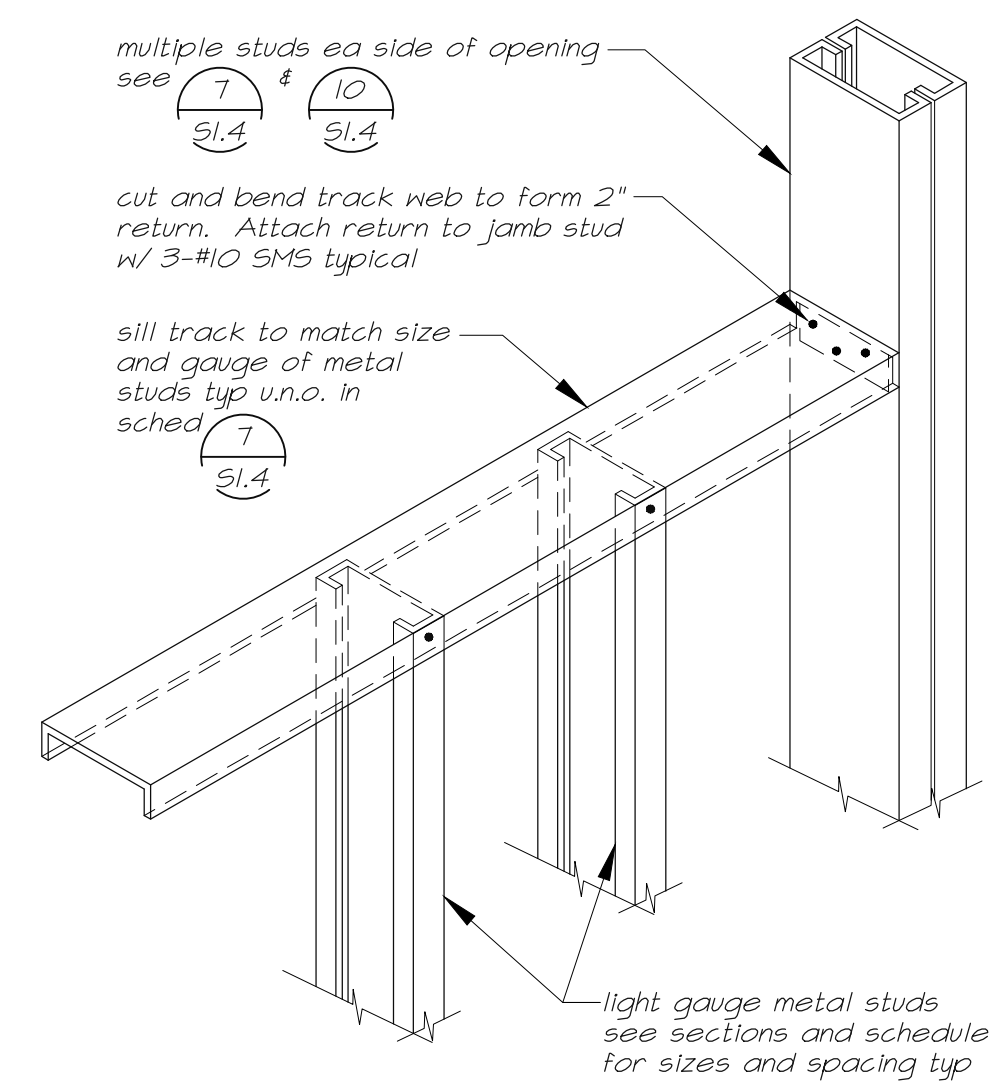


W' Opening width max	Required jamb stud	Required header section	Required sill section
4'-0" max	dbl 600S137-43	dbl 600S137-43 w/ 3-600T125-43 t4b	600T125-43
5'-0" max	dbl 600S137-43	dbl 600S137-54 w/ 600T125-43 t4b	600T125-54
12'-0" max	dbl 600S162-54	dbl 600S162-54 w/ 600T125-54 t4b	600T125-54

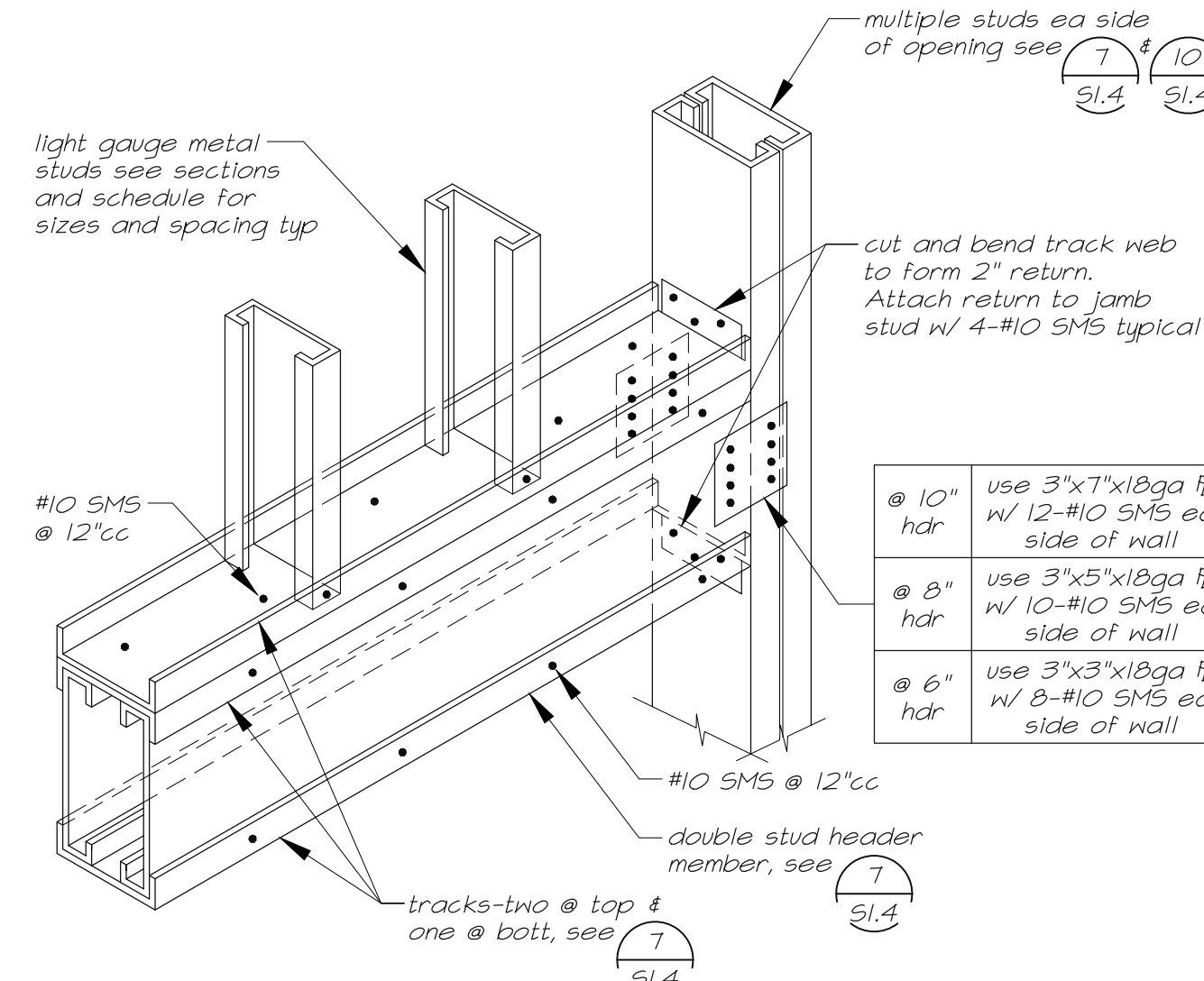
**Schedule Notes:**

- The requirements of this schedule shall govern unless specifically detailed or noted otherwise.
- Metal stud section properties shall conform to the Steel Stud Manufacturer's Association Product Catalog (SSMA) as specified in the Lightgauge Steel notes.
- All header members shall be un-pinched.
- At interior non-bearing conditions, track width @ headers and sills shall match depth of studs as specified in the architectural drawings. Flange length & ga shall match this table.
- At spans over 12'-0", provide diagonal bracing of headers/sills to structural framing @ 4'-0"cc max per sections. ("N" = 4'-0" max @ these locations)
- Maximum width shown in table is either the width between the jamb studs or between diagonal bracing as shown in note #5 above (where diag bracing occurs.)

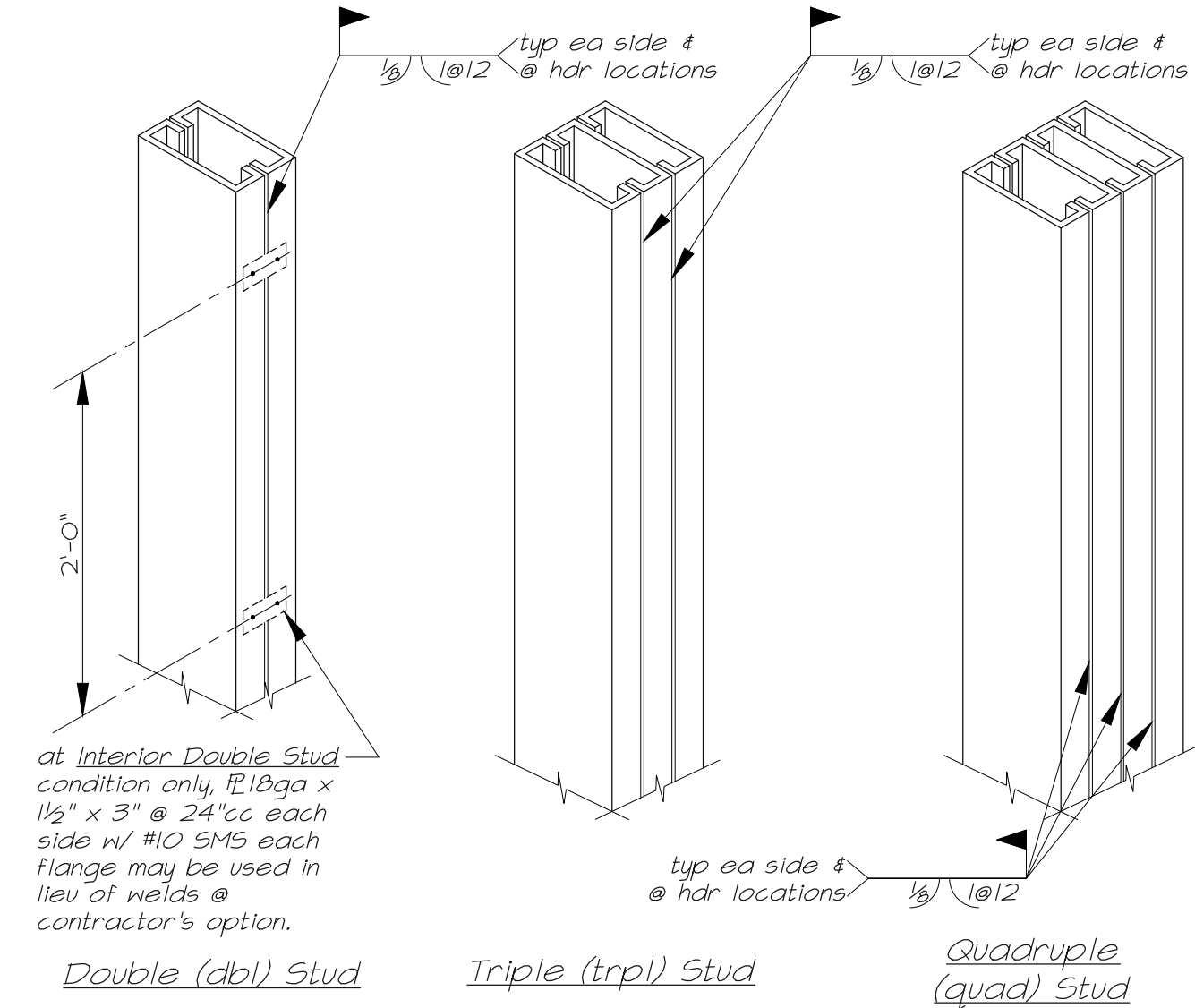
7 Exterior Wall Header Schedule



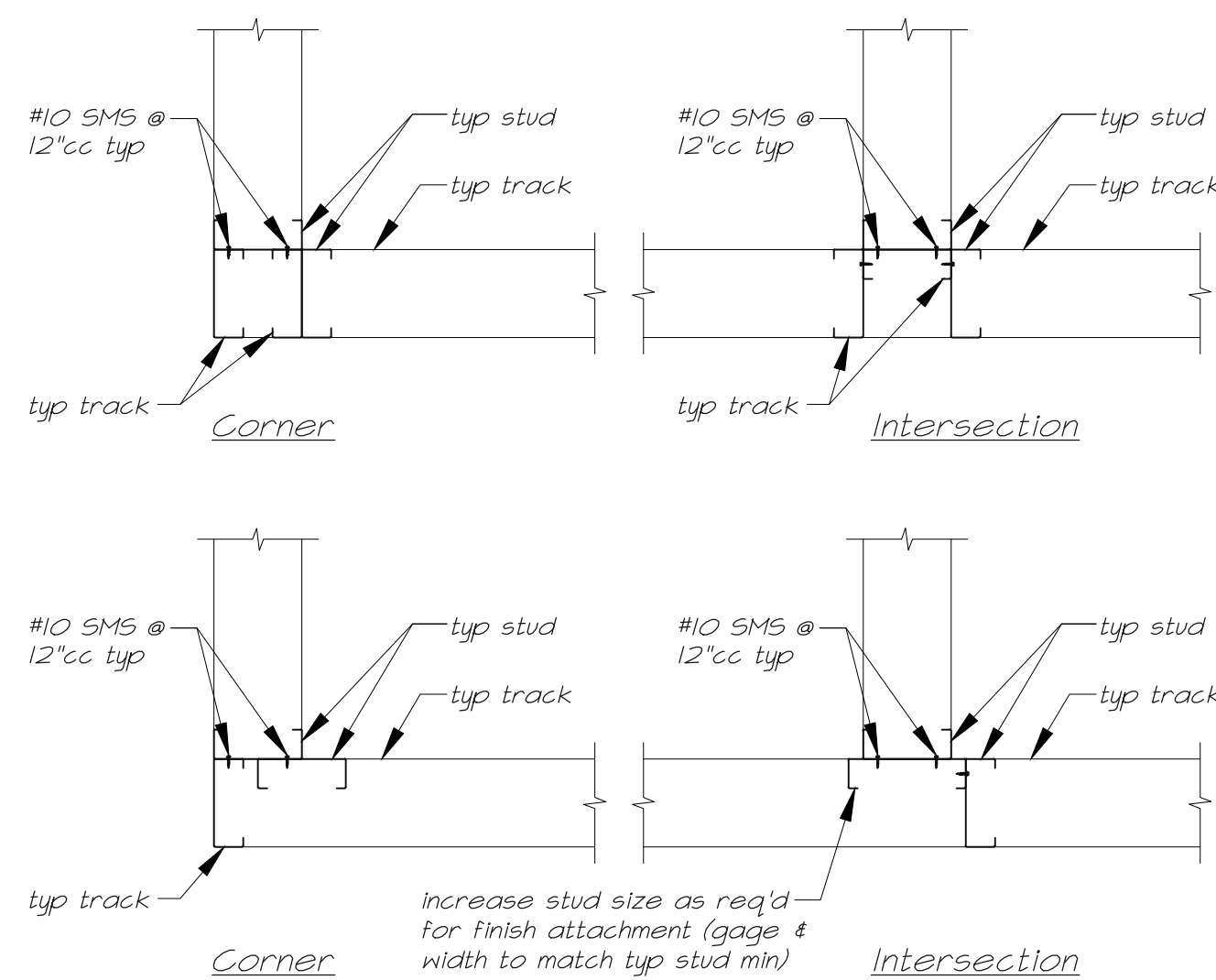
8 Typical Sill Framing



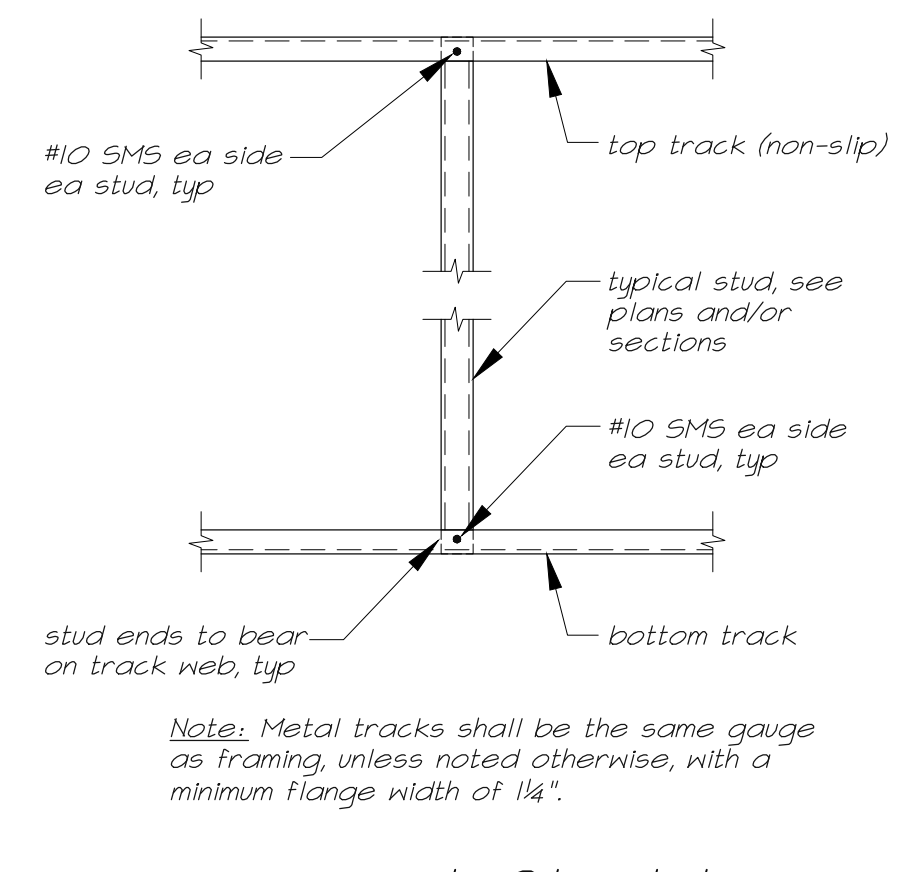
9 Typical Header Framing



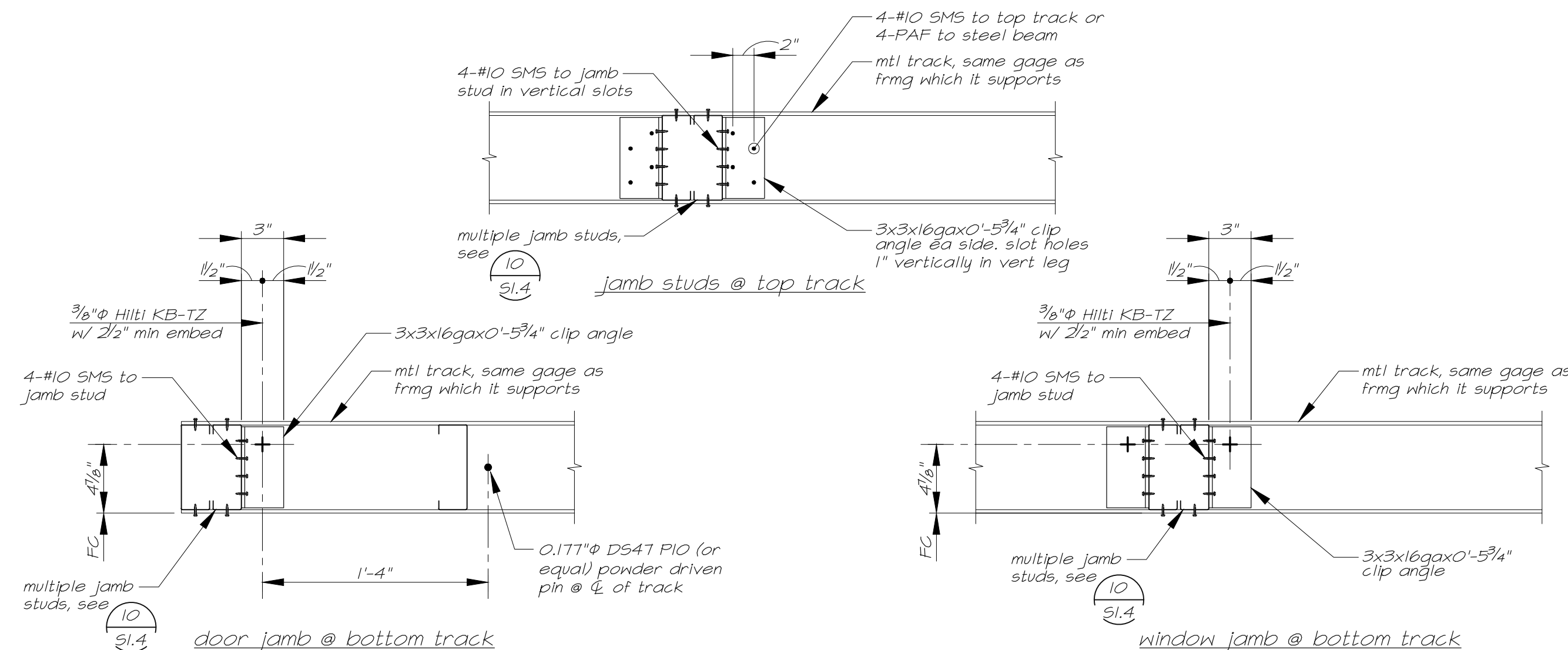
10 Typ Multiple Stud Connection



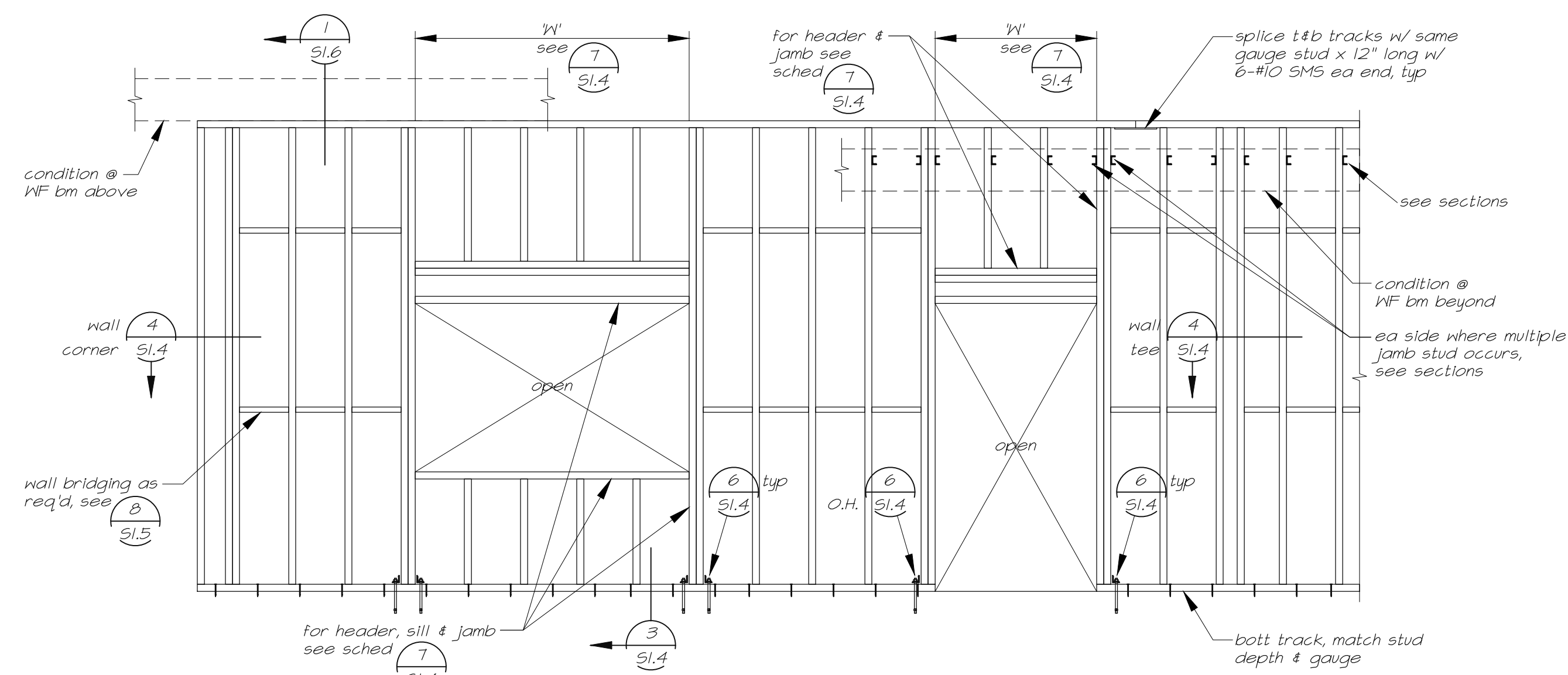
4 Typical Wall Framing Plans



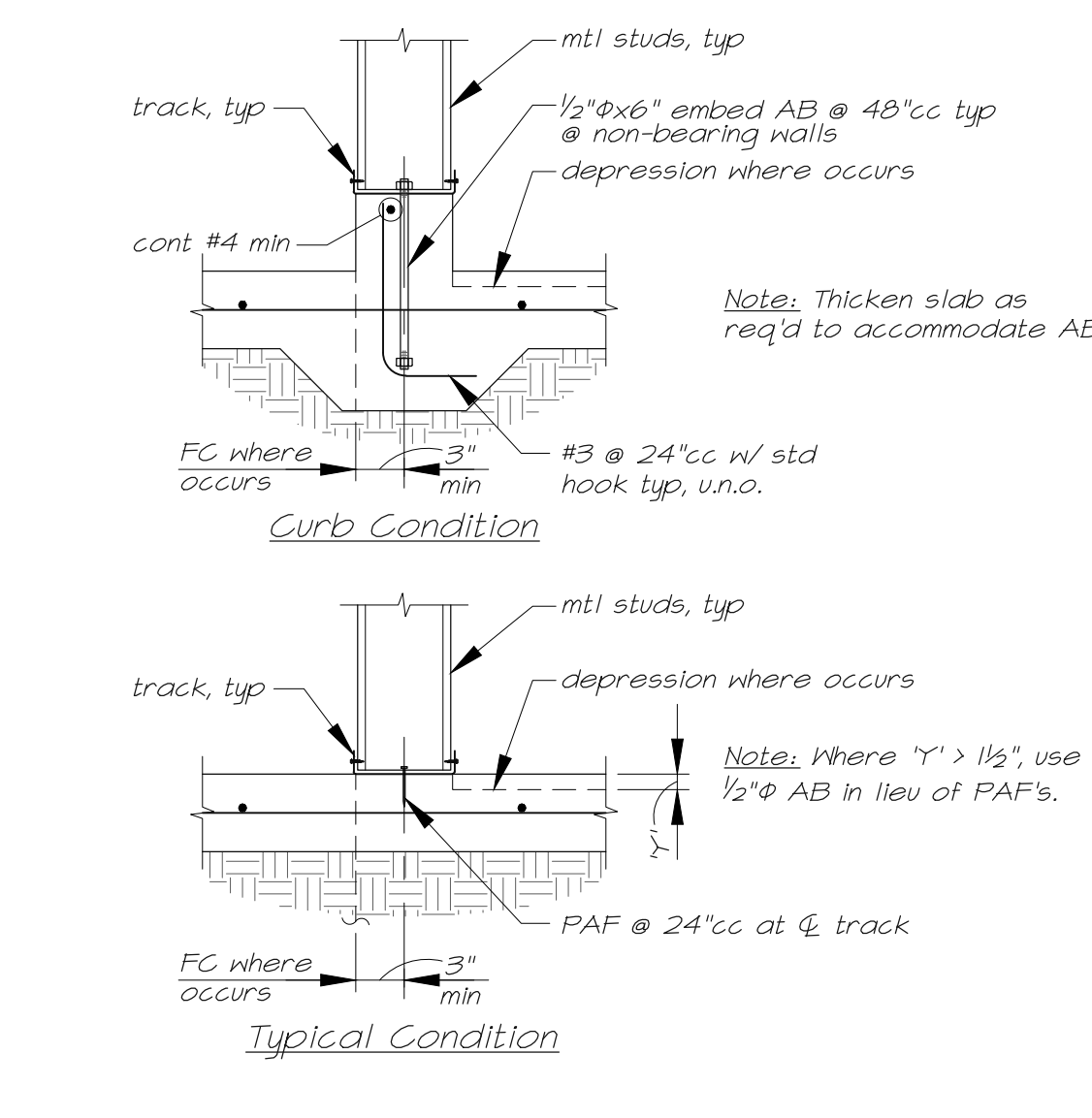
5 Typical Stud to Track Connection



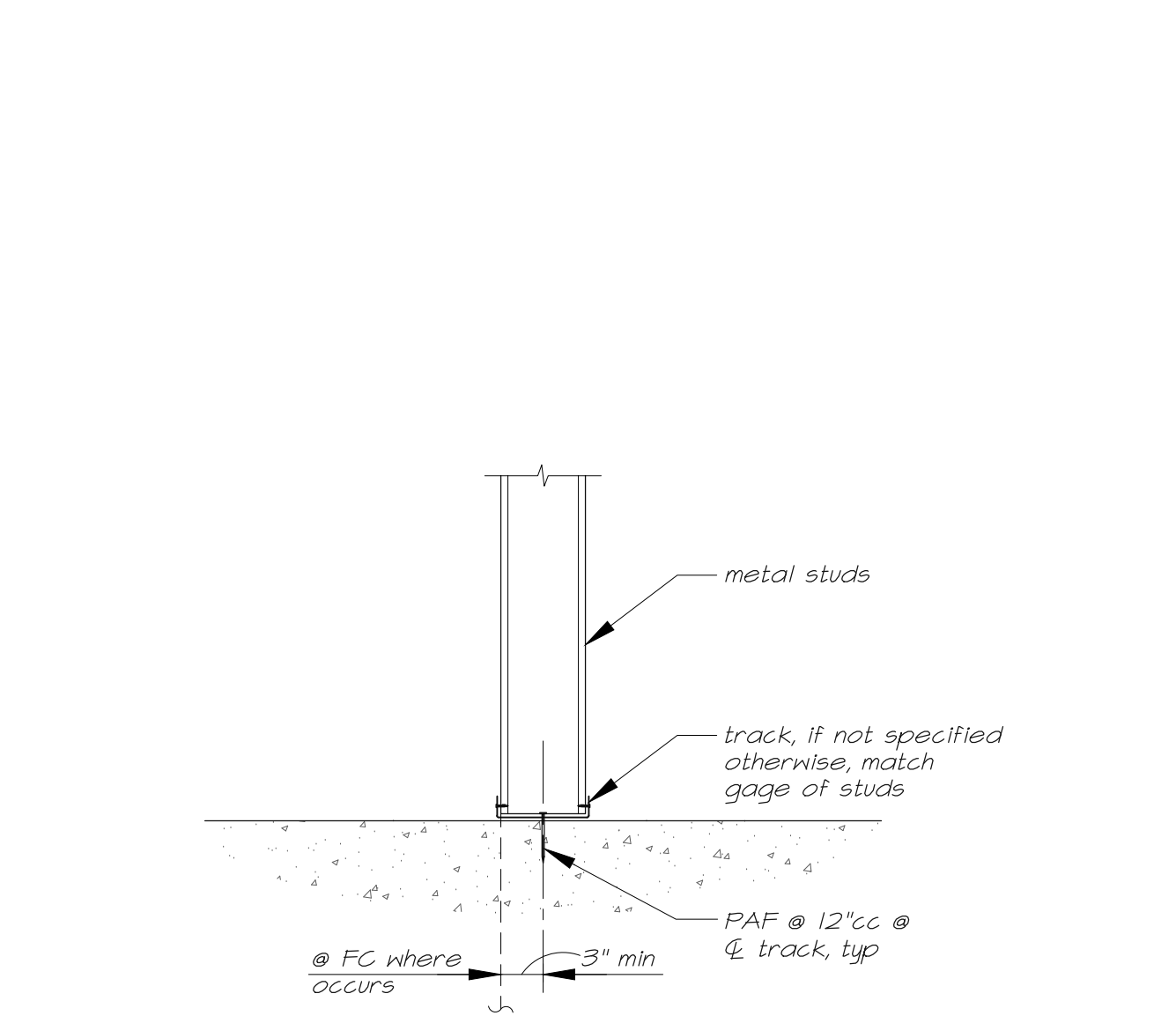
6 Typical Plan Details



1 Typical Exterior Wall Framing Elevation



2 Typ Sill Anchorage to Conc



3 Typ Anchorage to Concrete



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net  
www.architechnica.net

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Gold River, CA 95670  
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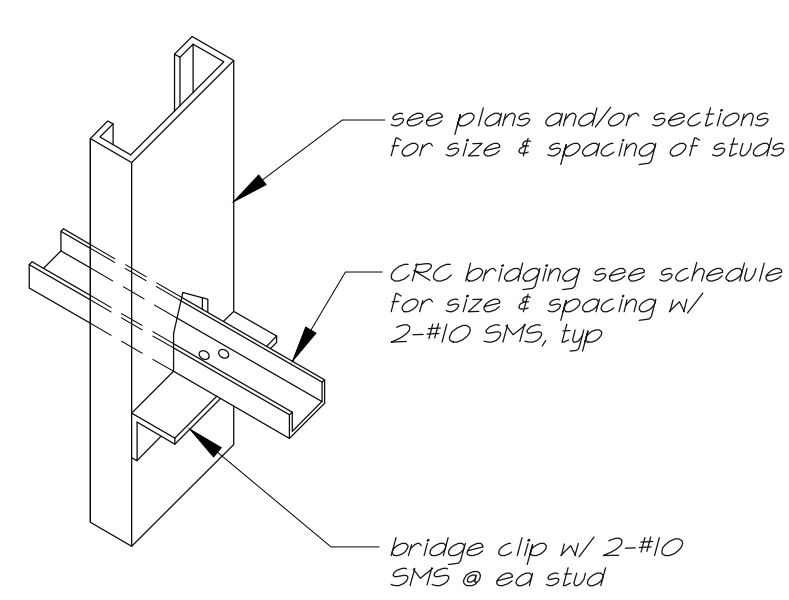
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TYPICAL DETAILS

S1.4

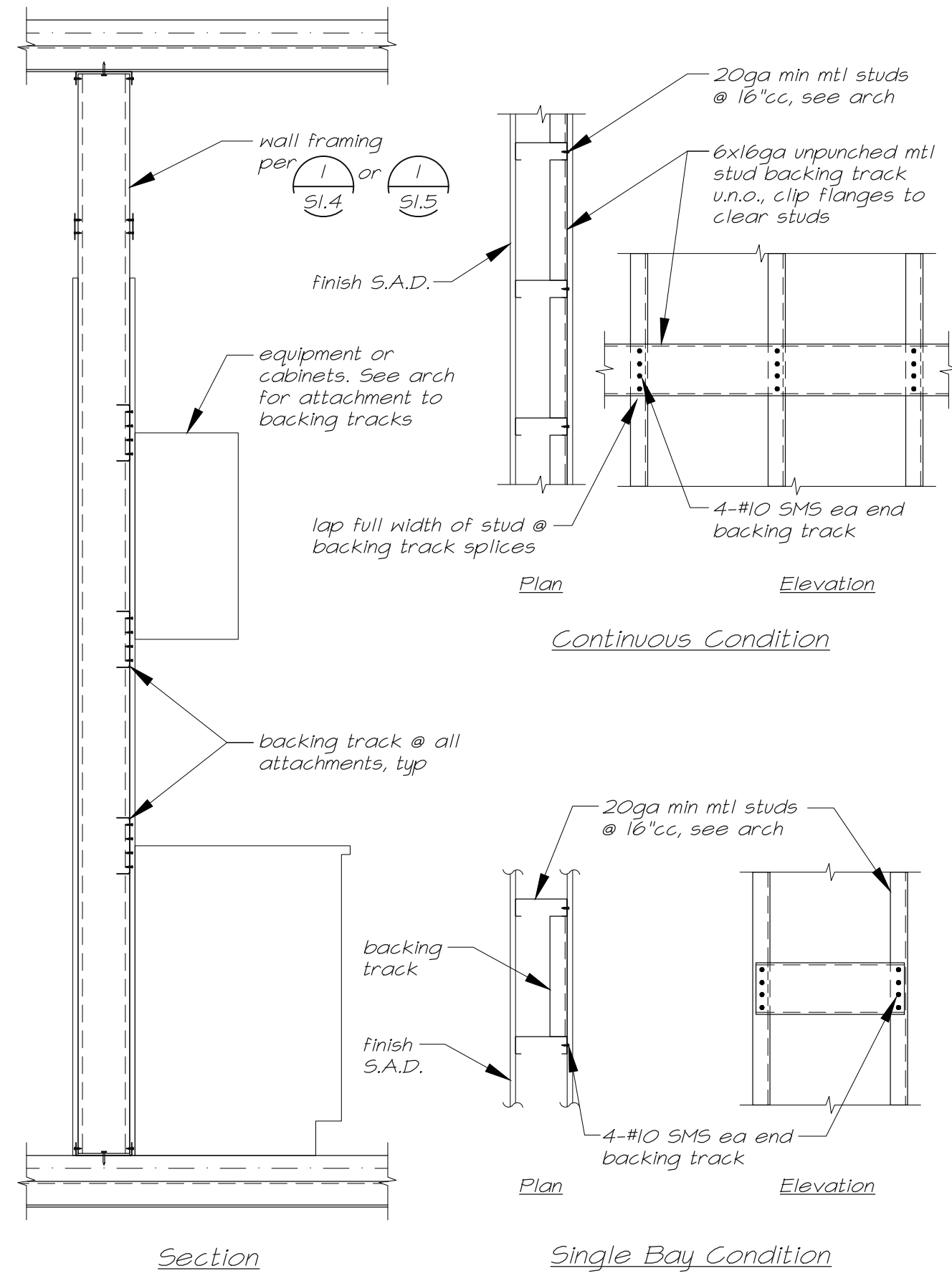




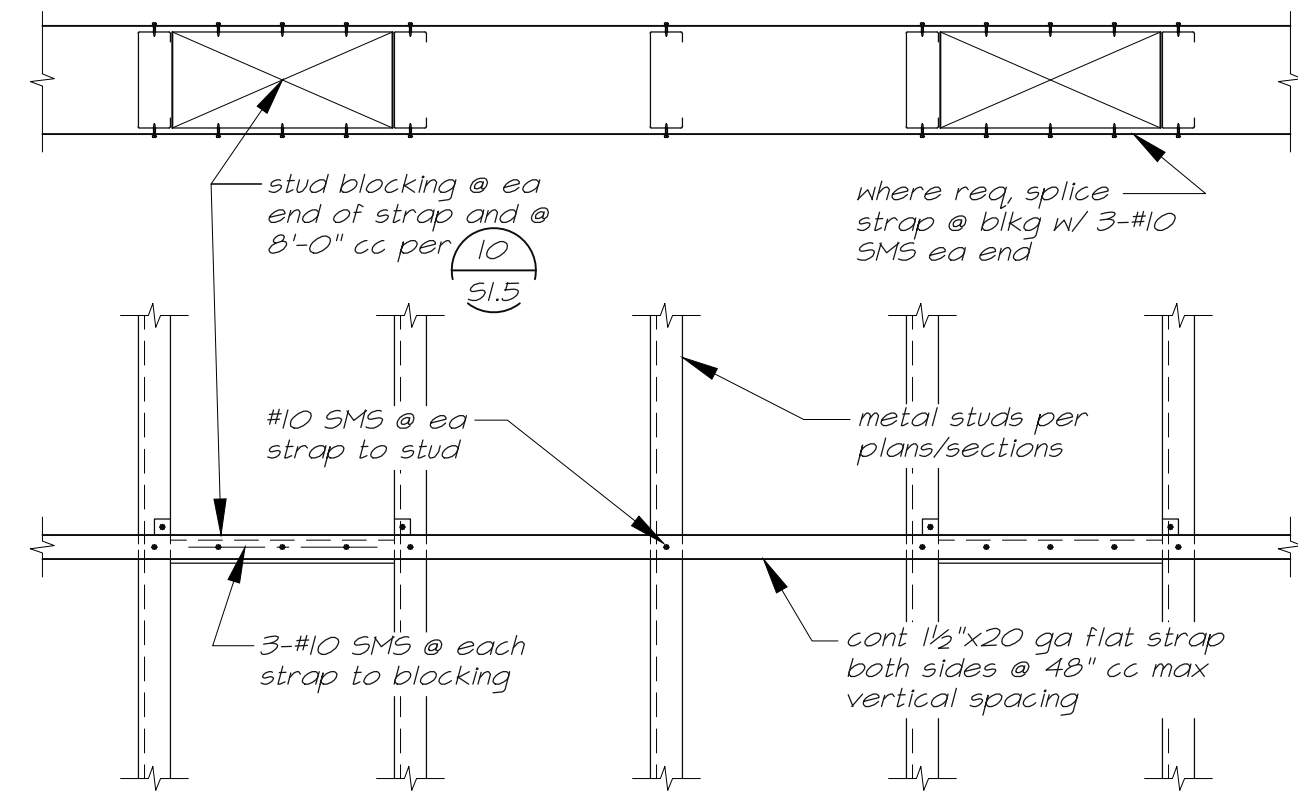
Bridging in Punch-Outs		
Stud Size	Bridging Member	Bridge Clip
3 1/2"-8"	CRC-1 1/2"x1 1/2"ga @ 4'-0"cc max	1 1/2"x2"x1 1/2"ga x length to be 1/4" less than stud width LLH typ

- Notes:
- CRC = Cold Rolled Channel
  - Bridging is not req'd when gyp board is applied to both sides of the wall

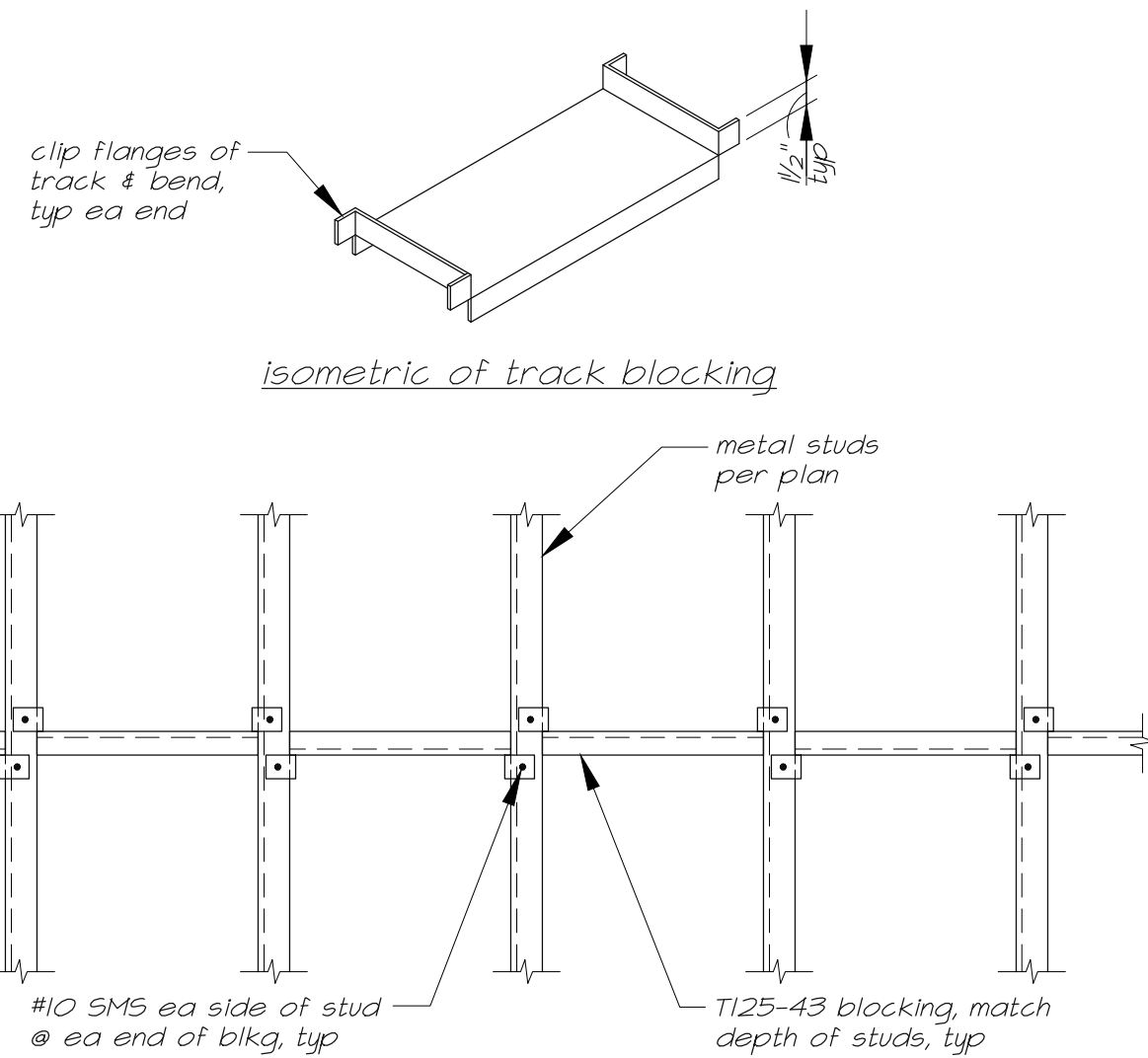
Typical Stud Bridging



Typical Backing



Typ. Flat Strap Bridging



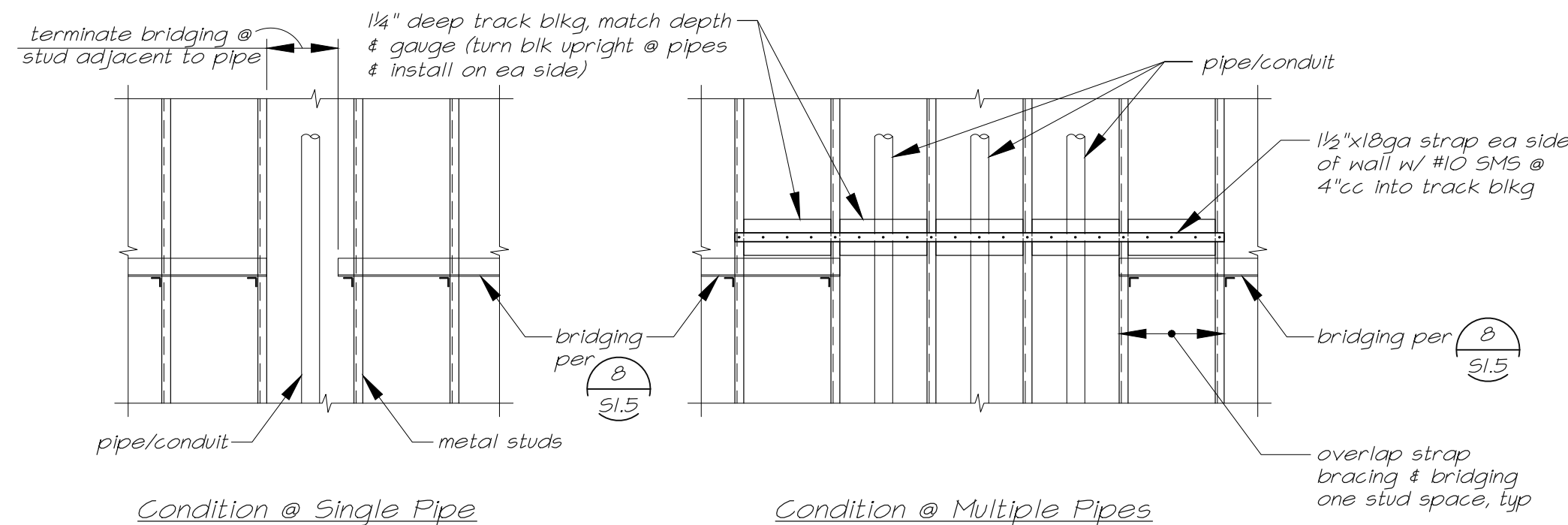
Typical Blocking Elevation

Depth	Gage	Designation <sup>3</sup>	Height	
			@ 16"cc	@ 24"cc
4"	20	400S162-33	18'-11"	16'-6"
4"	18	400S162-43	20'-7"	18'-0"
4"	16	400S162-54	22'-1"	19'-3"
4"	14	400S162-68	23'-8"	20'-8"
6"	20	600S162-33	26'-0"	22'-8"
6"	18	600S162-43	28'-4"	24'-9"
6"	16	600S162-54	30'-4"	26'-6"
6"	14	600S162-68	32'-7"	28'-5"
8"	18	800S162-43	35'-8"	31'-1"
8"	16	800S162-54	38'-4"	33'-6"
8"	14	800S162-68	41'-1"	35'-11"

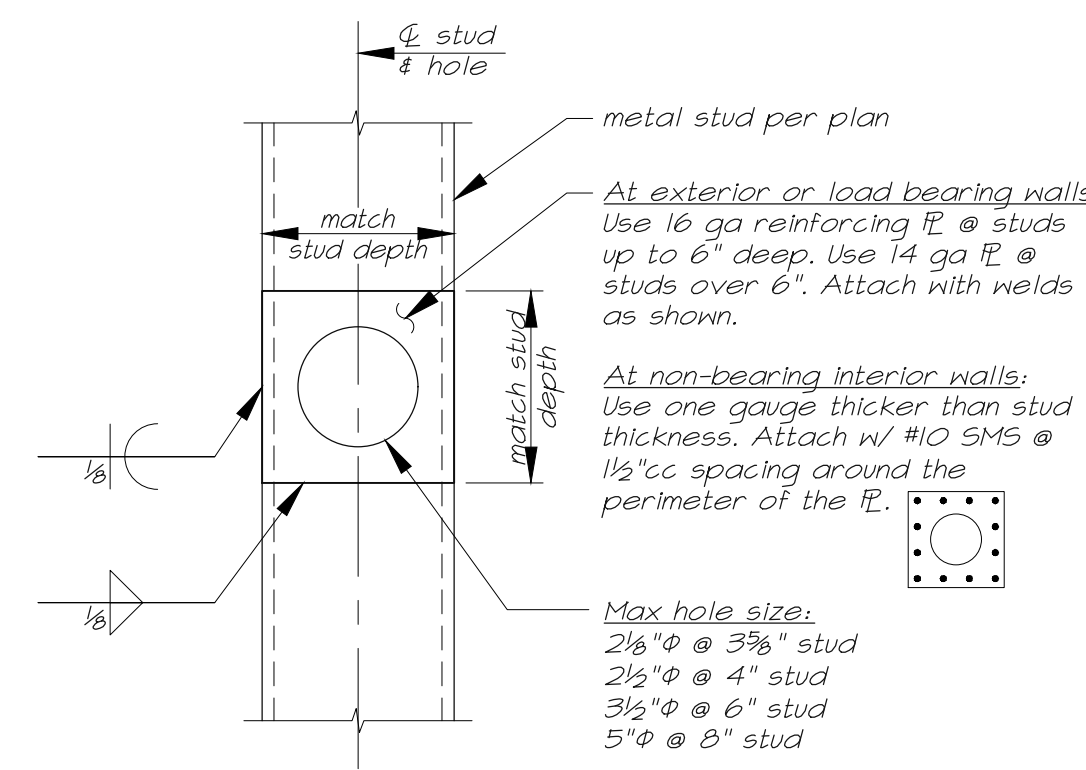
- Notes:
- Studs shall be depth as indicated on Arch drawings and gauge as determined by height of wall and the schedule above.
  - For typical wall framing conditions, see elevation 7
  - Designation conforms to Steel Stud Manufacturers Association standards.

Interior Metal Stud Partitions

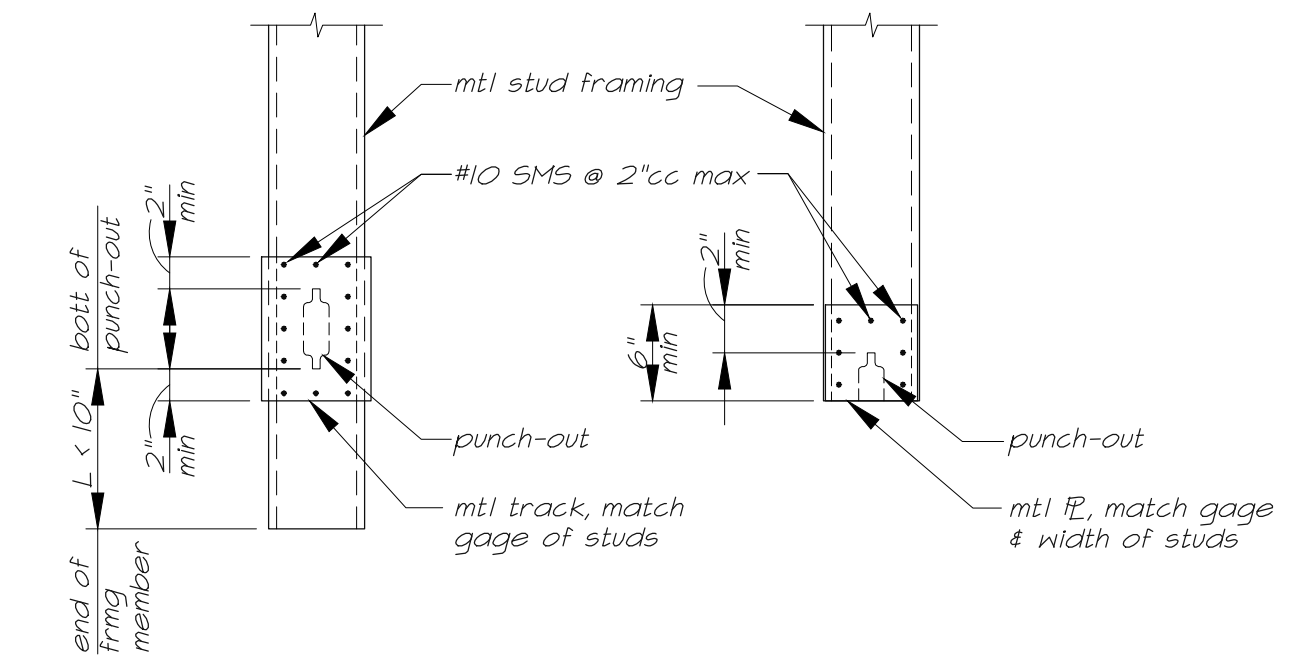
Maximum Height For Metal Studs with Deflection Limit L/240 - Non Bearing



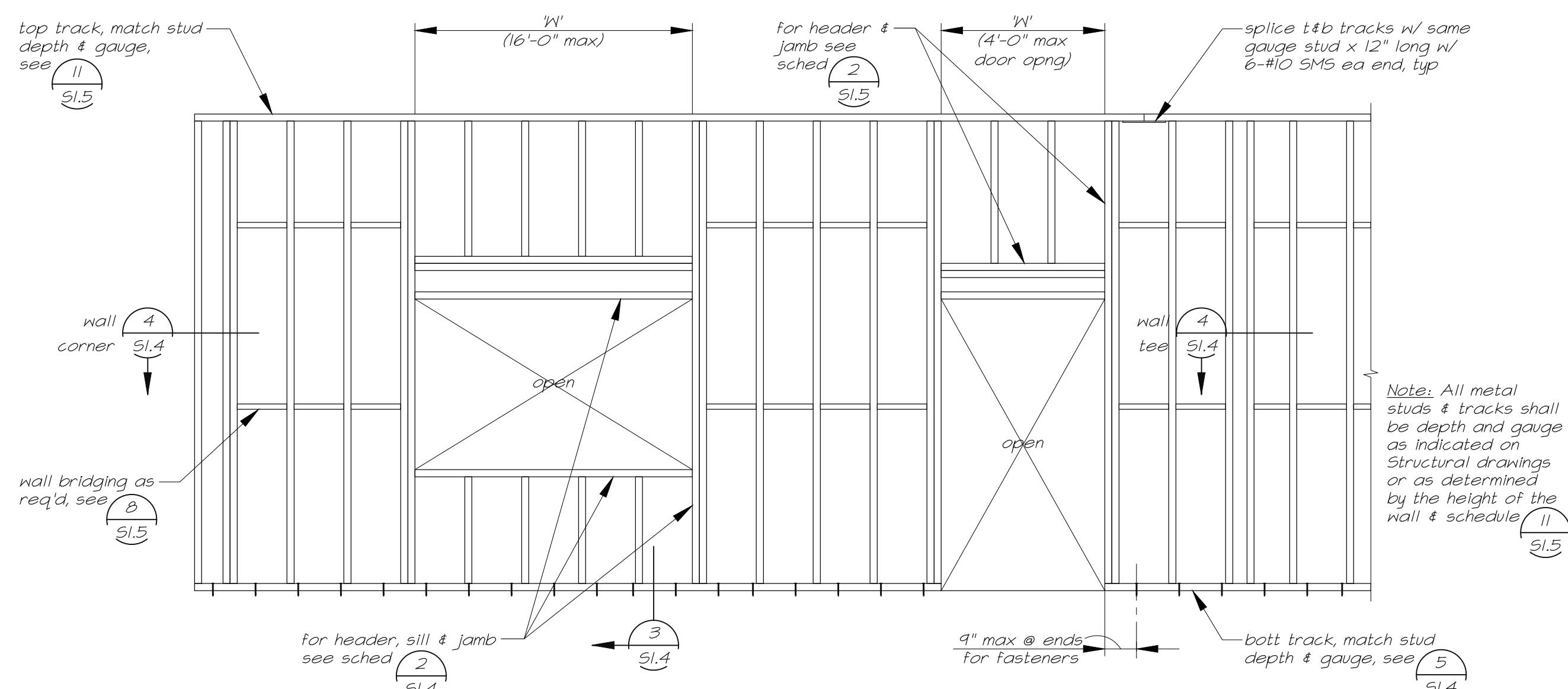
Typical Bridging @ Pipes/Conduits



Typical Penetration Through Metal Stud



Typ Stud Reinf @ Punch-Out



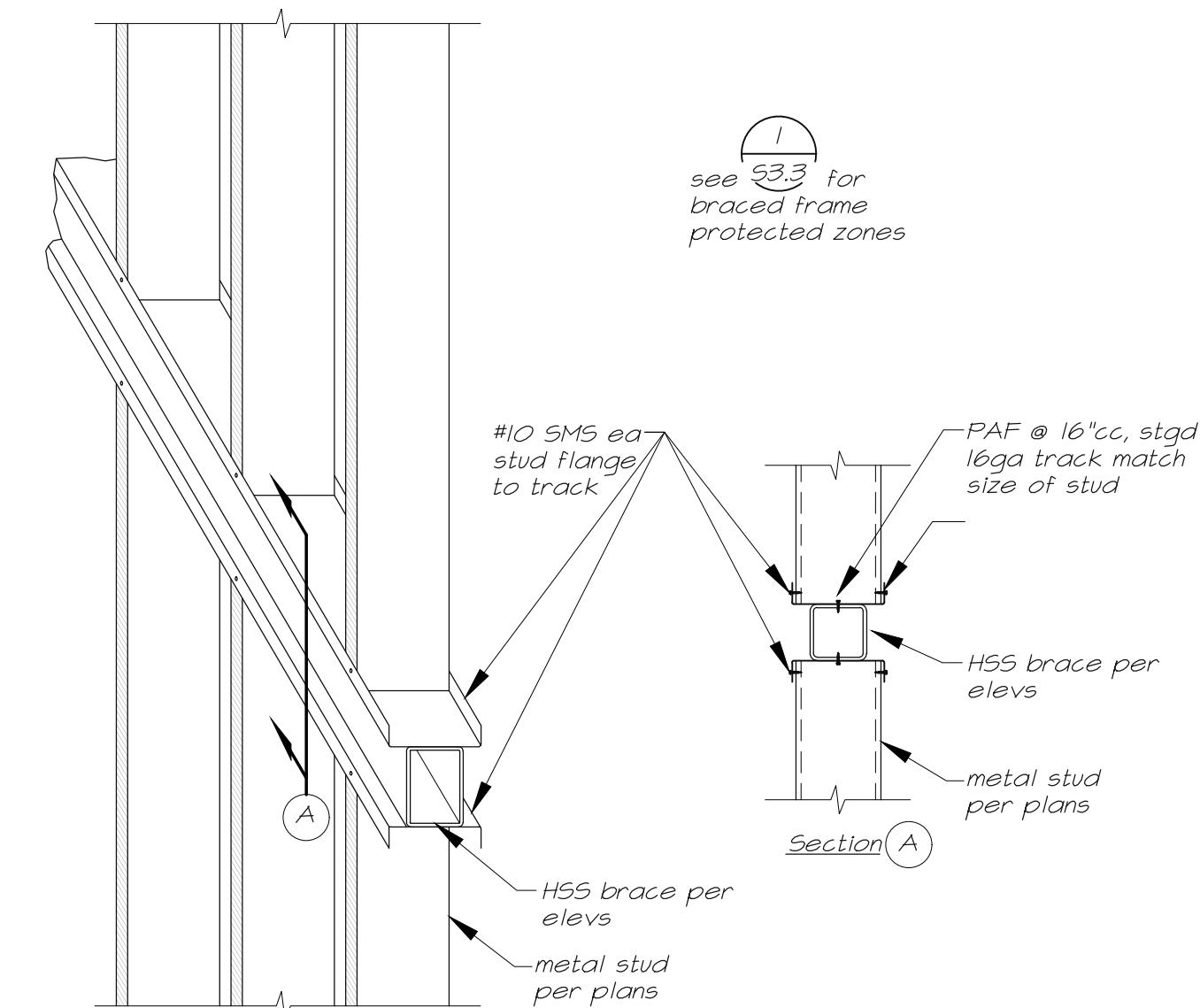
Typical Non-Bearing Partition Wall Framing Elevation

W' Opening width max (or max width bwn diag braces)	Required jamb stud	typ	Required hdr section	Required sill section
6'-0" max	dbl 800S137-43	dbl 800S137-43 w/ 800T125-43 1 1/2b	800T150-43	800T150-43
9'-0" max	dbl 800S137-43	dbl 800S162-43 w/ 800T125-43 1 1/2b	n/a	n/a
12'-0" max	dbl 800S162-43 (all H554x4x4)	dbl 800S162-43 w/ 800T125-68 1 1/2b	n/a	n/a
over 12'-0" (see note #5)	dbl 800S137-43	dbl 800S137-43 w/ 800T125-43 1 1/2b	800T150-43	800T150-43

Schedule Notes:

- The requirements of this schedule shall govern unless specifically detailed or noted otherwise.
- Metal stud section properties shall conform to the Steel Stud Manufacturer's Association Product Catalog (SSMA) as specified in the Lightgauge Steel notes.
- All header members shall be un-punched.
- At interior non-bearing conditions, track width @ headers and sills shall match depth of studs as specified in the architectural drawings. Flange length & ga shall match this table.
- At spans over 18'-0", provide diagonal bracing of headers/sills to structural framing @ 4'-0"cc max per sections. (W' = 4'-0" max @ these locations)
- Maximum width shown in table is either the width between the jamb studs or between diagonal bracing as shown in note #5 above (where diag bracing occurs).

Non-Bearing Wall Header Schedule



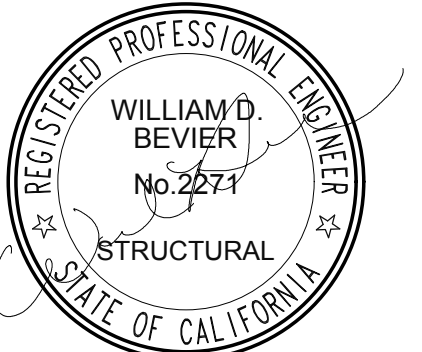
Studs @ Braced Frame



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95270  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net  
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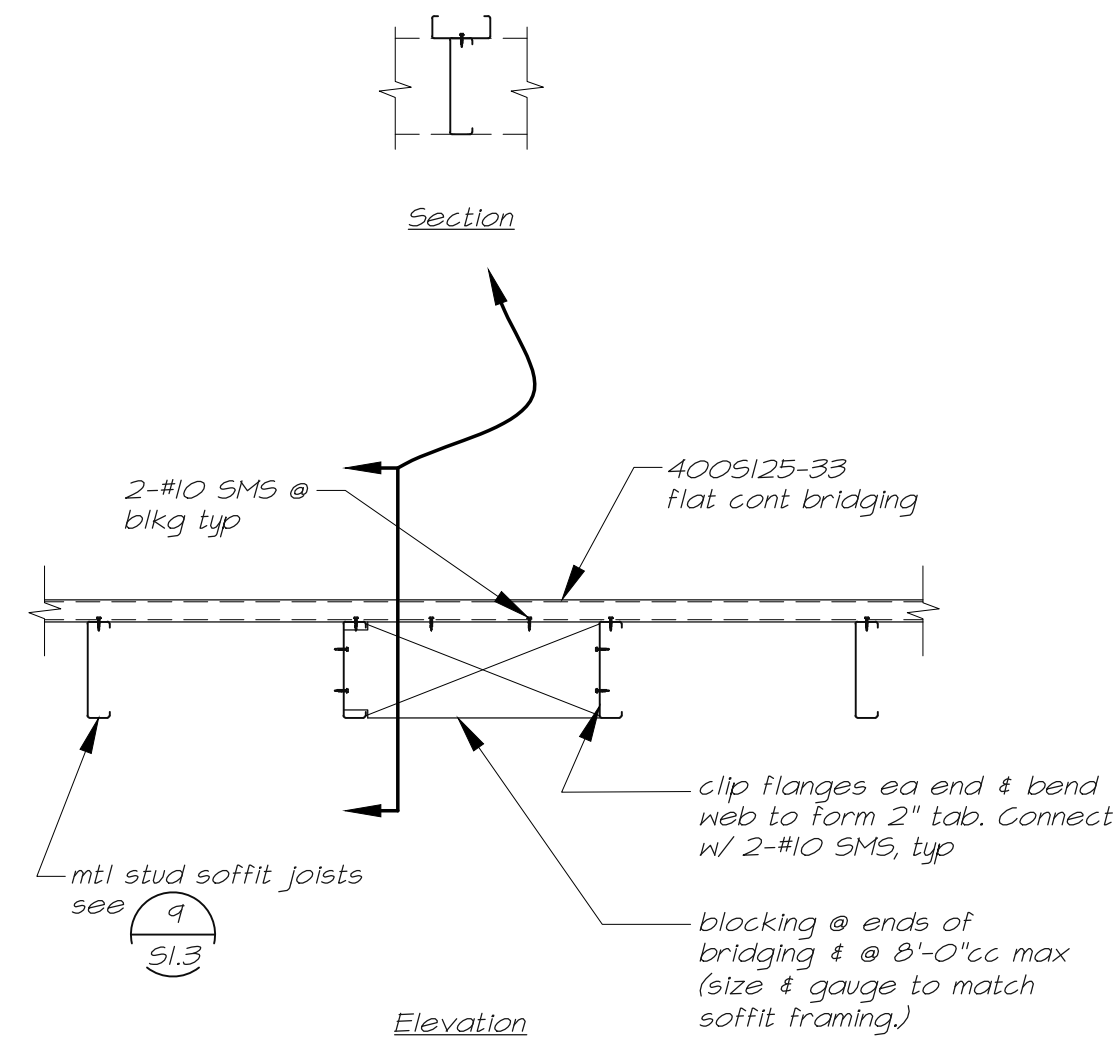
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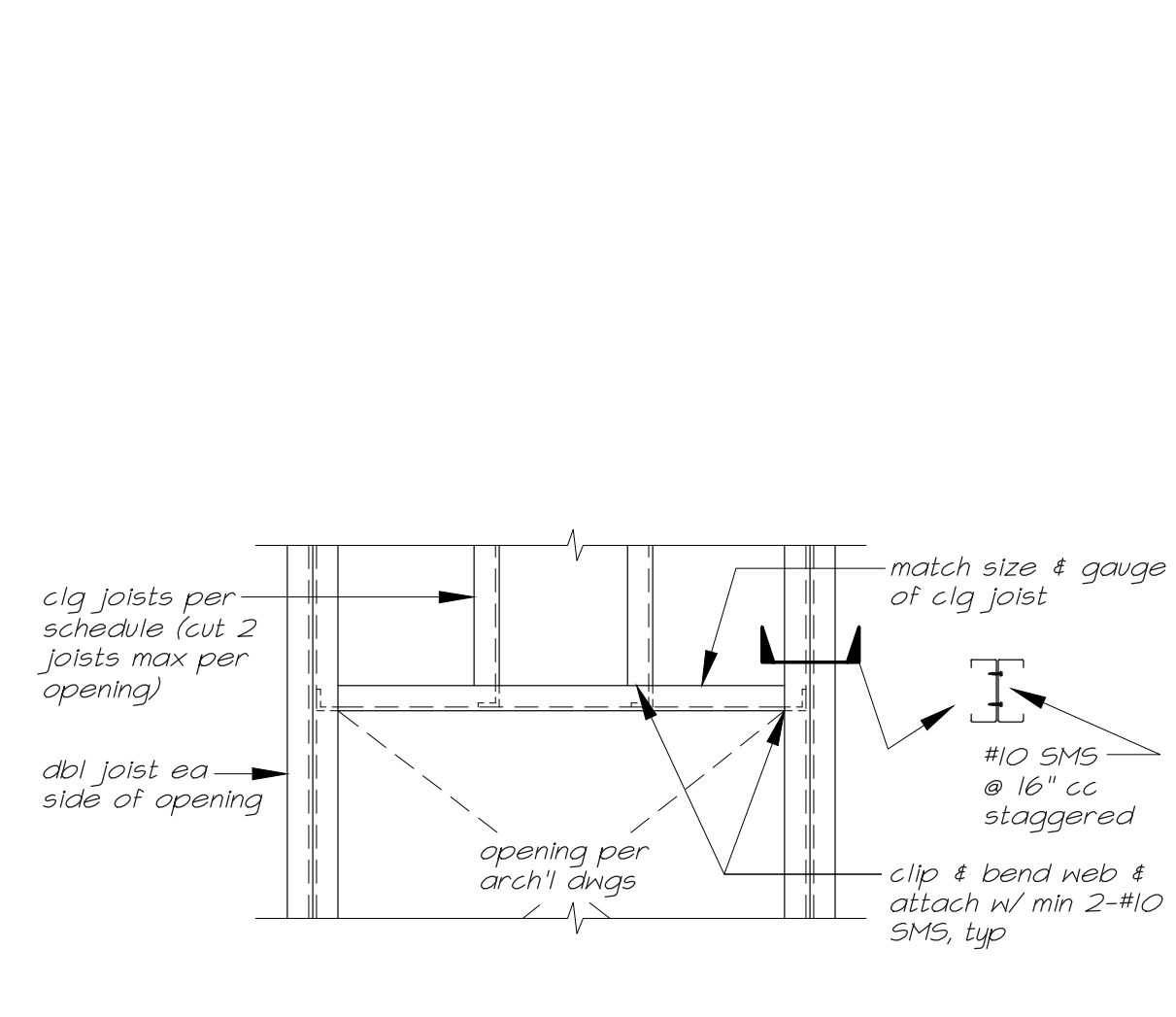
TYPICAL DETAILS

S1.5

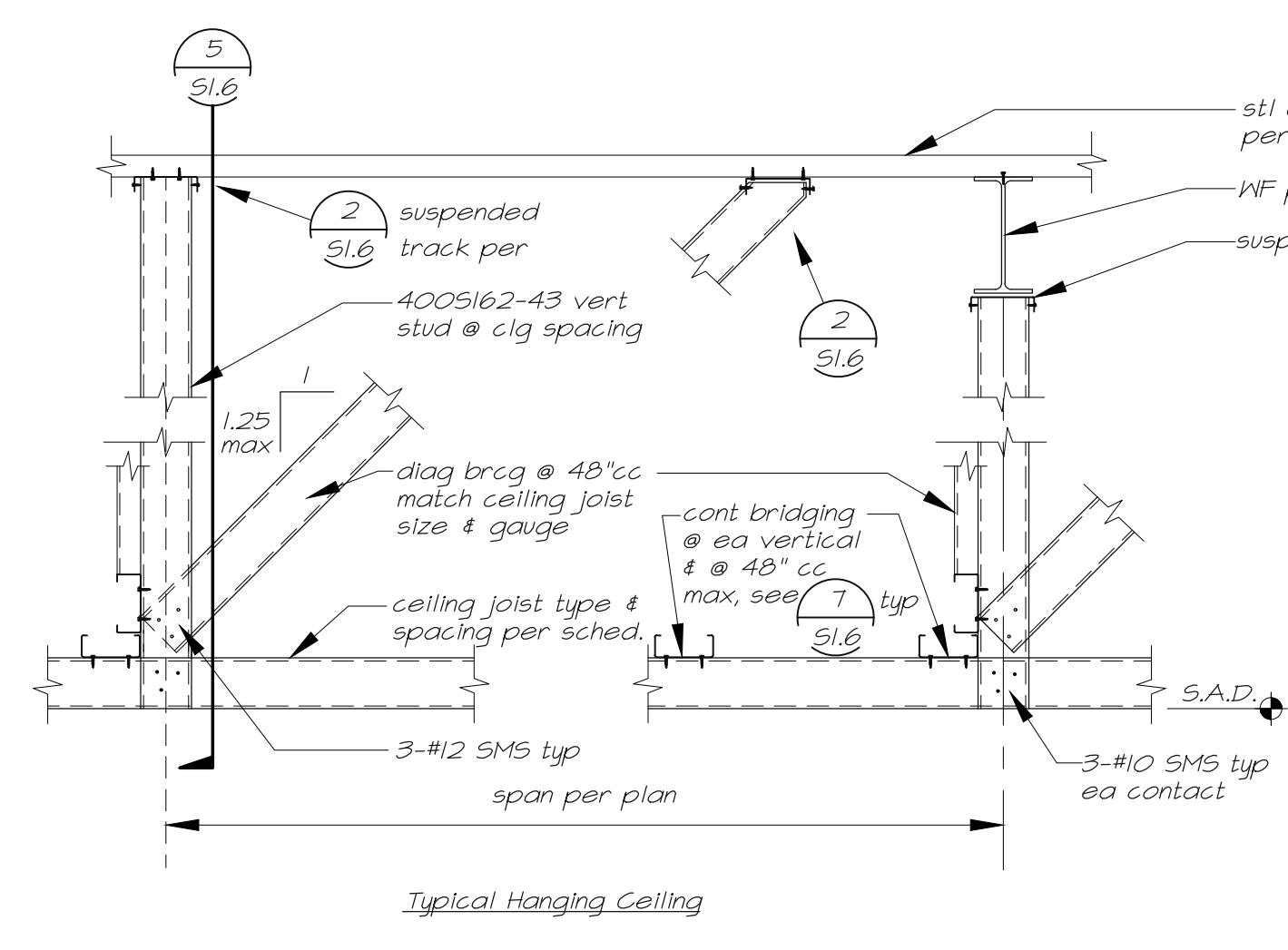




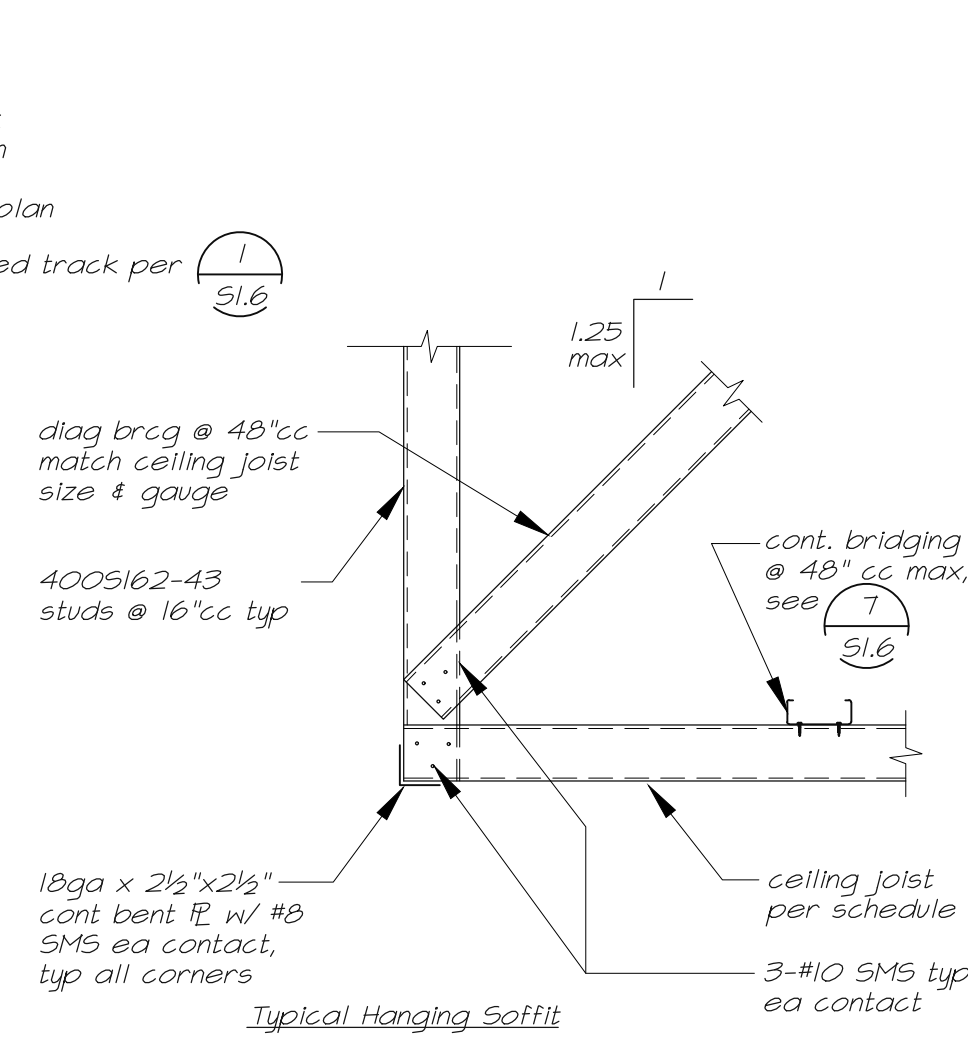
7 Detail  
1"=1'-0" Typ Soffit Joist Bridging



8 Typical Opening in Ceiling



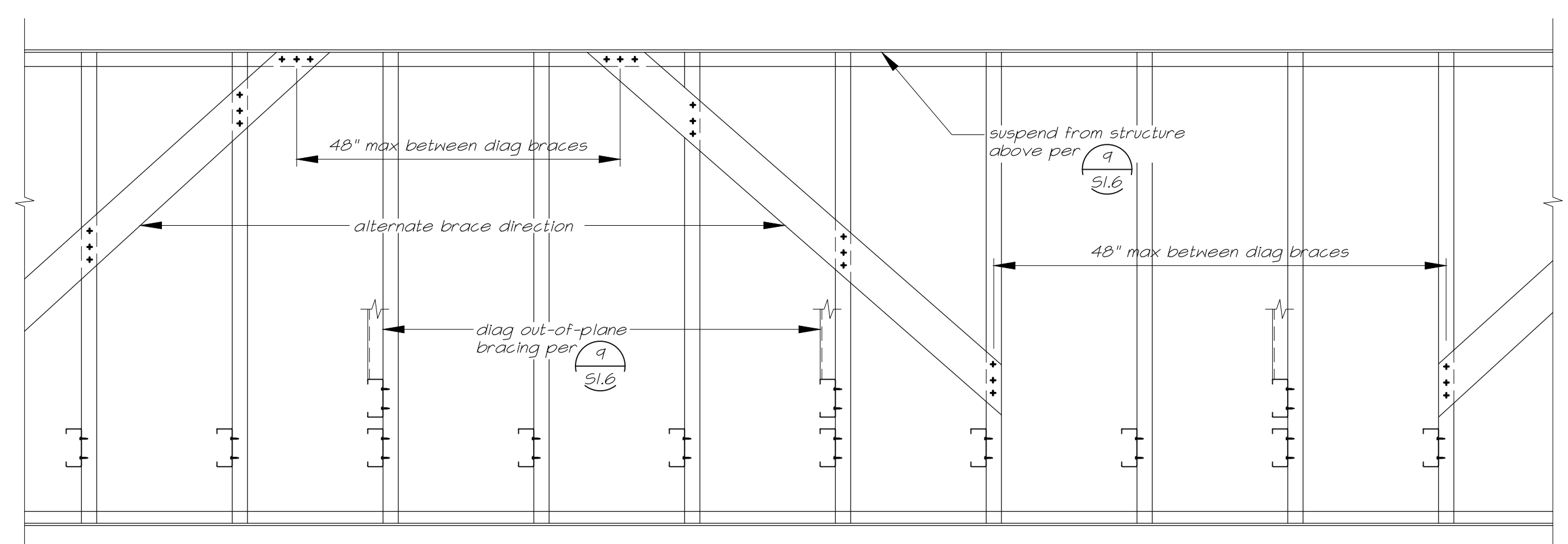
9 Typ. Framed Ceiling Details  
1"=1'-0" Note: S.A.D. for rated ceiling details



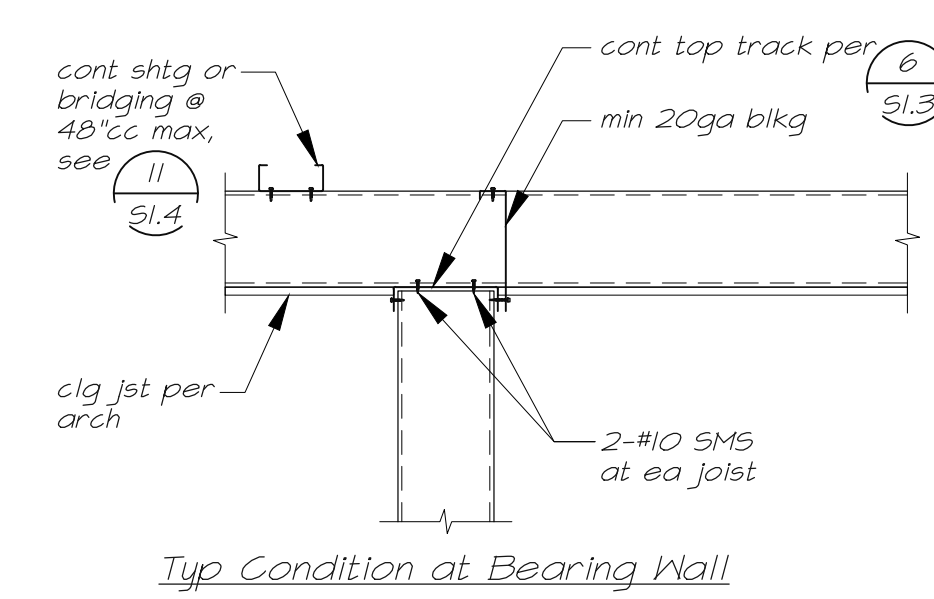
Typical Hanging Soffit

Span	Ceiling Joist
5'-0" max	400S125-33 @ 16"cc
8'-0" max	400S137-33 @ 16"cc
10'-0" max	400S137-43 @ 16"cc
12'-0" max	600S137-43 @ 24"cc
14'-0" max	600S137-43 @ 16"cc

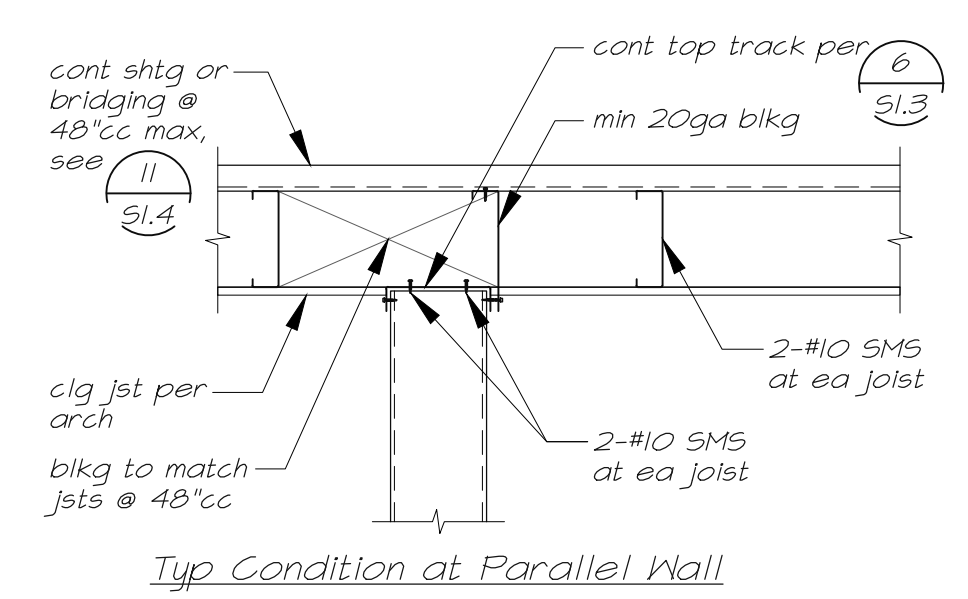
NOTE:  
1. Provide bridging @ 48" cc, see 7.  
2. Maximum load on ceiling joists shall be 15 psf DL + 10 psf LL.



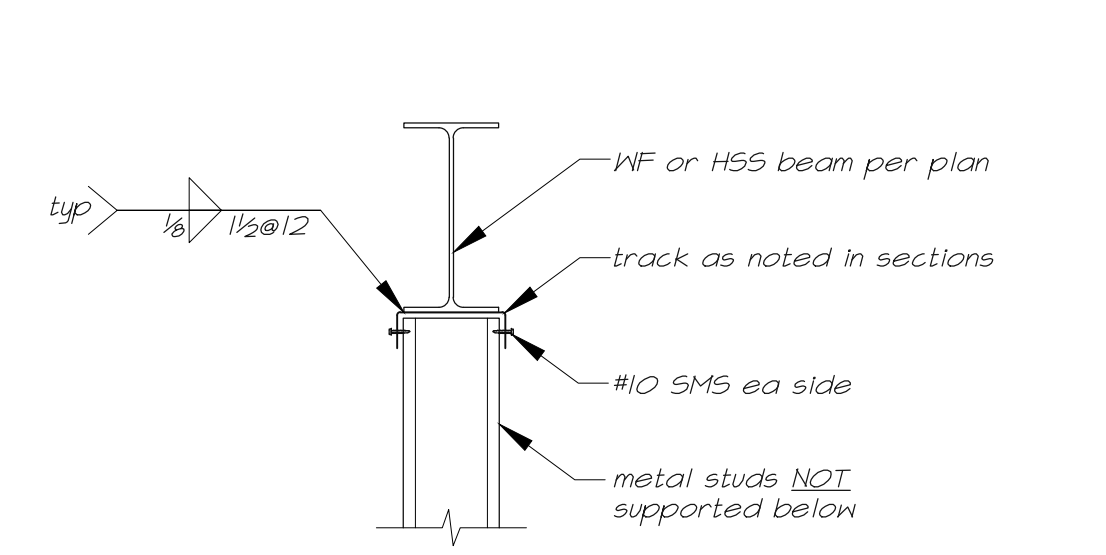
5 Typical Diagonal In-Plane Bracing @ Suspended Studs



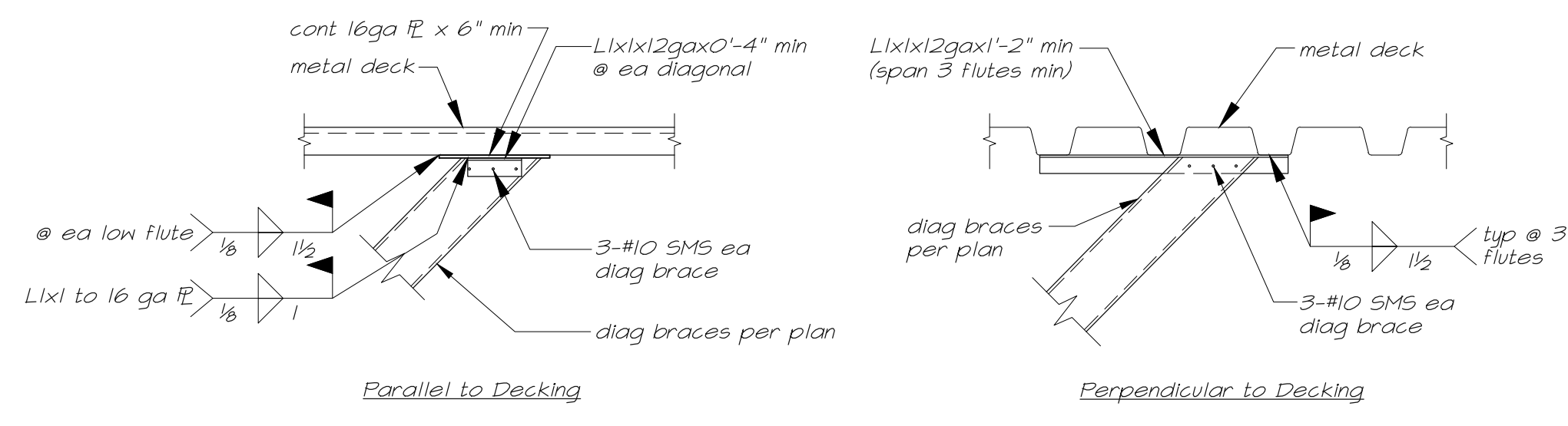
Typ Condition at Bearing Wall



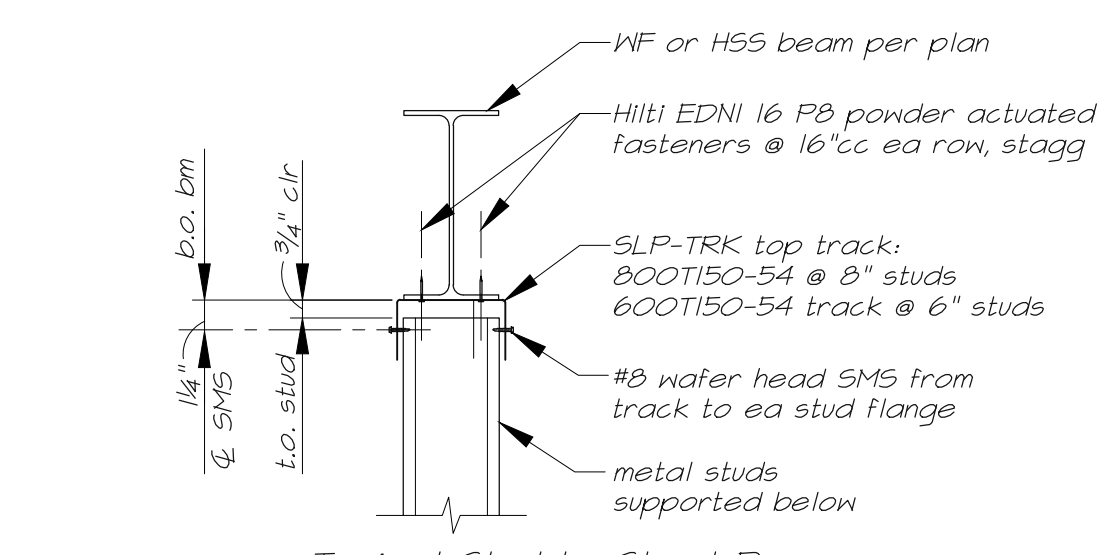
Typ Condition at Parallel Wall



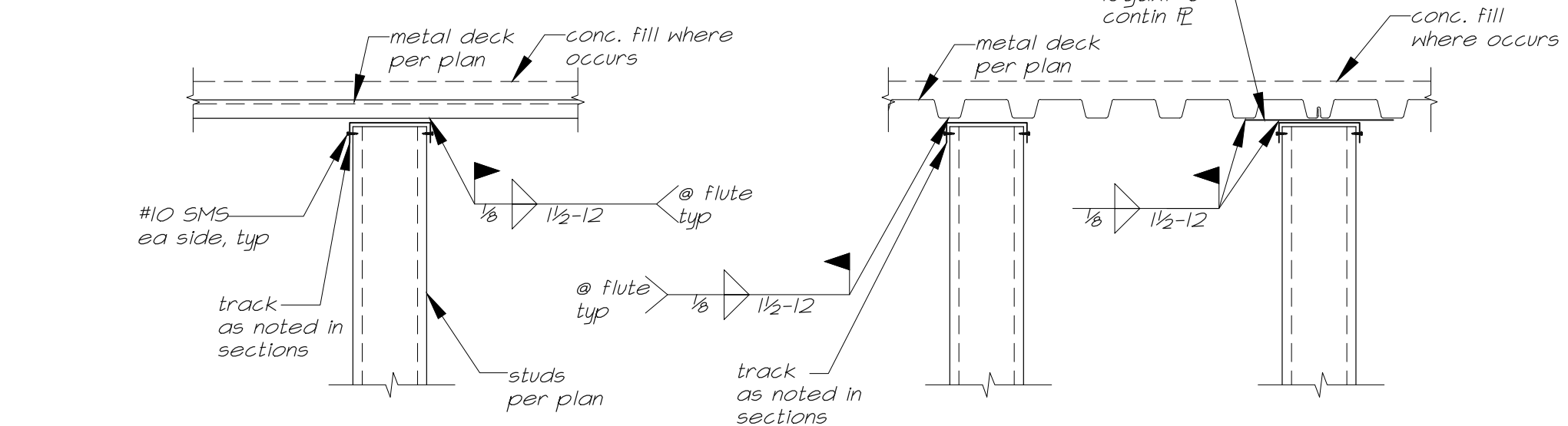
Typical Stud to Steel Beam Connection (Suspended)



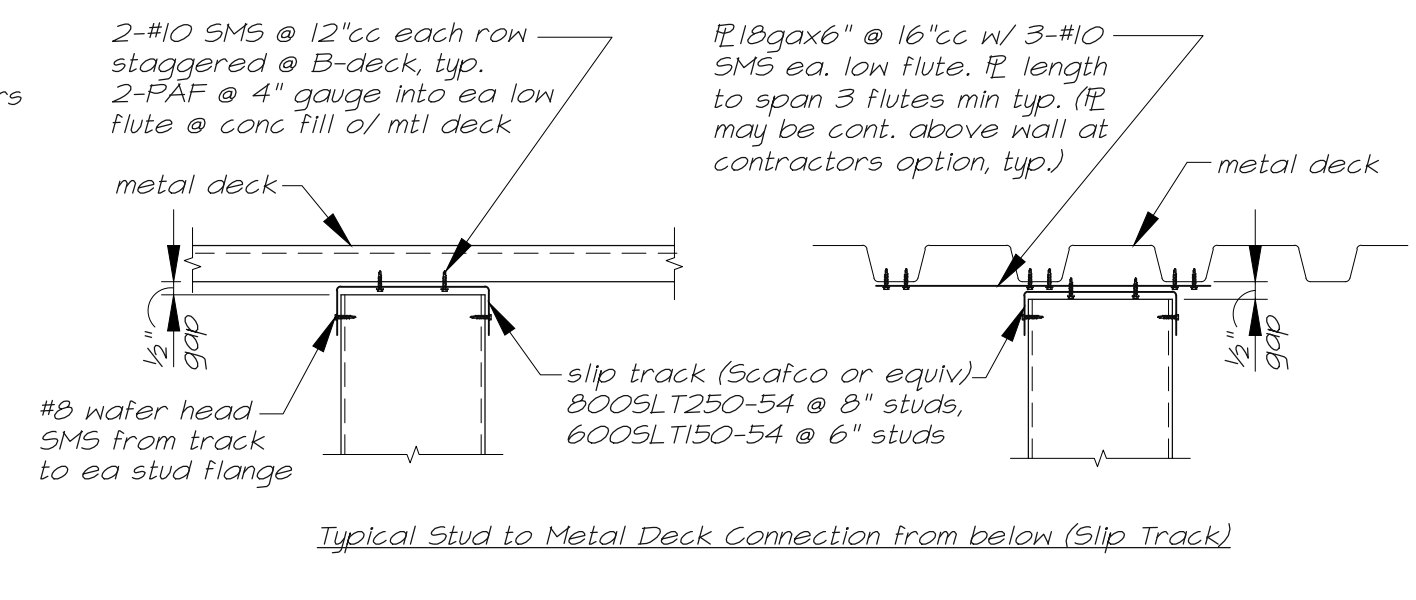
Typical Stud to Metal Deck Connection (Suspended)



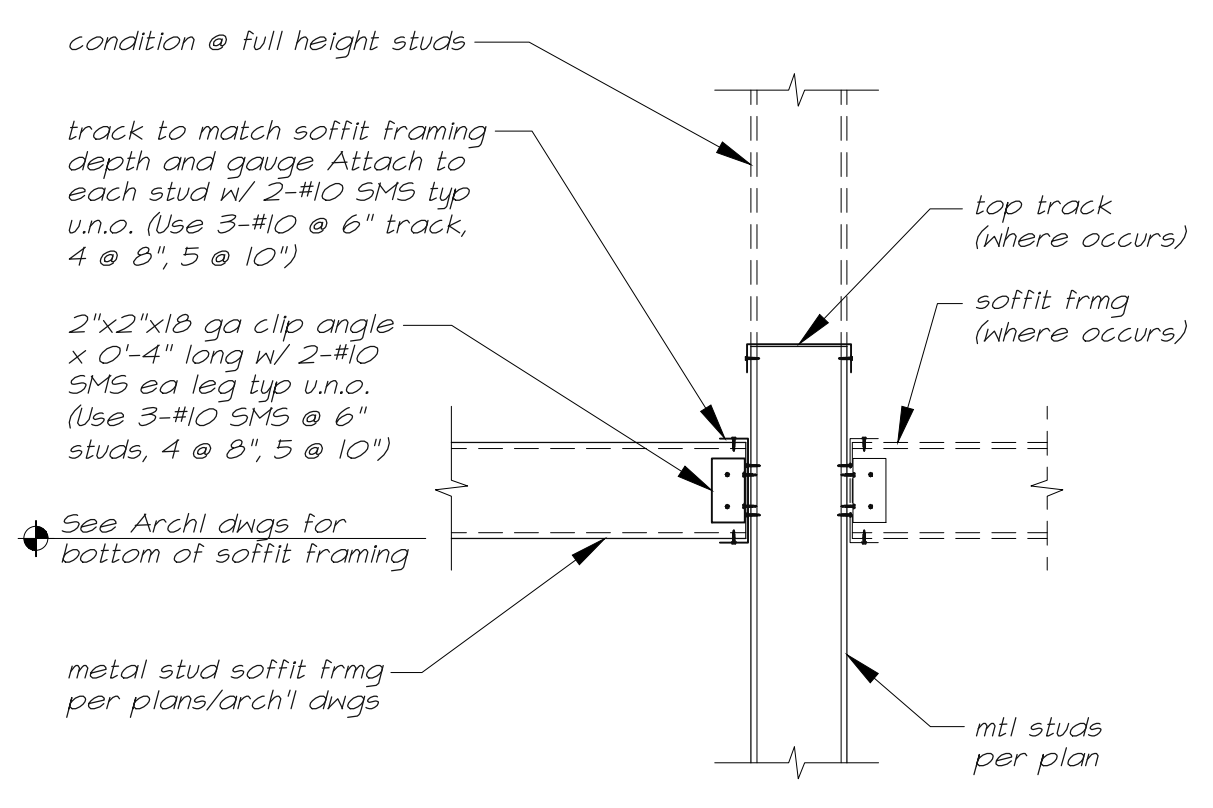
Typical Stud to Steel Beam Connection (Slip Track)



Typical Stud to Metal Deck Connection (Suspended)



Typical Stud to Metal Deck Connection from below (Slip Track)



4 Typical Joist to Stud

1 Detail  
stud to beam

2 Detail  
1"=1'-0"

3 Detail  
1 1/2"=1'-0" Intr condition



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net  
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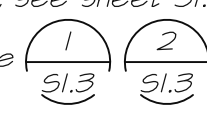
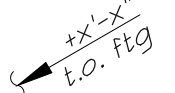

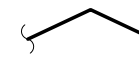
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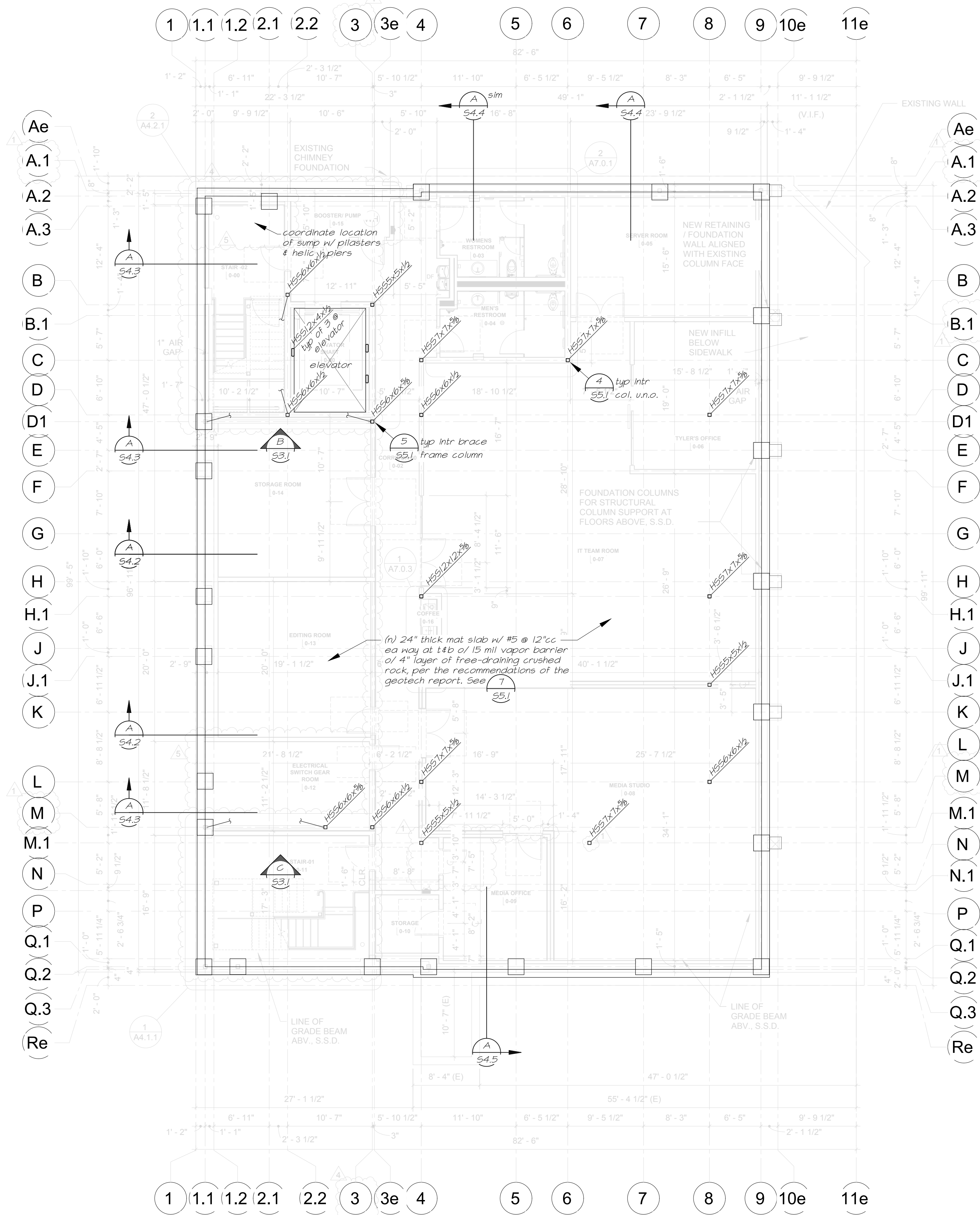
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TYPICAL DETAILS



**Foundation Notes**

1. Site preparation and building pad construction shall be done in accordance with the recommendations in the Geotechnical Engineering Report by Mid Pacific Engineering, Inc. (MPE), MPE No. 0635T-01, dated April 18, 2024.
2. Verify all dimensions with architectural drawings. Notify Architect immediately of any discrepancies for resolution prior to proceeding.
3. Dimensions are to face of concrete (FC) or column centerlines, typical u.n.o.
4. Spread Footings are centered on columns and braced frames, typical u.n.o.
5. Basement top of concrete slab = elevation -11'-0" with respect to reference first floor t.o.c. elevation +0'-0".
6. HSS column base t.o.c. elevation = -11'-10" below reference first floor elevation +0'-0" typical u.n.o.
7. Slab block-outs shown at columns are diagrammatic. Actual size and configuration is to be determined by the contractor for constructability.
8. Provide 3" concrete cover minimum @ base of anchor bolts, and columns typical.
9. For Typical Framing details at exterior metal stud walls, see sheet 51.4.
10. For typical reinforcing bends and corner reinforcing, see .
11.  Indicates top of concrete elevation below reference elevation +0'-0"
12.  Indicates HSS column, size indicated on plan.
13.  Indicates braced frame location. See elevations and details referenced.



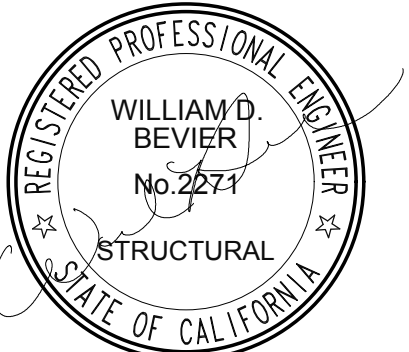
**Basement/Foundation Plan**  
S2.1 1/8"=1'-0"



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P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net  
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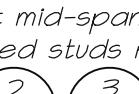
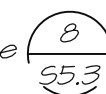
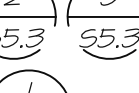
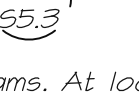

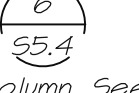
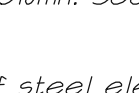
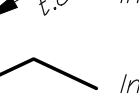
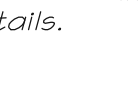
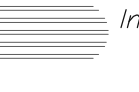
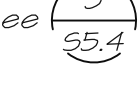
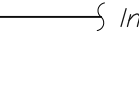

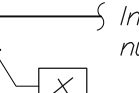
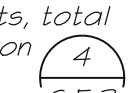
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NO.	DESCRIPTION

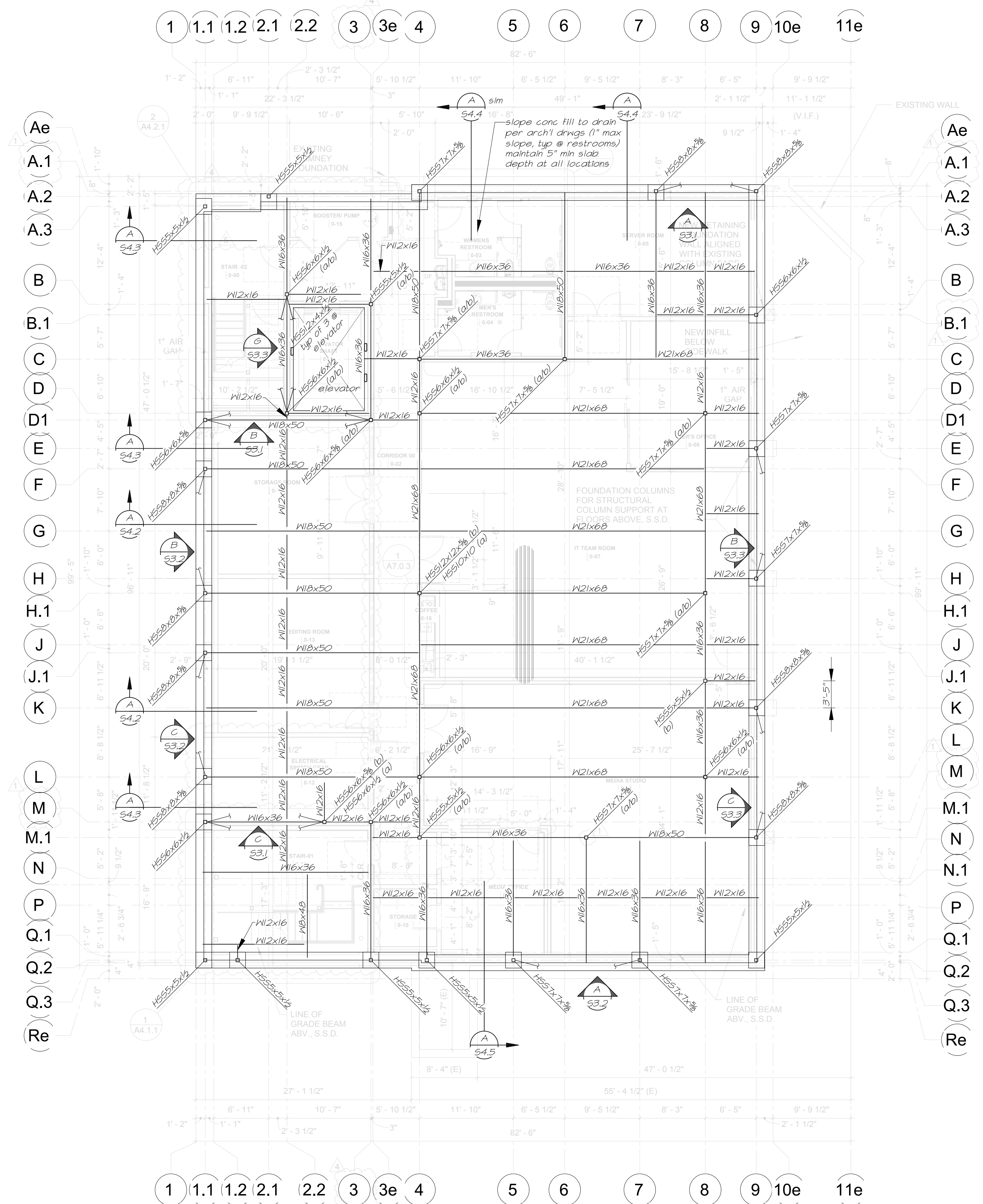
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DRAWN BY: TB, JRW, MC

**BASEMENT FOUNDATION PLAN**

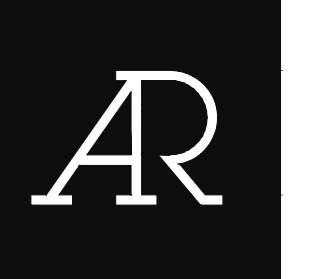


### Floor Framing Notes

- 3rd Floor: Top of slab elevation is +28'-0" above reference elevation +0'-0", typical u.n.o.  
Top of steel elevation is +27'-5 1/2" above reference elevation typical u.n.o.
- 2nd Floor: Top of slab elevation is +16'-0" above reference elevation +0'-0", typical u.n.o.  
Top of steel elevation is +15'-5 1/2" above reference elevation typical u.n.o.
- 1st Floor: Top of slab elevation is +0'-0" above reference elevation +0'-0", typical u.n.o. Top of steel elevation is -0'-6 1/2" with respect to reference elevation typical u.n.o.
- Dimensions are to centerline of steel or face of closure/angle at slab edge typical u.n.o.
- Verify all openings in floor with Architectural, Mechanical, and Electrical drawings. For openings at concrete fill over metal deck, see  and .
- For typical framing details at exterior metal stud walls, see sheet S1.4.
- C=3/4" indicates amount of camber required at mid-span of beam or girder.
- (34) indicates number of automatic end welded studs required at beam or girder see .
- For typical beam to beam connections, see  and .
- For typical beam to column connections, see .
- Metal deck must be attached to all steel beams. At locations where low flutes do not align with beam, split deck as in .
-  Indicates HSS column. See plans for size.
-  Indicates top of steel elevation above reference elevation +0'-0".
-  Indicates braced frame location. See braced frame elevations and referenced details.
-  Indicates span direction of metal deck. For metal deck types and typ details, see .
-  Indicates moment connection at end of beam, see .
-  Indicates beam connection requiring A325 SC Class A bolts, total number of bolts required is shown inside box, see note #1 on .

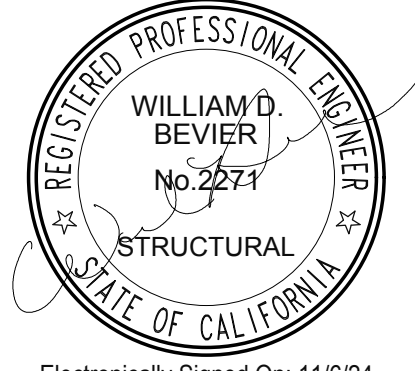


**First Floor Framing Plan**  
  
 1/8"=1'-0"



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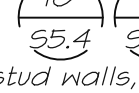

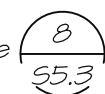
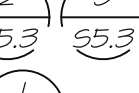

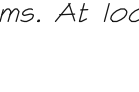





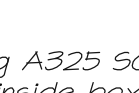




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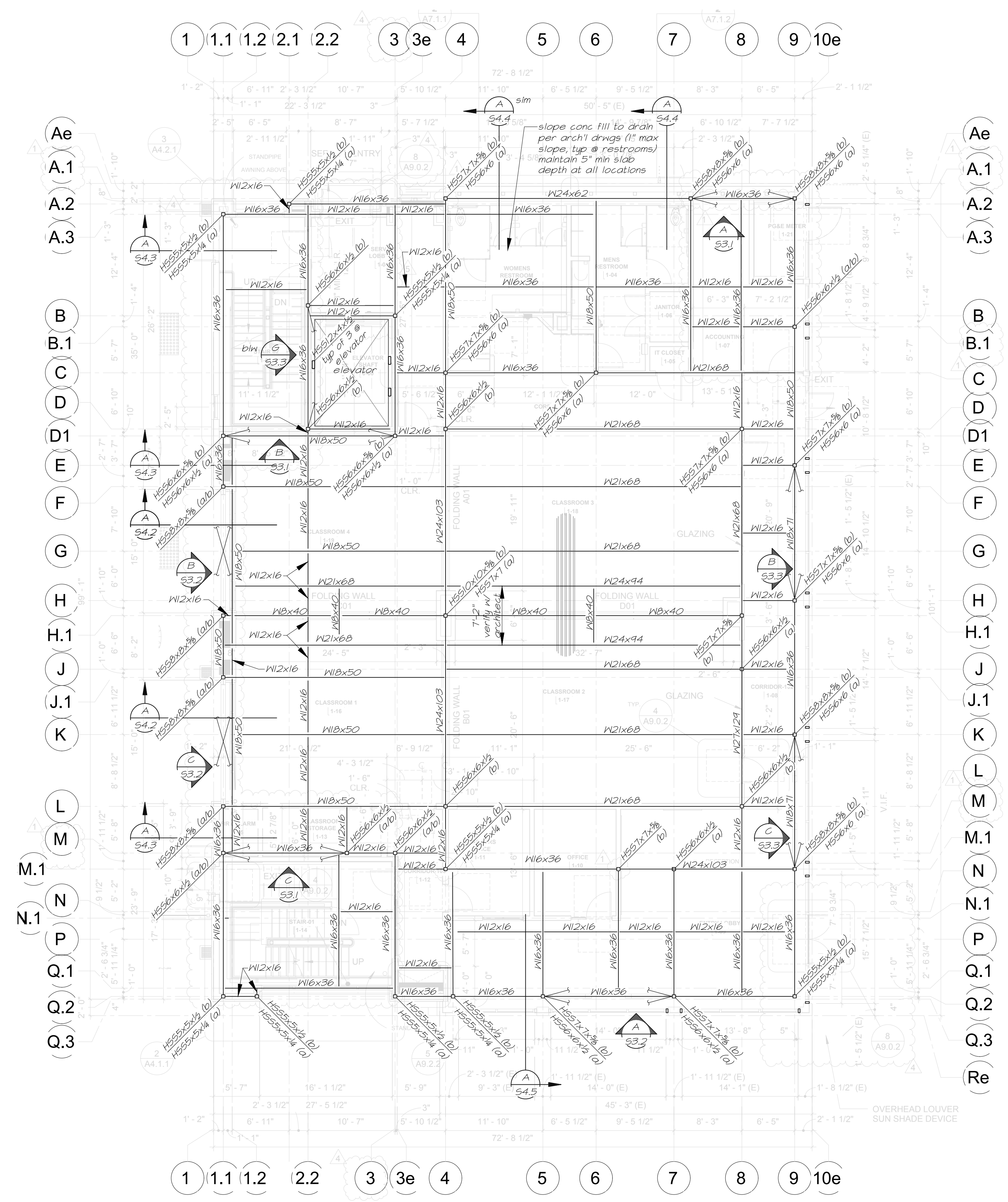
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**FIRST FLOOR FRAMING PLAN**



**Floor Framing Notes**

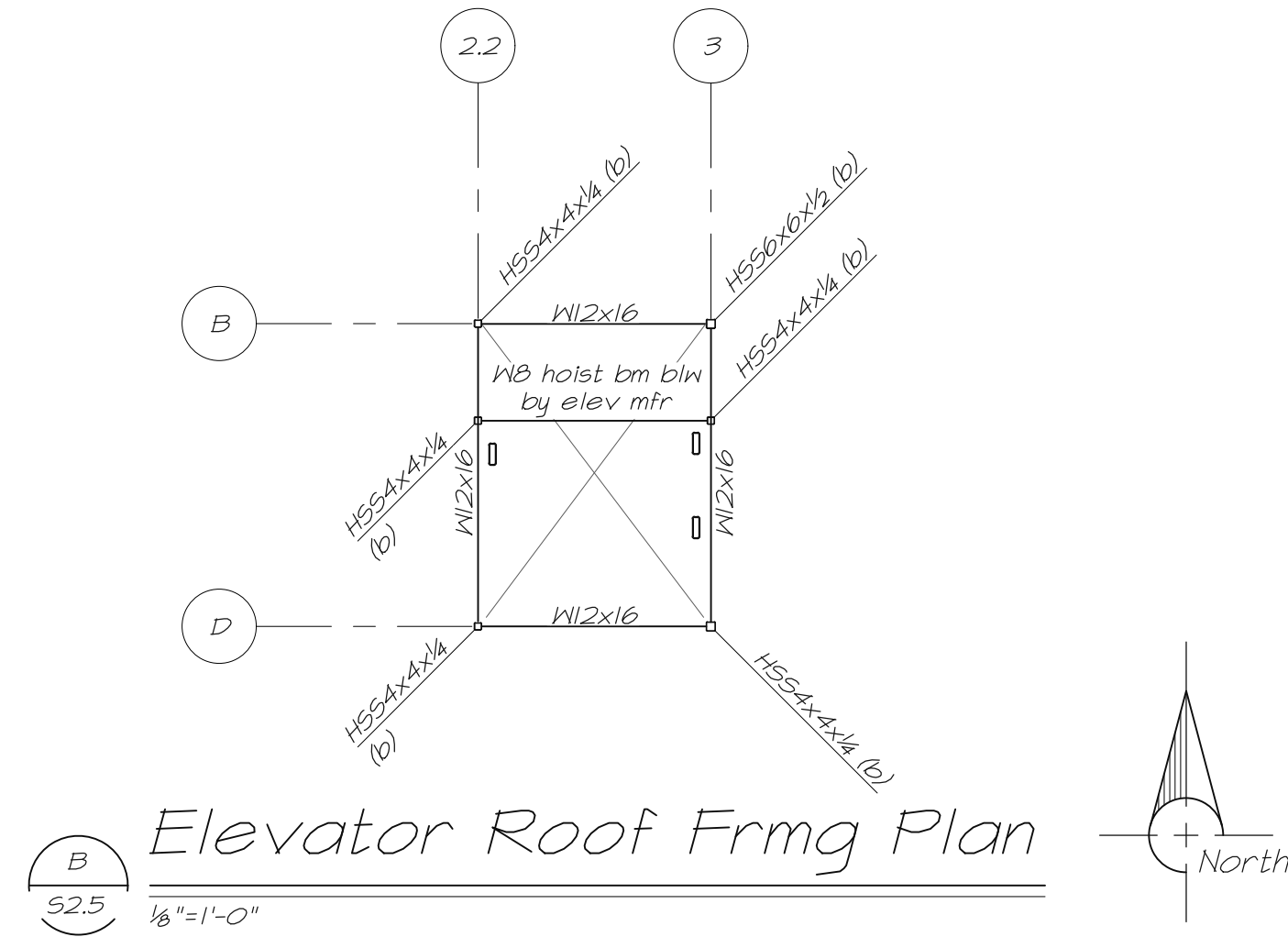
- 3rd Floor: Top of slab elevation is +28'-0" above reference elevation +0'-0", typical u.n.o.  
Top of steel elevation is +27'-5 1/2" above reference elevation typical u.n.o.
- 2nd Floor: Top of slab elevation is +16'-0" above reference elevation +0'-0", typical u.n.o.  
Top of steel elevation is +15'-5 1/2" above reference elevation typical u.n.o.
- 1st Floor: Top of slab elevation is +0'-0" above reference elevation +0'-0", typical u.n.o.  
Top of steel elevation is -0'-6 1/2" with respect to reference elevation typical u.n.o.
- Dimensions are to centerline of steel or face of closure/angle at slab edge typical u.n.o.
- Verify all openings in floor with Architectural, Mechanical, and Electrical drawings. For openings at concrete fill over metal deck, see  and .
- For typical framing details at exterior metal stud walls, see sheet S1.4.
- C=3/4" indicates amount of camber required at mid-span of beam or girder.
- (34) indicates number of automatic end welded studs required at beam or girder see .
- For typical beam to beam connections, see  and .
- For typical beam to column connections, see .
- Metal deck must be attached to all steel beams. At locations where low flutes do not align with beam, split deck as in .
-  Indicates HSS column. See plans for size.
-  Indicates top of steel elevation above reference elevation +0'-0".
-  Indicates braced frame location. See braced frame elevations and referenced details.
-  Indicates span direction of metal deck. For metal deck types and typ details, see .
-  Indicates moment connection at end of beam, see .
-  Indicates beam connection requiring A325 SC Class A bolts, total number of bolts required is shown inside box, see note #1 on .





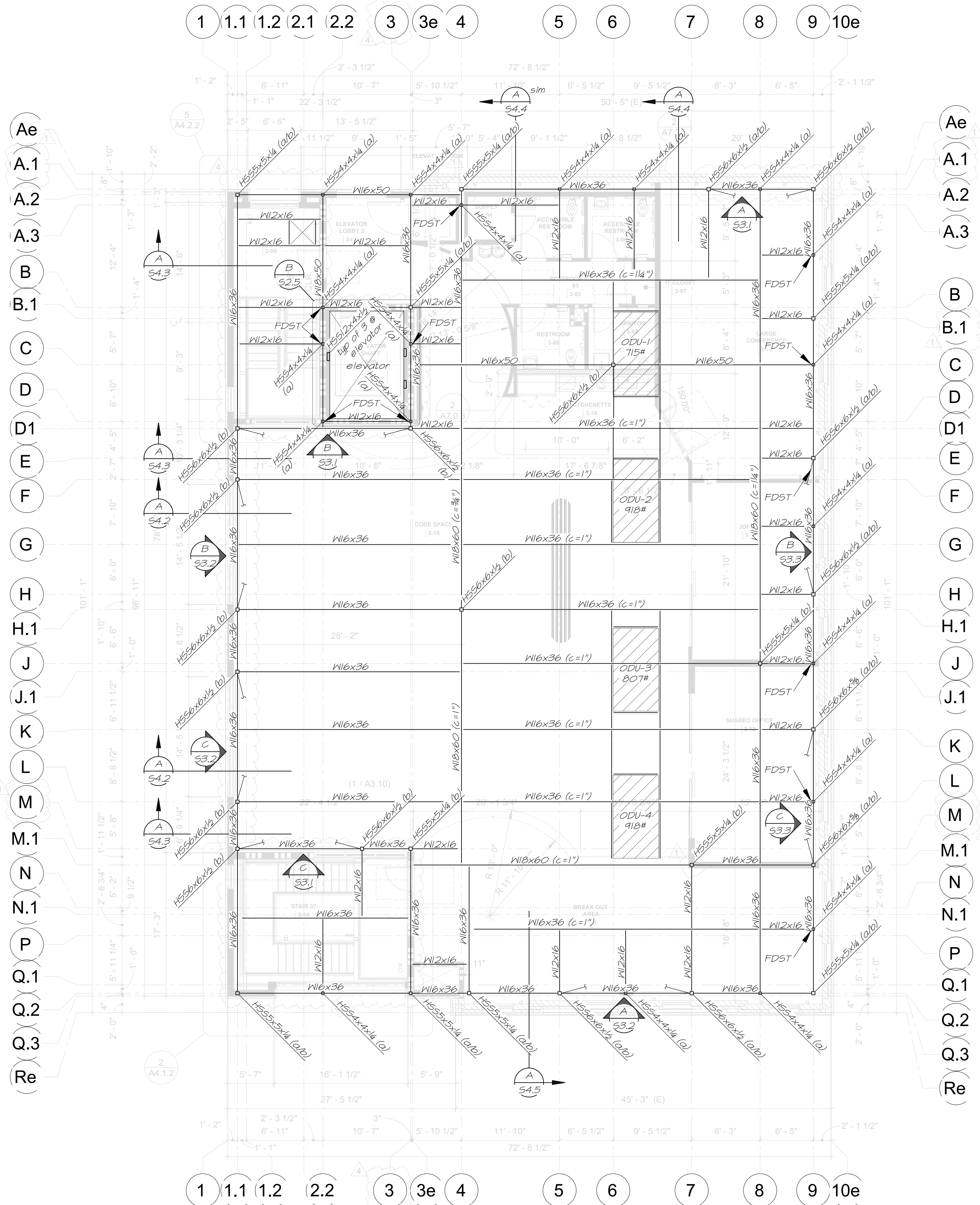






**Roof Framing Notes**

- Dimensions are to centerline of steel or edge of decking typical u.n.o.
- Verify all openings in roof with Architectural, Mechanical and Electrical drawings. For openings at metal deck, see 55.5
- For typical framing details at exterior metal stud walls, see sheet S1.4.
- C=3/4" indicates amount of camber required at mid-span of beam or girder.
- For typical beam to beam connections, see 55.3
- For typical beam to column connections, see 55.3
- Metal deck must be attached to all steel beams. At locations where low flutes do not align with beam, split deck as in 55.5
- Indicates HSS column. See Roof Framing Plan for sizes.
- ↗ 12°/0.9' Indicates top of steel elevation above reference elevation +0'-0"
- ∧ Indicates braced frame location. See braced frame elevations and details referenced.
- ▨ Indicates span direction of metal deck. For metal deck types and typical details, see 55.4
- Indicates moment connection at end of beam, see 55.4
- ↗ Indicates beam connection requiring A325 SC Class A bolts, total number of bolts required is shown inside box, see note #1 in 55.3
- XXX# Indicates approximate location & weight of mechanical unit. Coordinate location of unit and of beams supporting unit with mechanical drawings



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 Stockton, California 95207  
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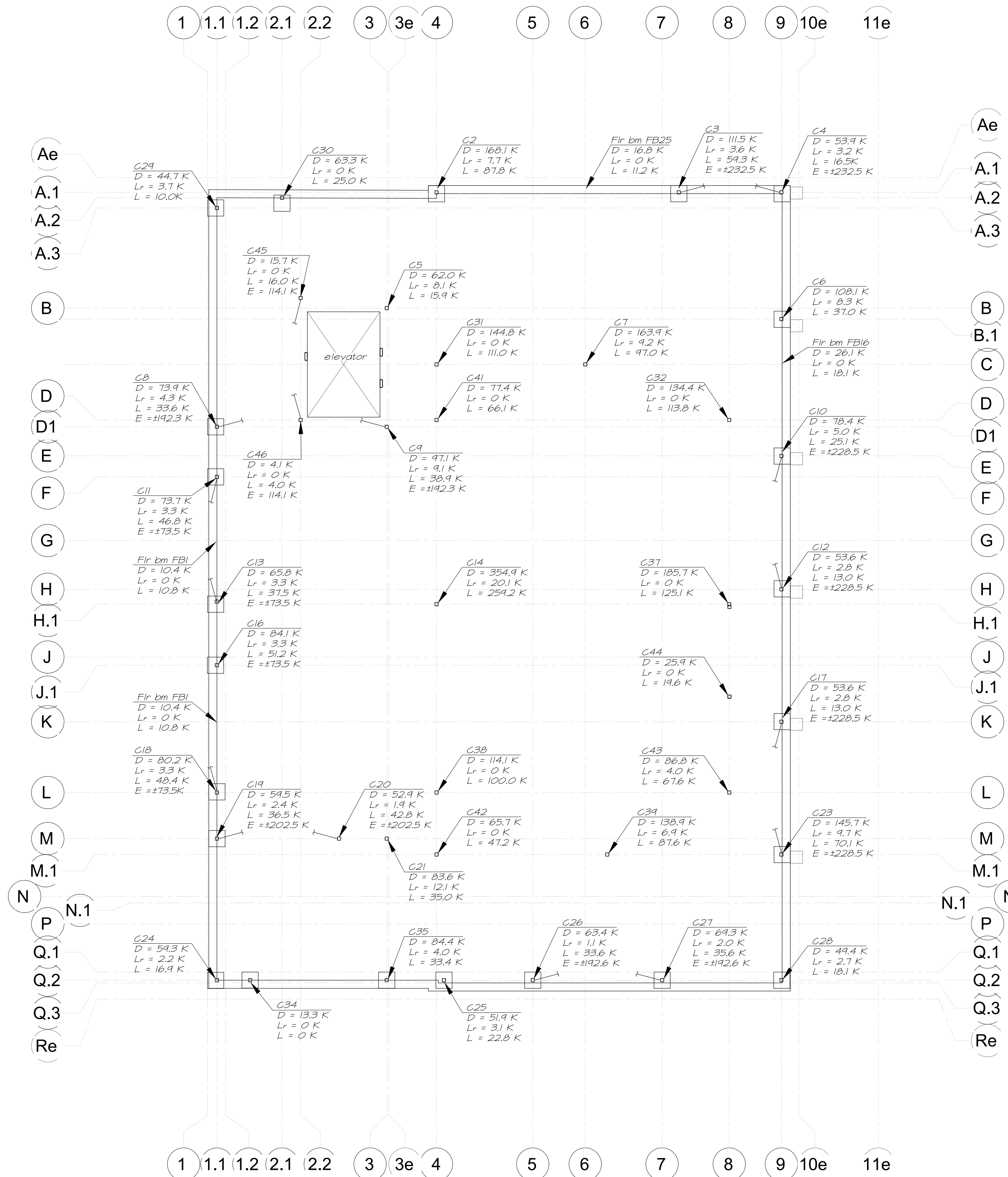
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**ROOF FRAMING PLAN**

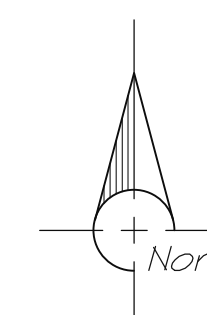




**ADDITIONAL LOADS AND NOTES:**

- Dead loads of (n) concrete pilasters (around perimeter), and (e) concrete pilasters at grid 10e are included in the loads noted on the plan.
- (n) 12" thick concrete retaining wall at basement at grid lines 1, 4, A, & G  
D n=1.50 kif
- (n) 24" sq concrete grade beam at top of (n) retaining wall at grid lines 1, 4, A, & G D n=0.60 kif
- (e) brick facade at grid lines Ae, Re, & 10e as noted on arch, drngs.  
Grids Ae&Re D n=5.94 kif  
Grid line 10e supported on (e) beams/pilasters/concrete walls - loads NOT included in (n) column loads
- (e) concrete wall below (e) brick facade at grid lines A&G (4 10e where occurs) D n=1.575 kif  
Grid line 10e brick facade supported on (e) beams/pilasters/concrete walls - beam and brick facade wall loads NOT included in (n) column loads.
- (e) footing below (e) conc walls D estimated n=6.0 kif
- Weight of (n) mat slab not included. Thickness = 24".
- Loads are unfactored:  
D = dead load  
L = floor live load  
Lr = roof live load  
E = seismic load

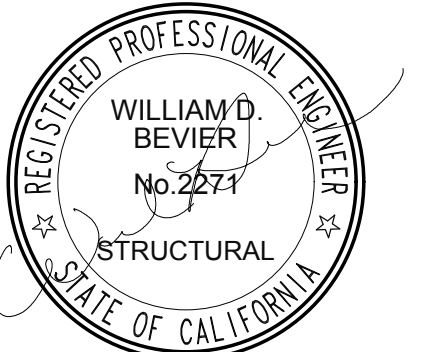
Col Load Summary at Foundation  
1/8"=1'-0"



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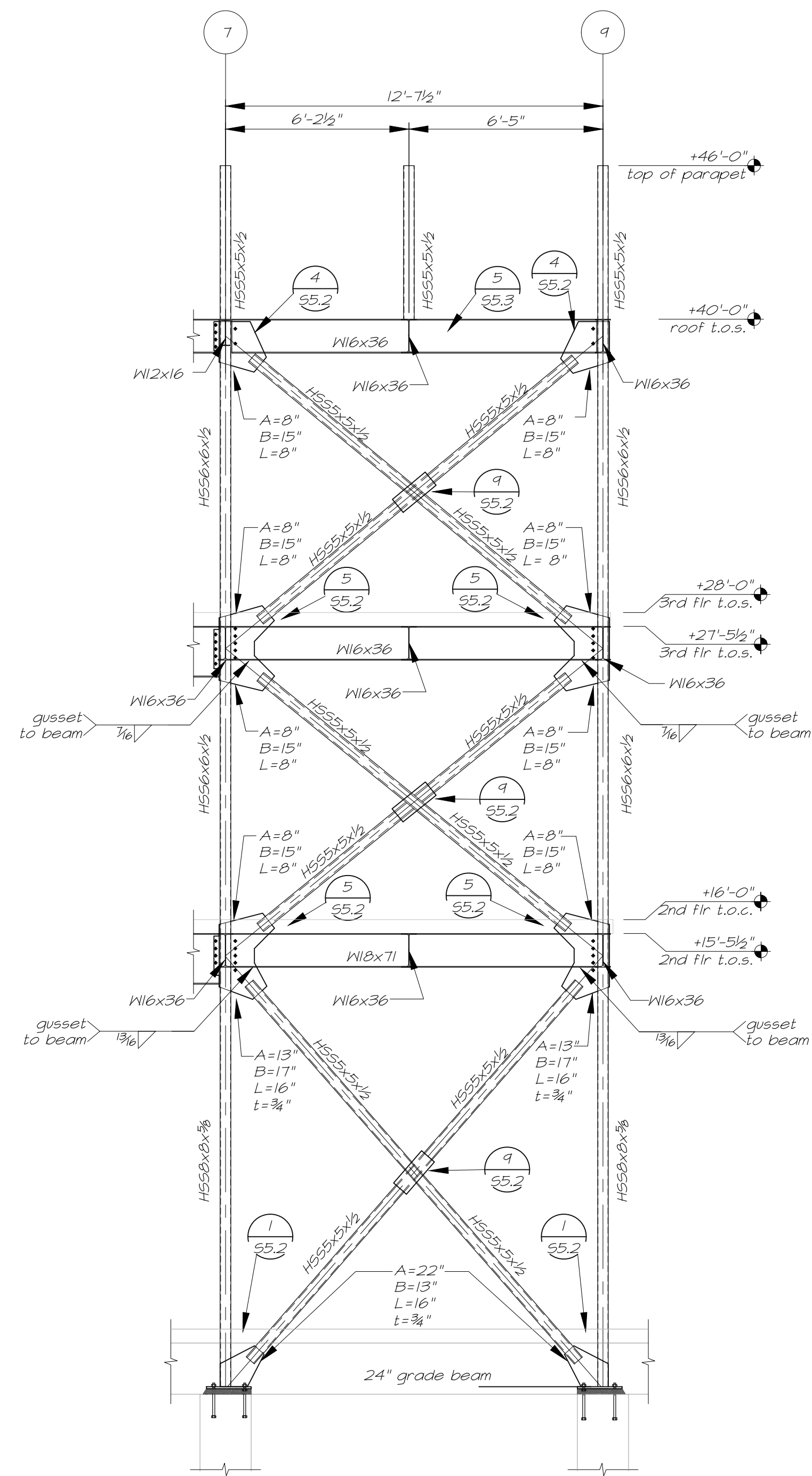
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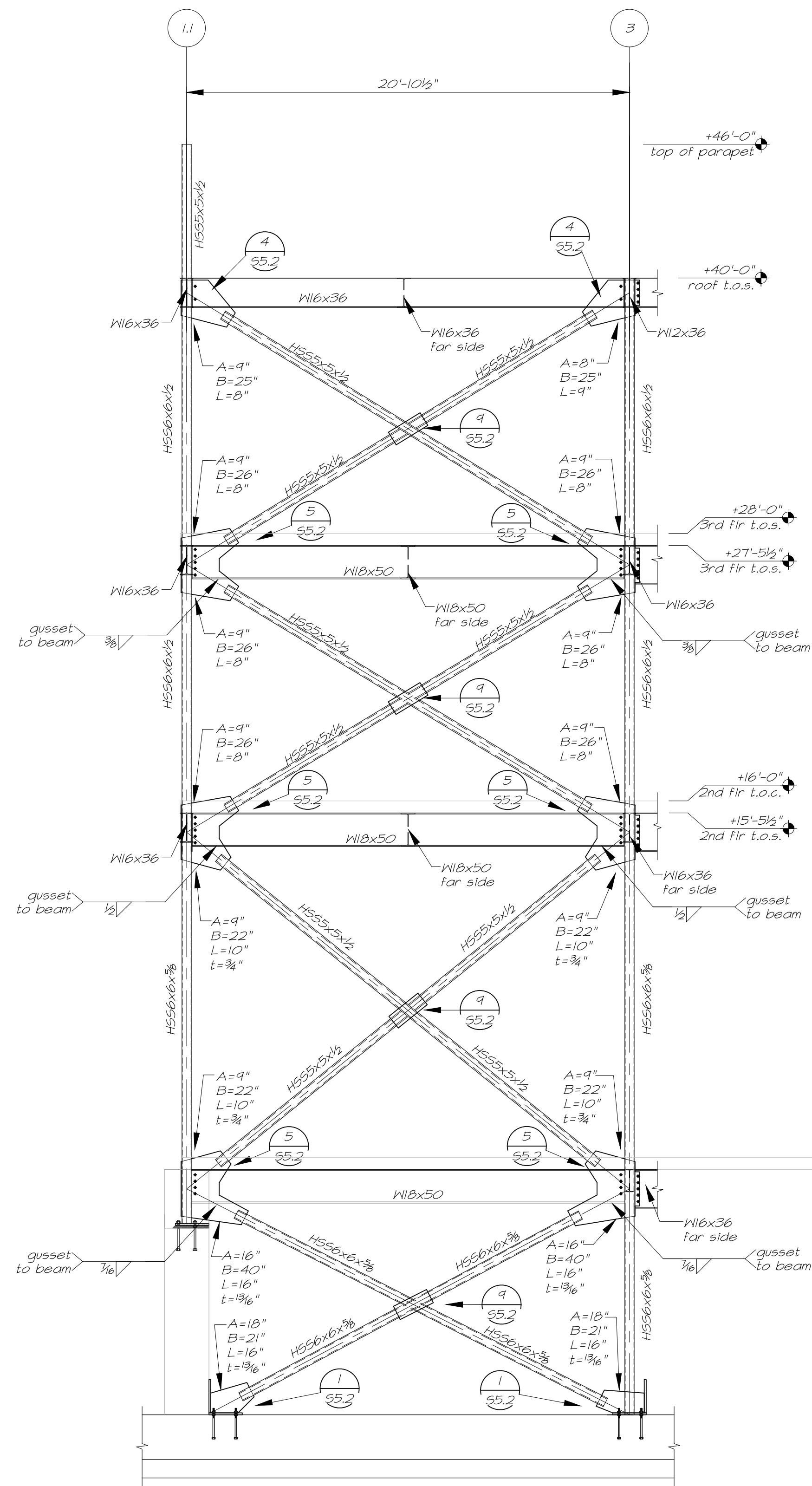
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**COLUMN LOAD  
SUMMARY AT  
FOUNDATION**

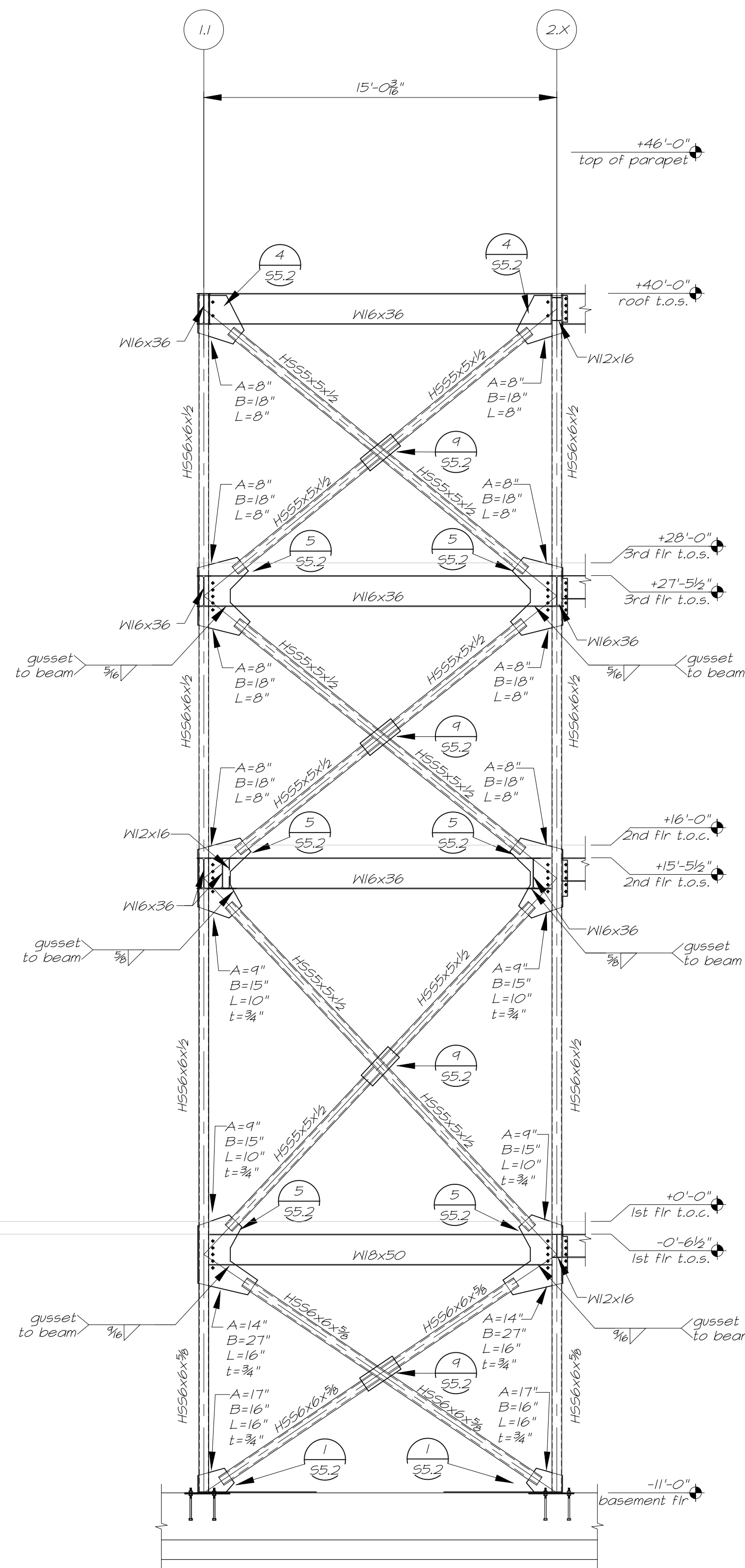




**A** Frame Line A.1 @ Grids 7-9  
 53.1 1/4"=1'-0"



**B** Frame Line D @ Grids 1.1-3  
 53.1 1/4"=1'-0"



**C** Frame Line M @ Grids 1.1-2.X  
 53.1 1/4"=1'-0"



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**FRAME ELEVATIONS**

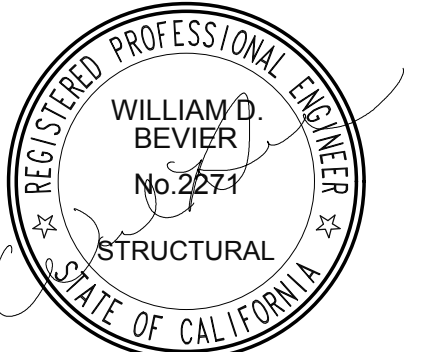




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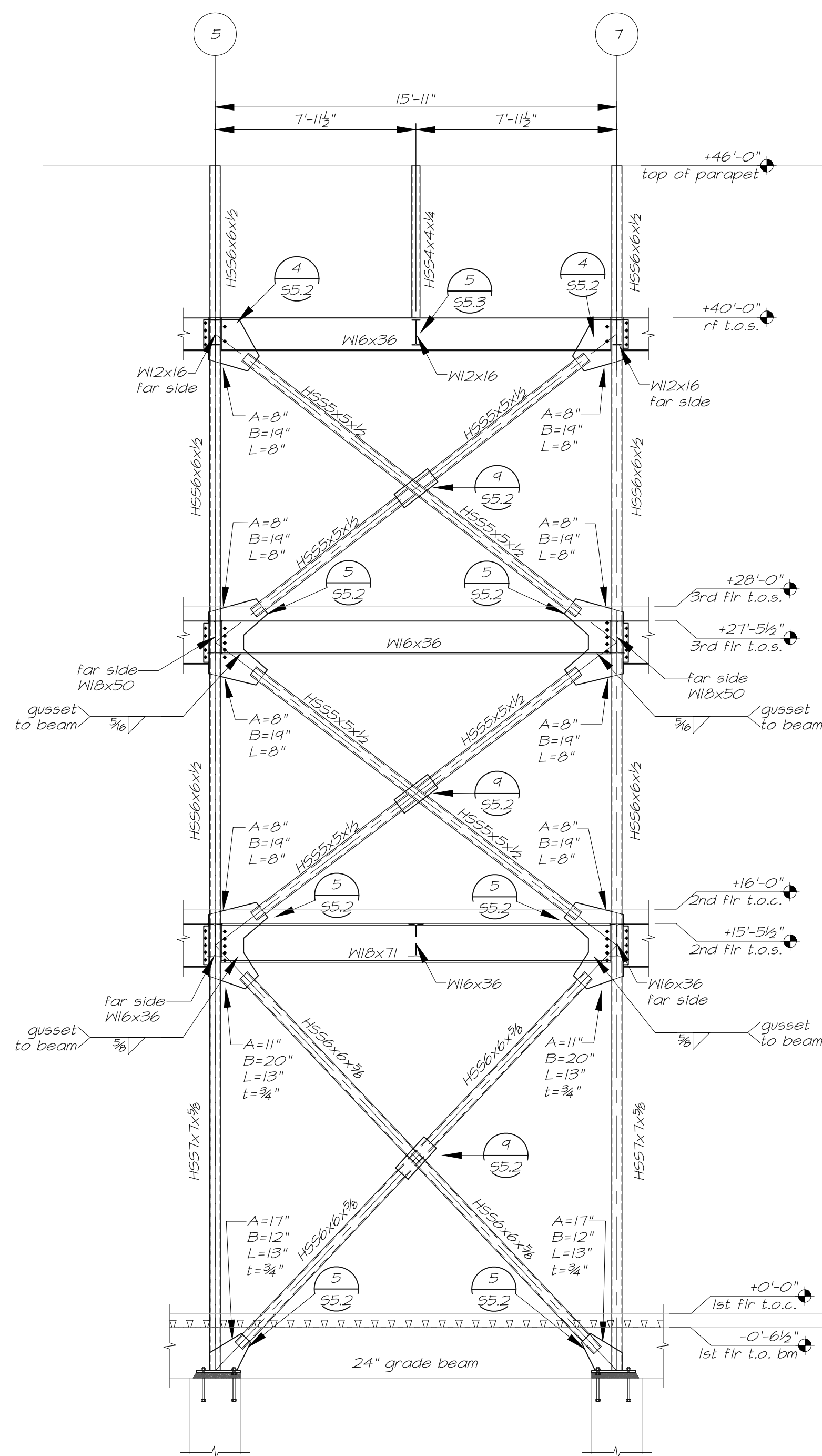
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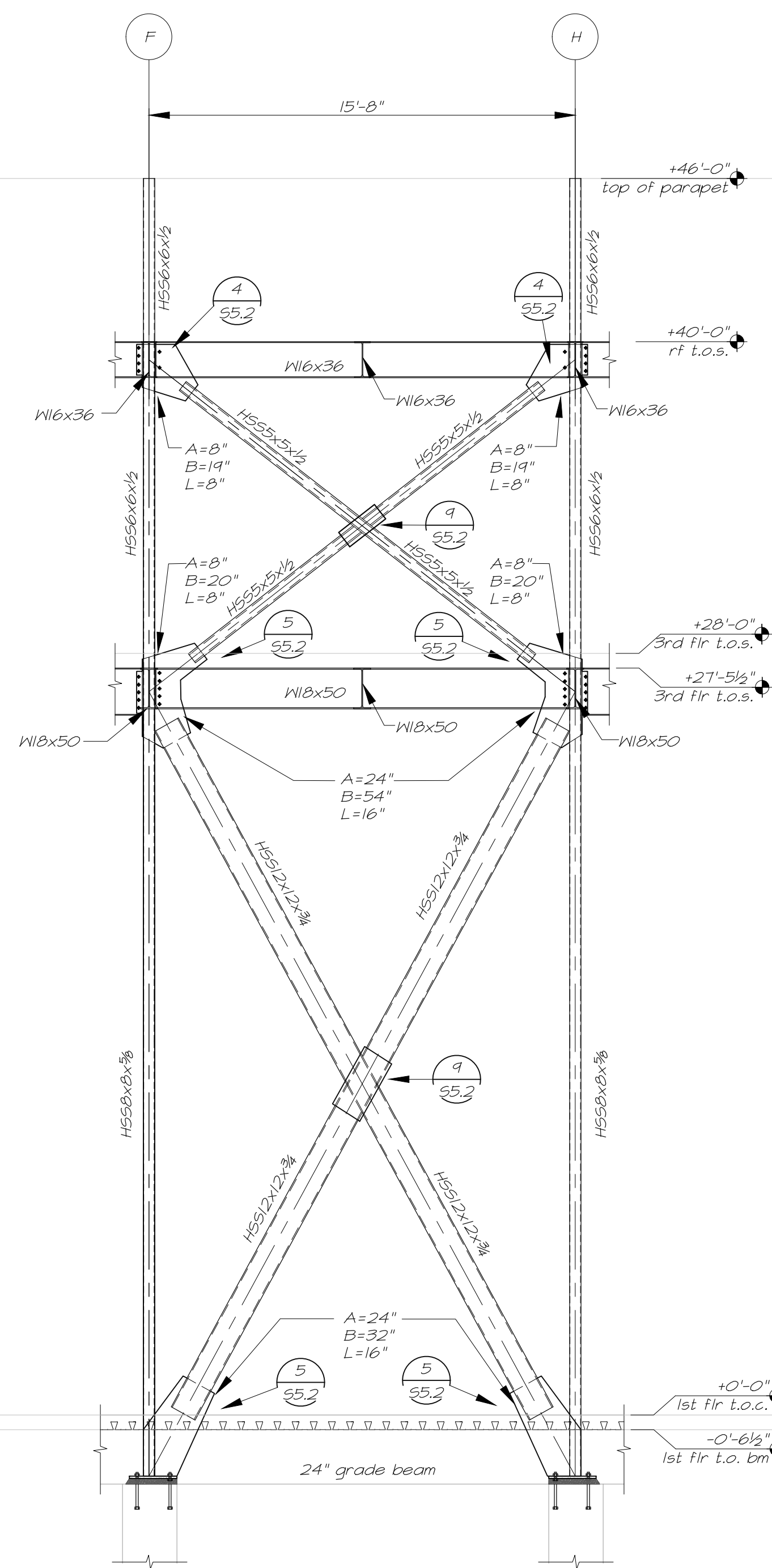
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**FRAME ELEVATIONS**

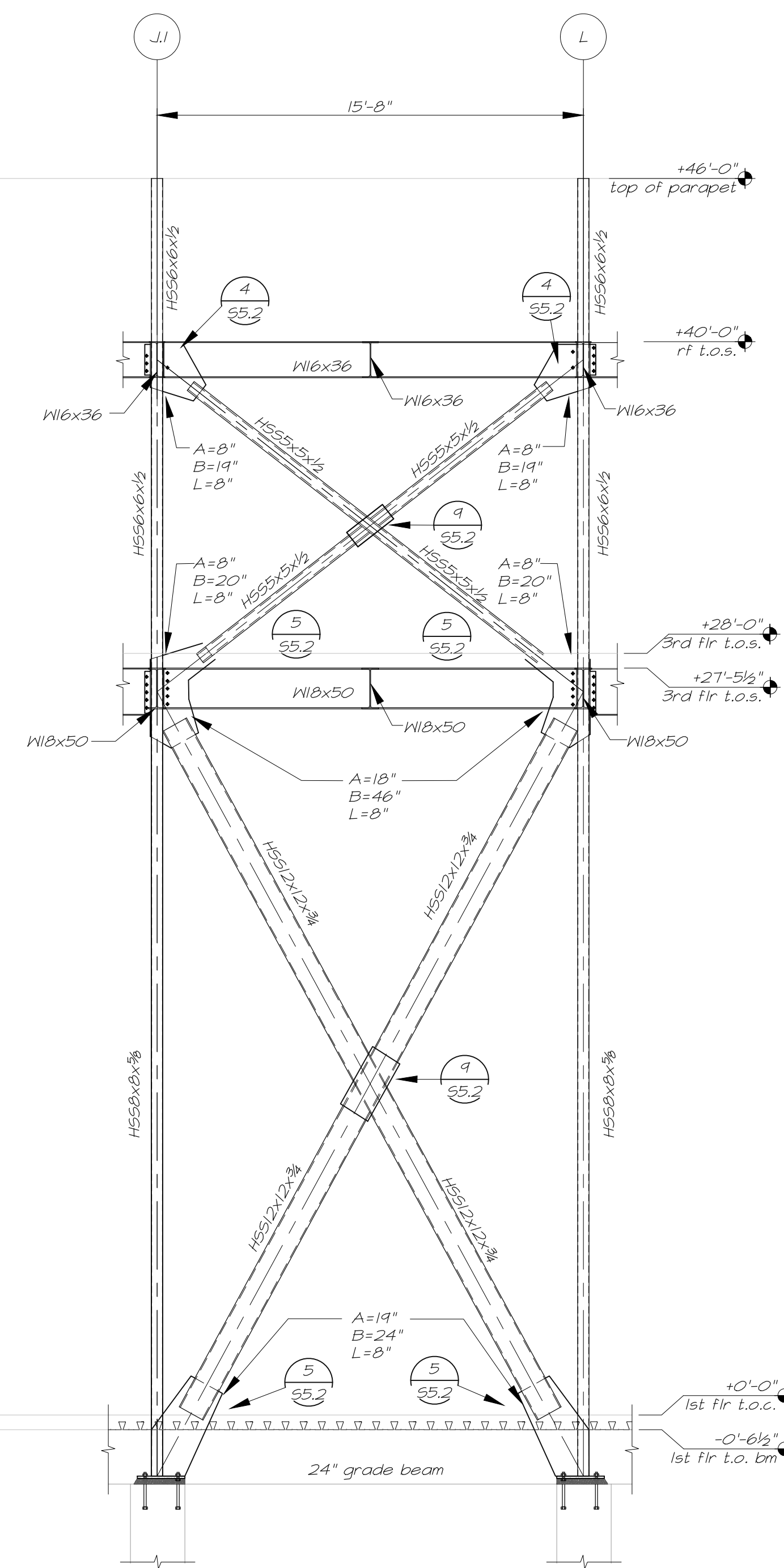
**S3.2**



**A** Frame Line Q.3 @ Grids 5-7  
 53.2 1/4"=1'-0"



**B** Frame Line I.1 @ Grids F-H  
 53.2 1/4"=1'-0"



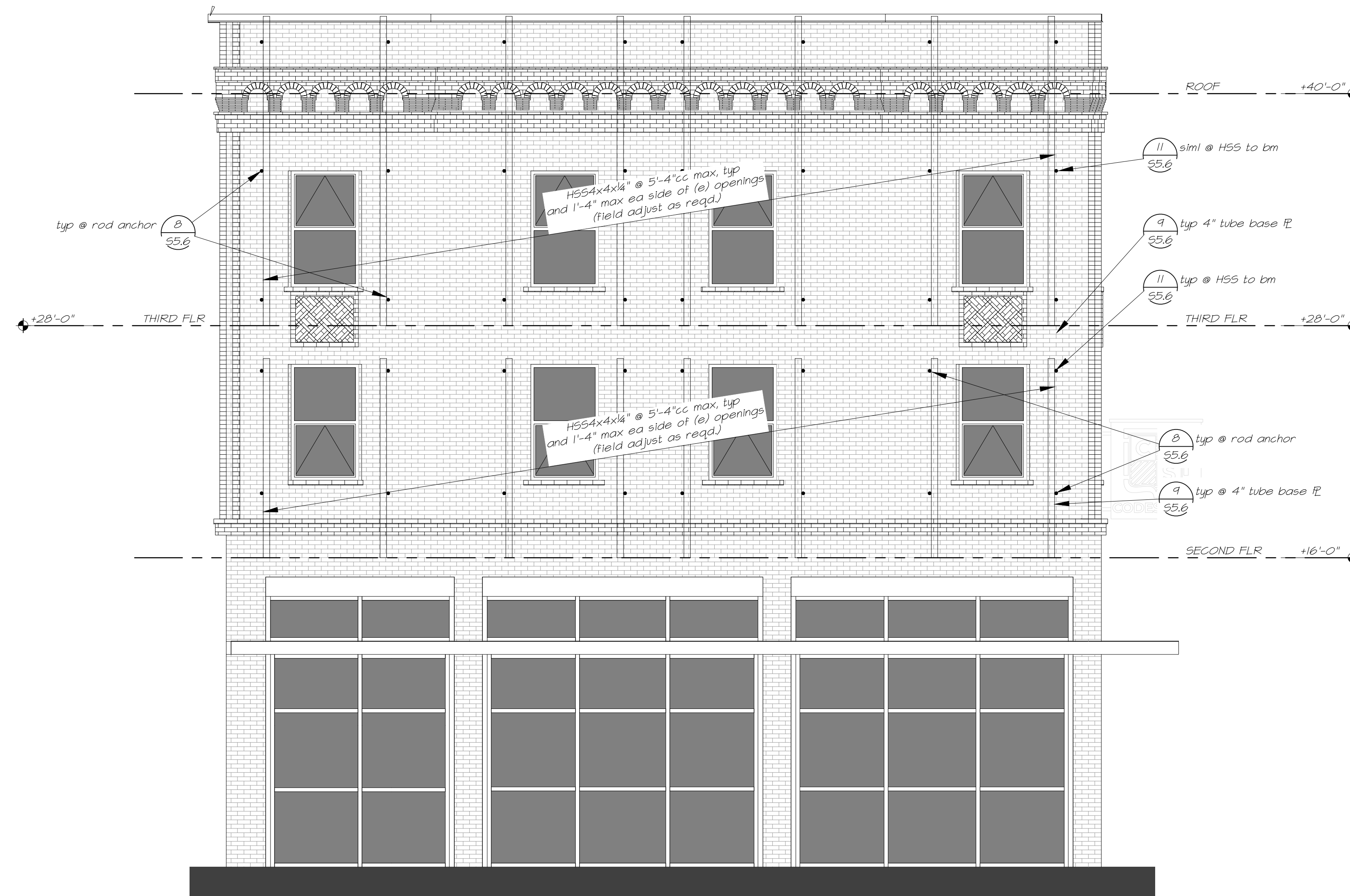
**C** Frame Line I.1 @ Grids J.I-L  
 53.2 1/4"=1'-0"

-11'-0"  
 basement flr









A  
S3.4  
1/4"=1'-0"

South Elevation



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**(E) BRICK FACADE  
BRACING BUILDING  
ELEVATION (SOUTH)**

**S3.4**





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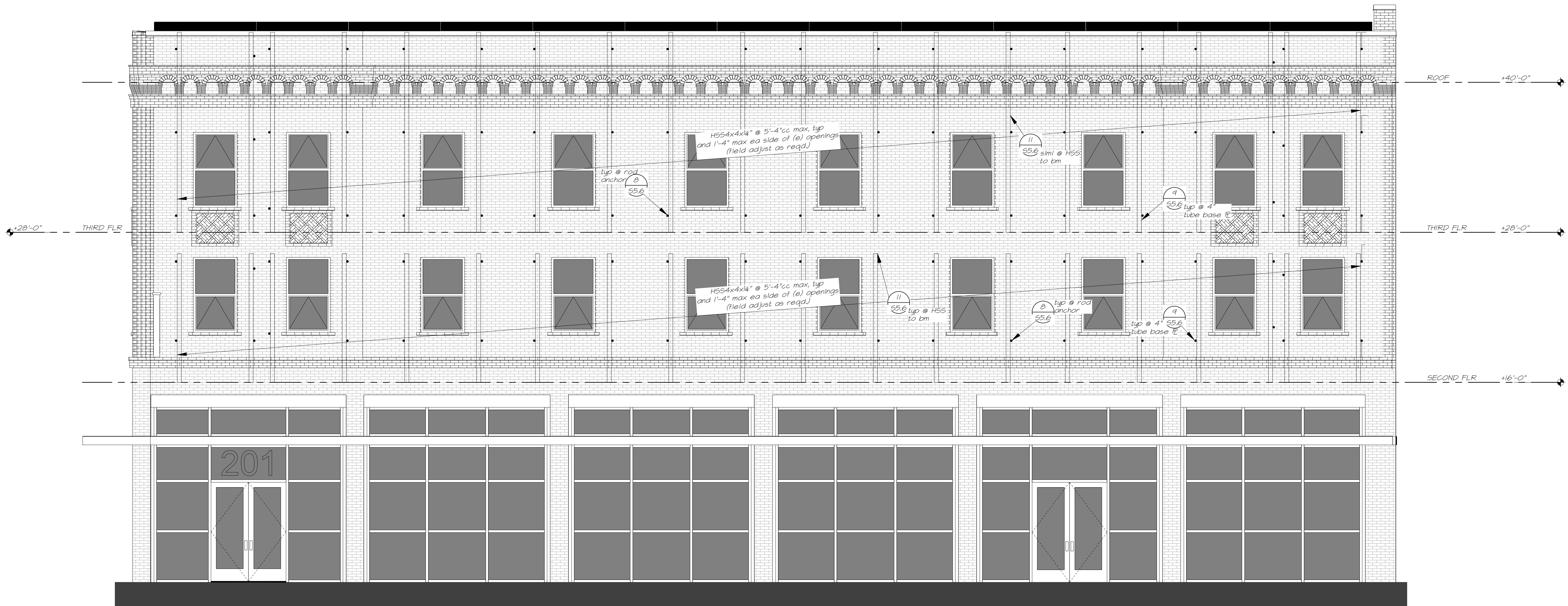
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**(E) BRICK FACADE  
 BRACING BUILDING  
 ELEVATION (EAST)**



**A**  
 53.5  
 East Elevation  
 1/4"=1'-0"





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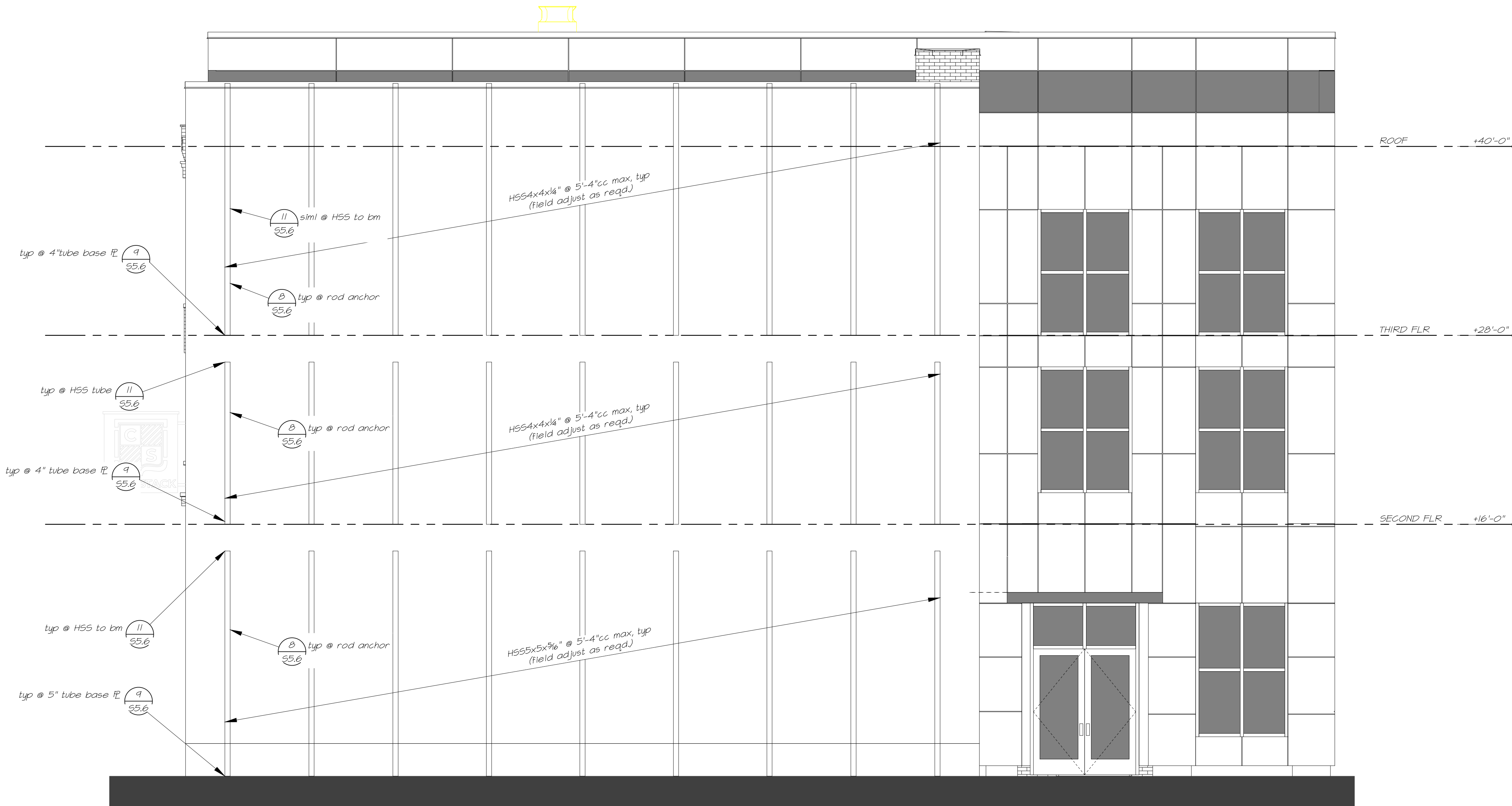
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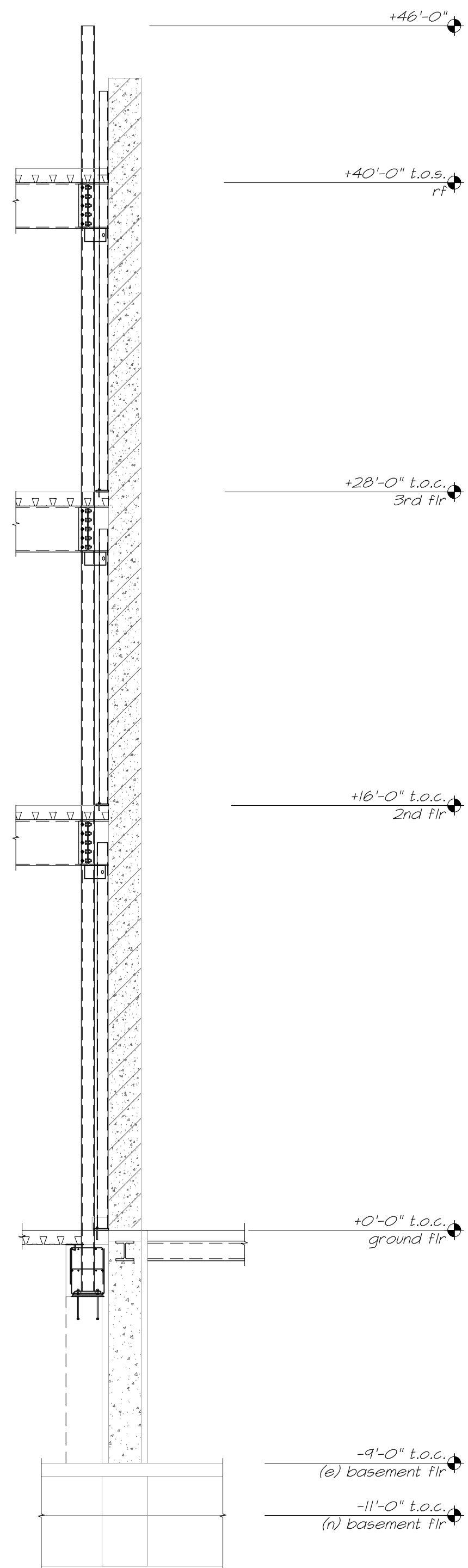
**(E) BRICK FACADE  
 BRACING BUILDING  
 ELEVATION (NORTH)**



**A** North Elevation  
 S3.6 1/4"=1'-0"

**S3.6**





A Section - grid  
S4.1 1/4"=1'-0"



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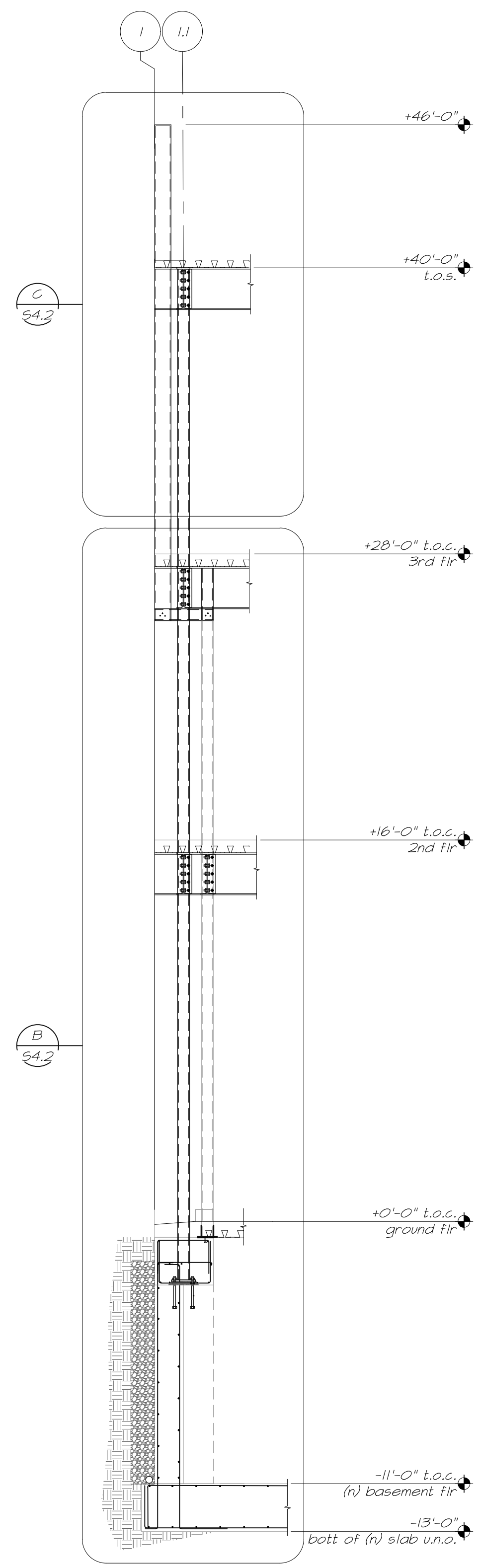
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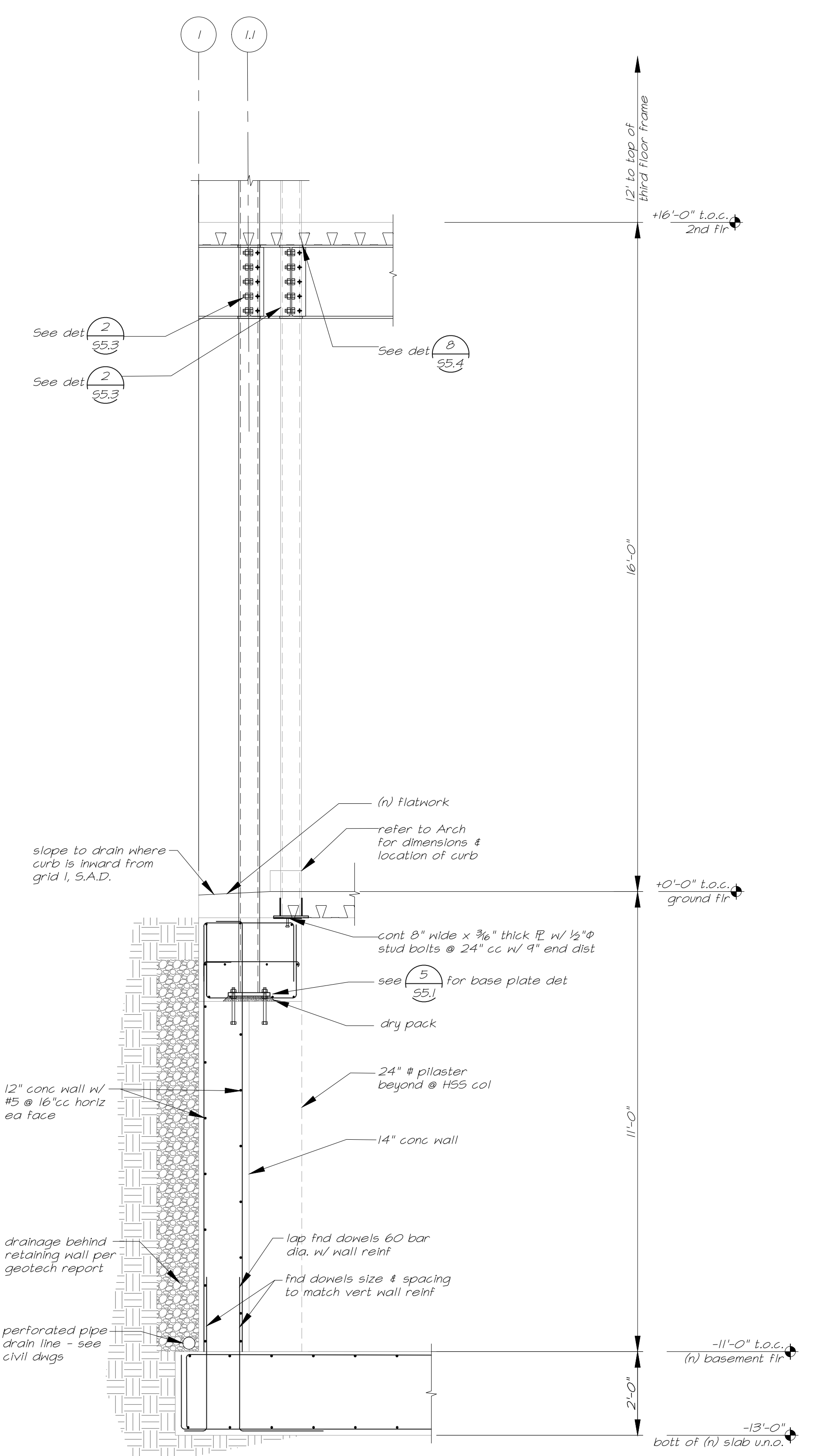
SECTIONS

S4.1

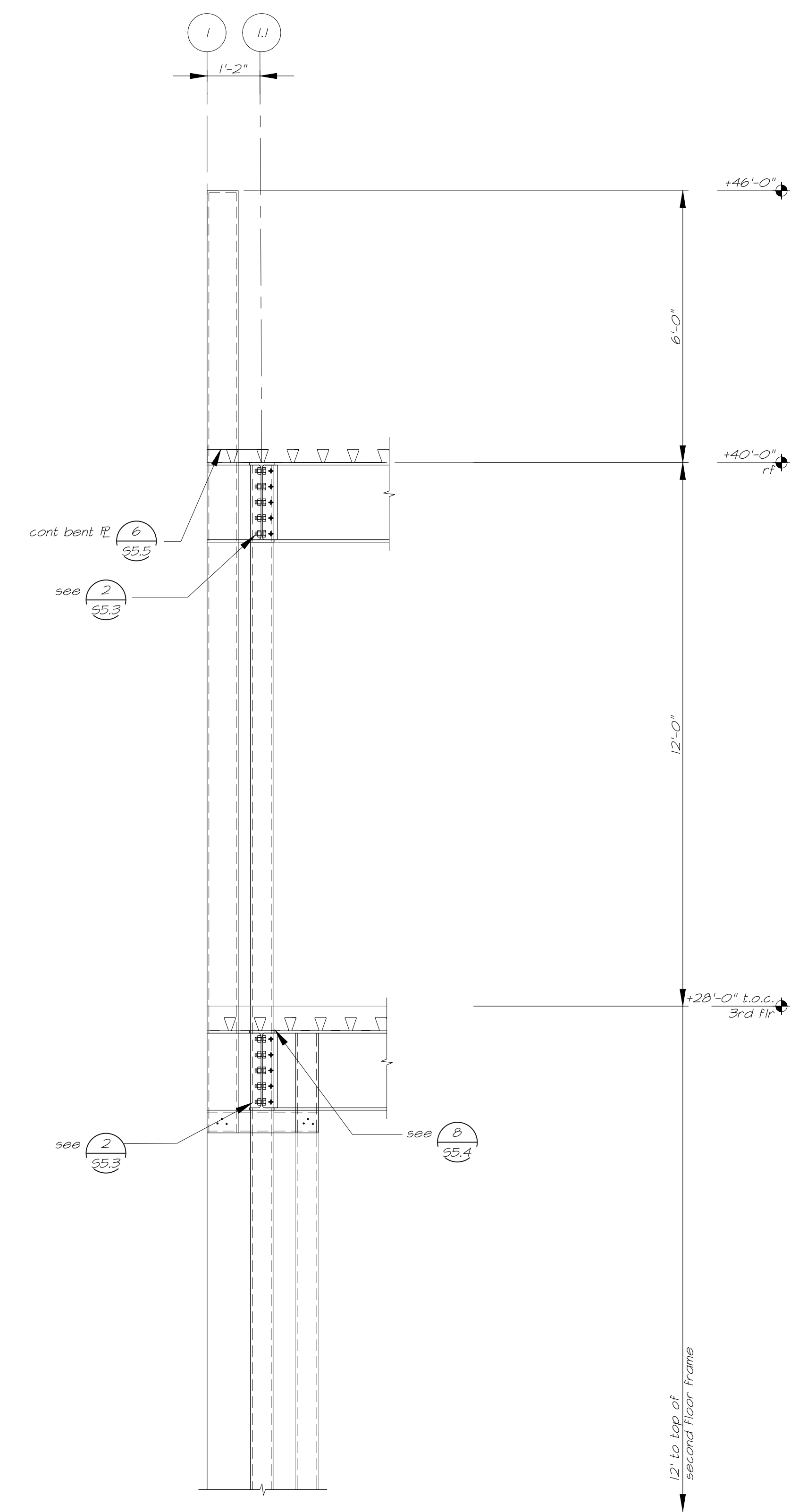




**A** Section - grid I at F-H.1/J.1-L  
 1/4"=1'-0"



**B** Section  
 1/2"=1'-0"



**C** Section  
 1/2"=1'-0"



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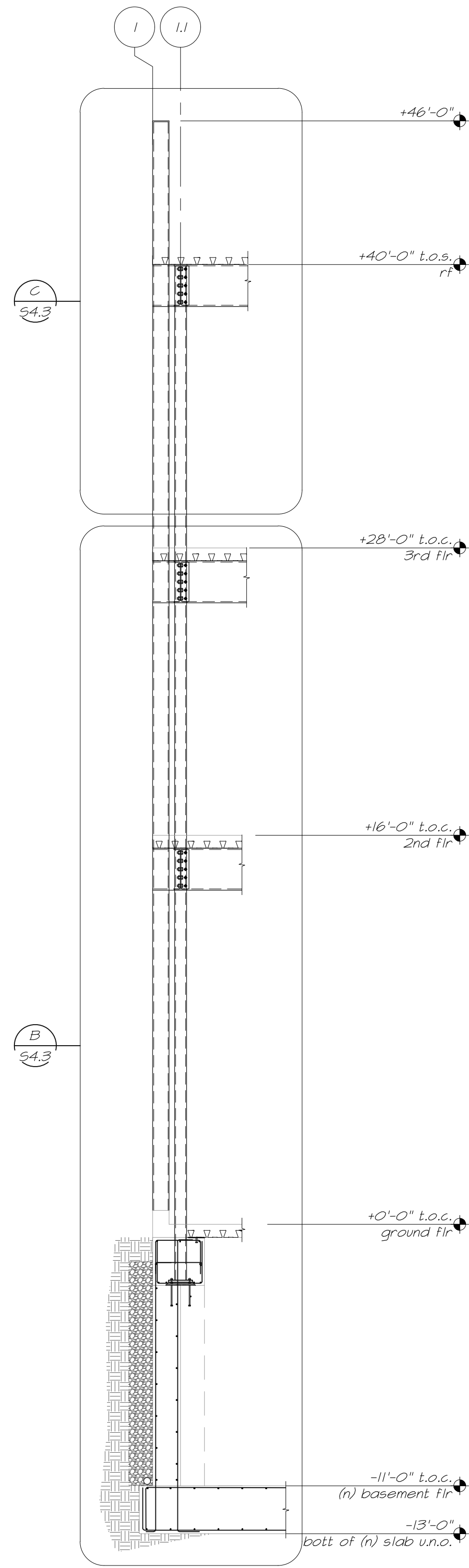
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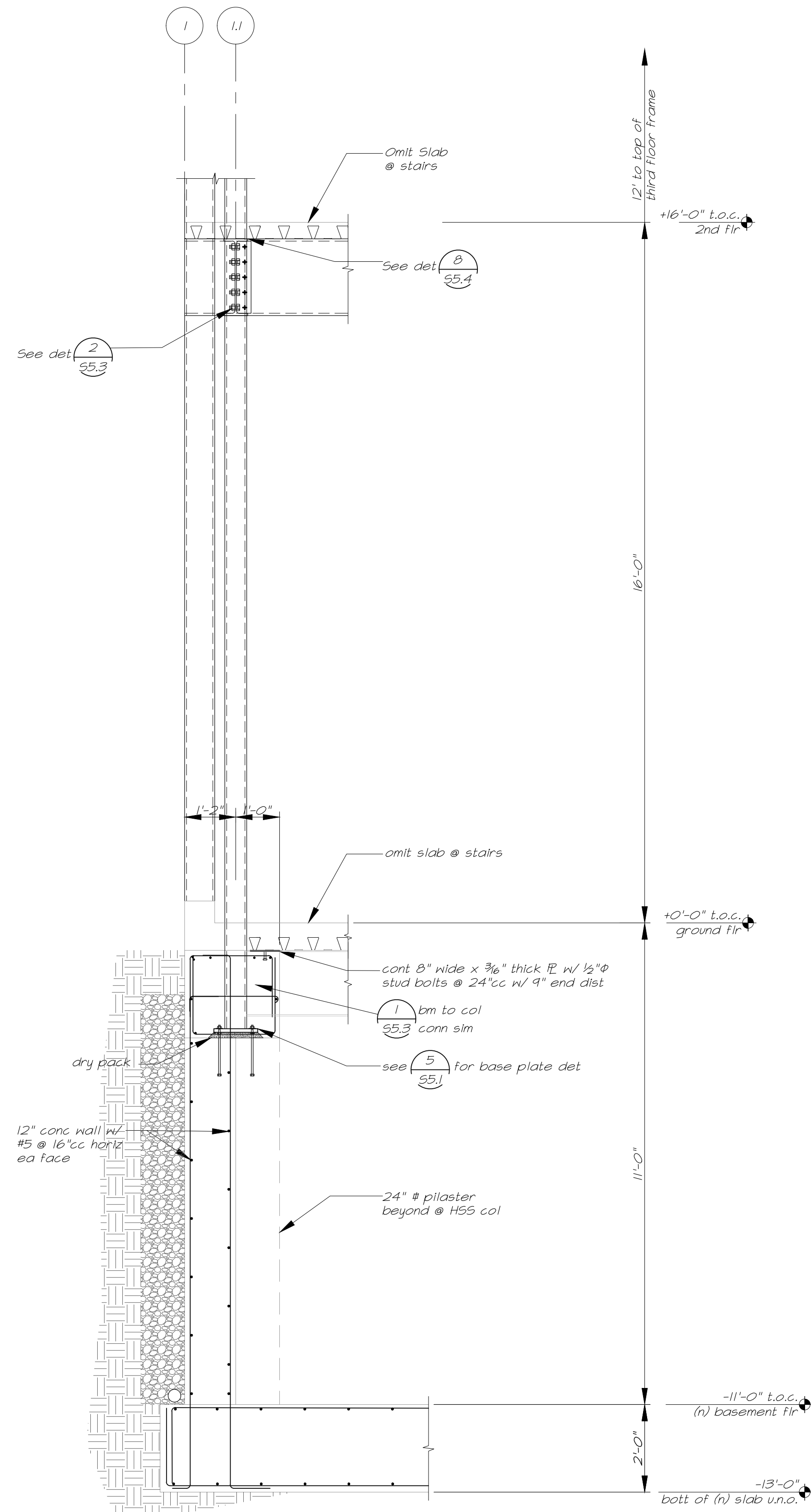
**BUILDING  
 ELEVATION (WEST)**

**S4.2**

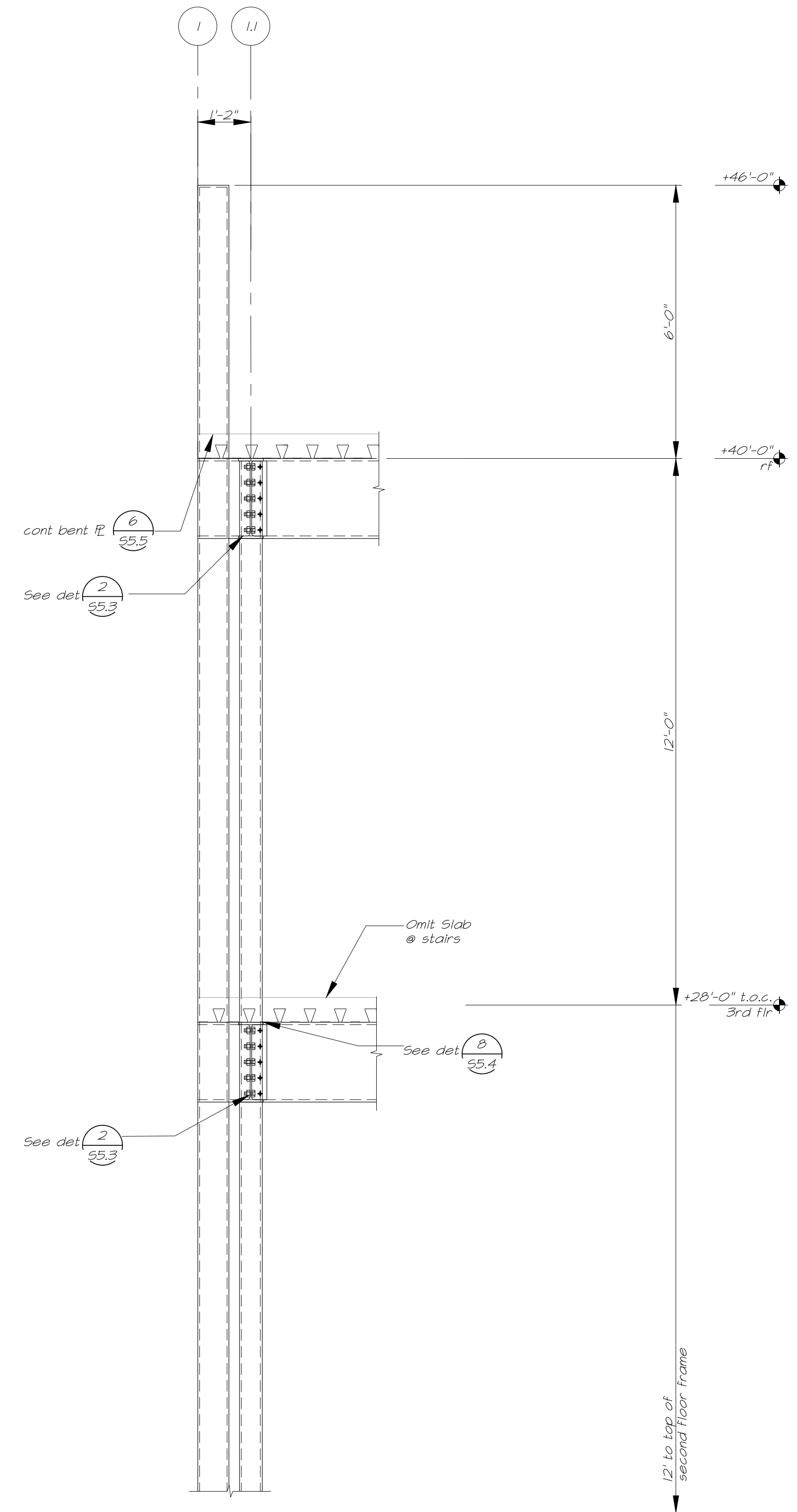




**A** Section - grid I at A-F/H.I-J.I/L-Q.3  
S4.3 1/4"=1'-0"



**B** Section  
S4.3 1/2"=1'-0"



**C** Section  
S4.3 1/2"=1'-0"



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SECTIONS

**S4.3**

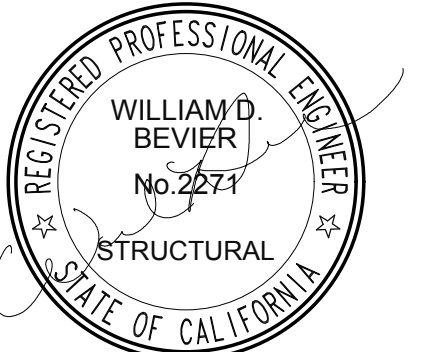




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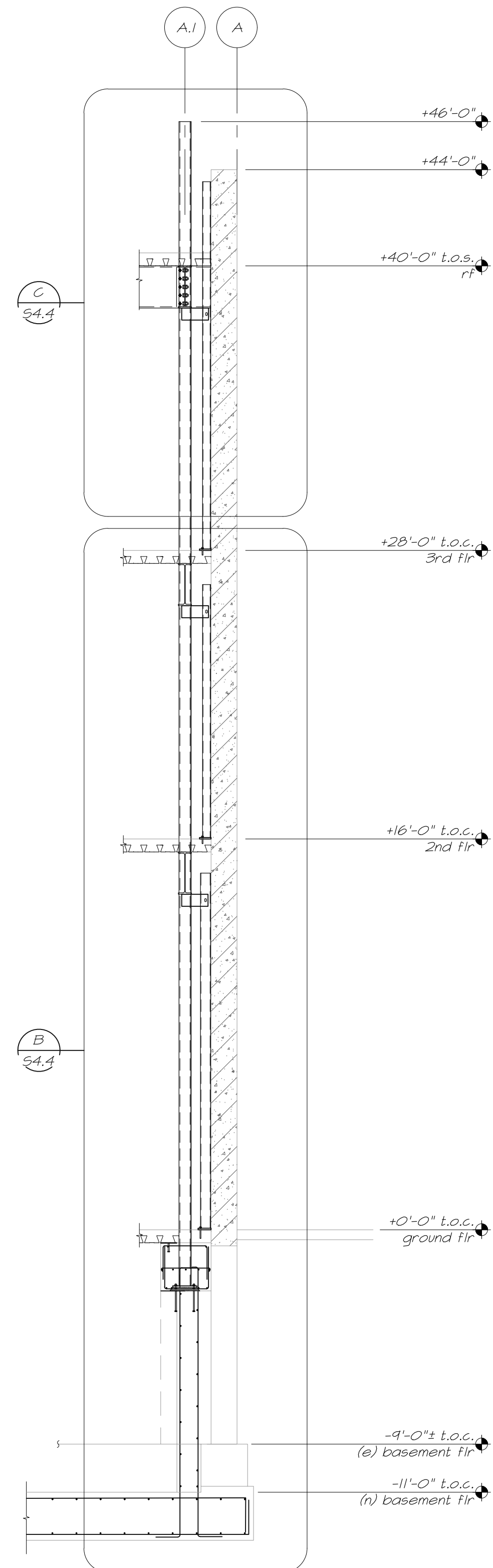
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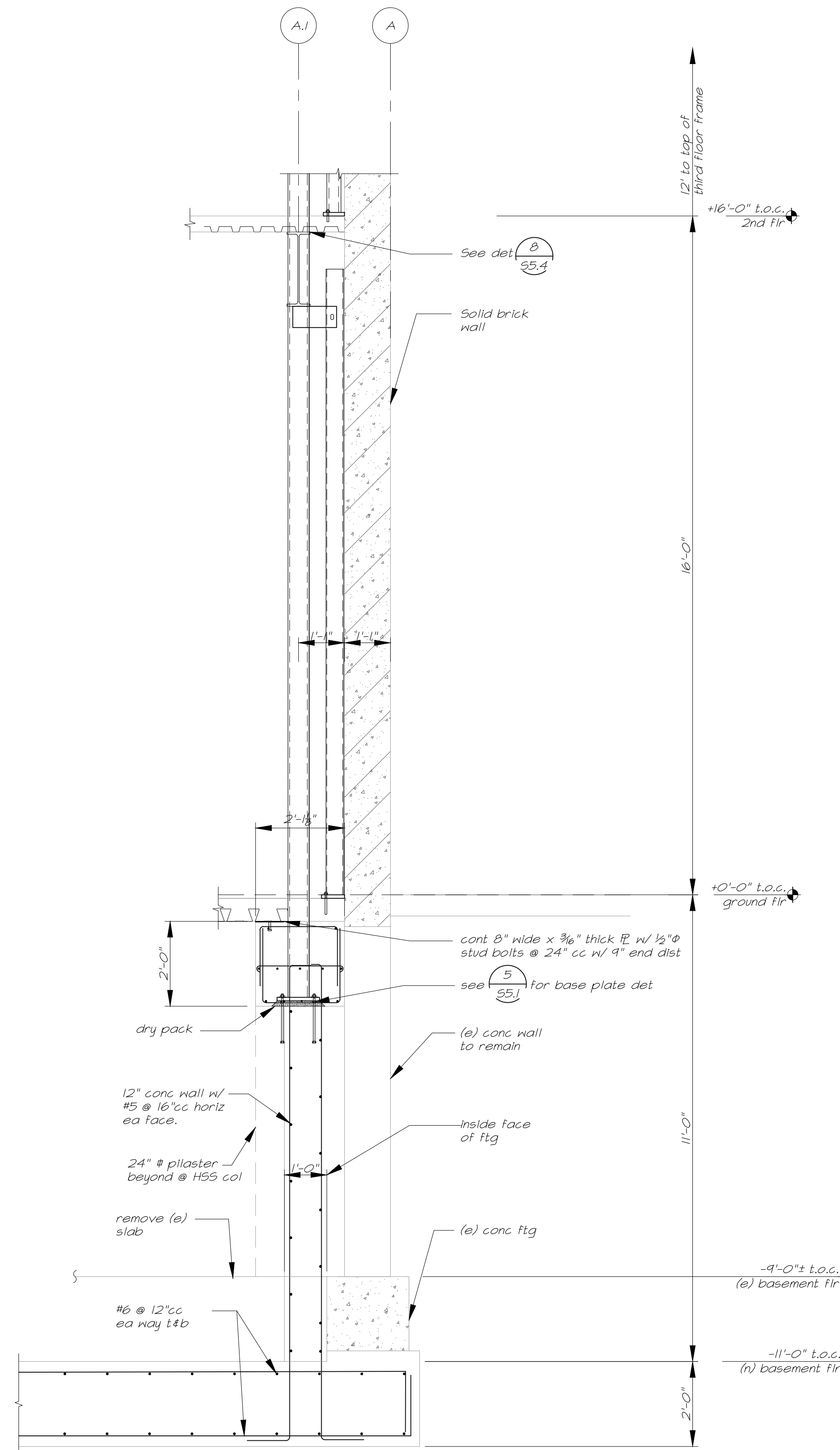
PROJECT NO: 2023-04  
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SECTIONS

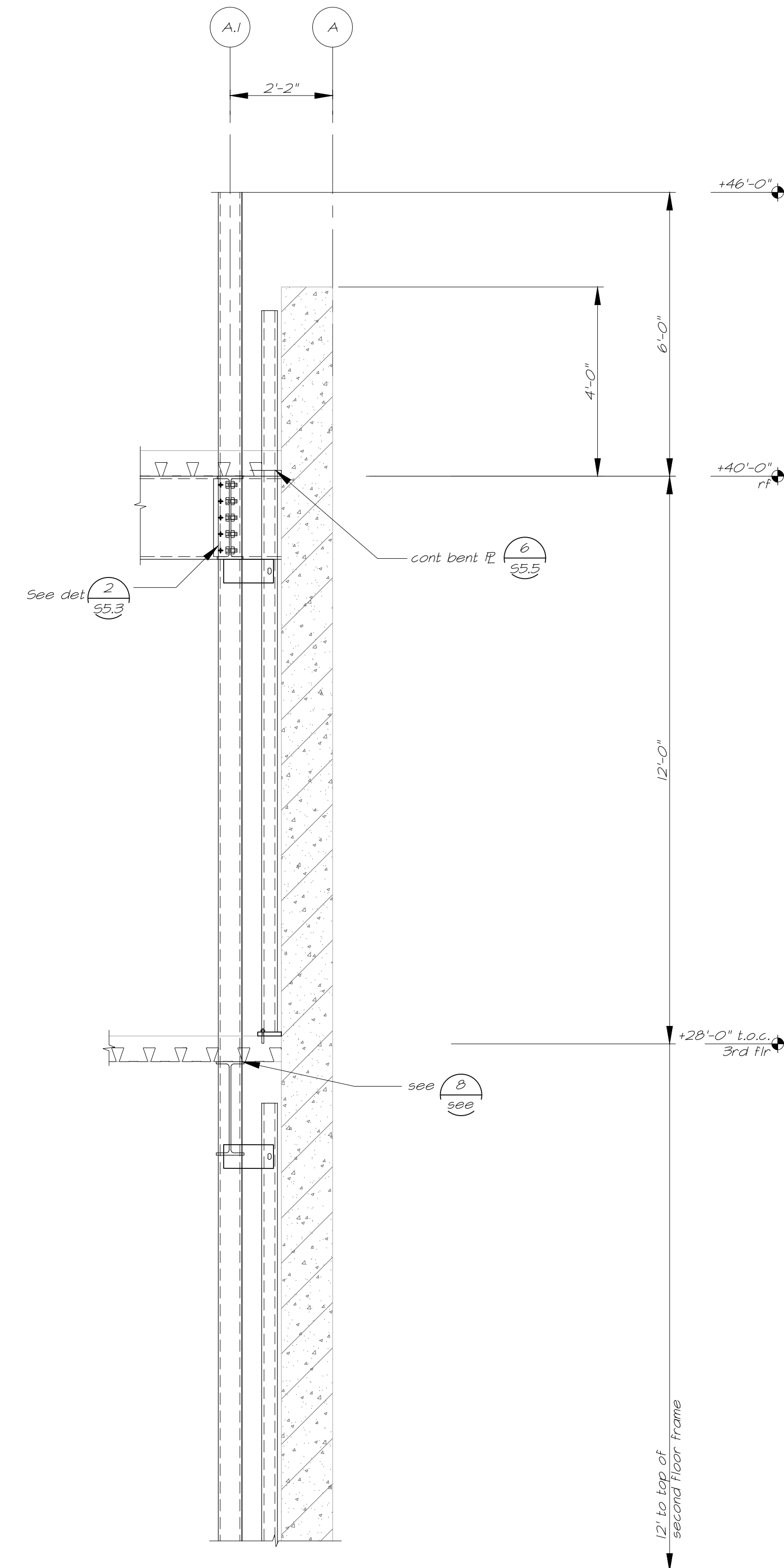
**S4.4**



**A** Section @ GL A from 3-9  
 54.4 1/4"=1'-0"

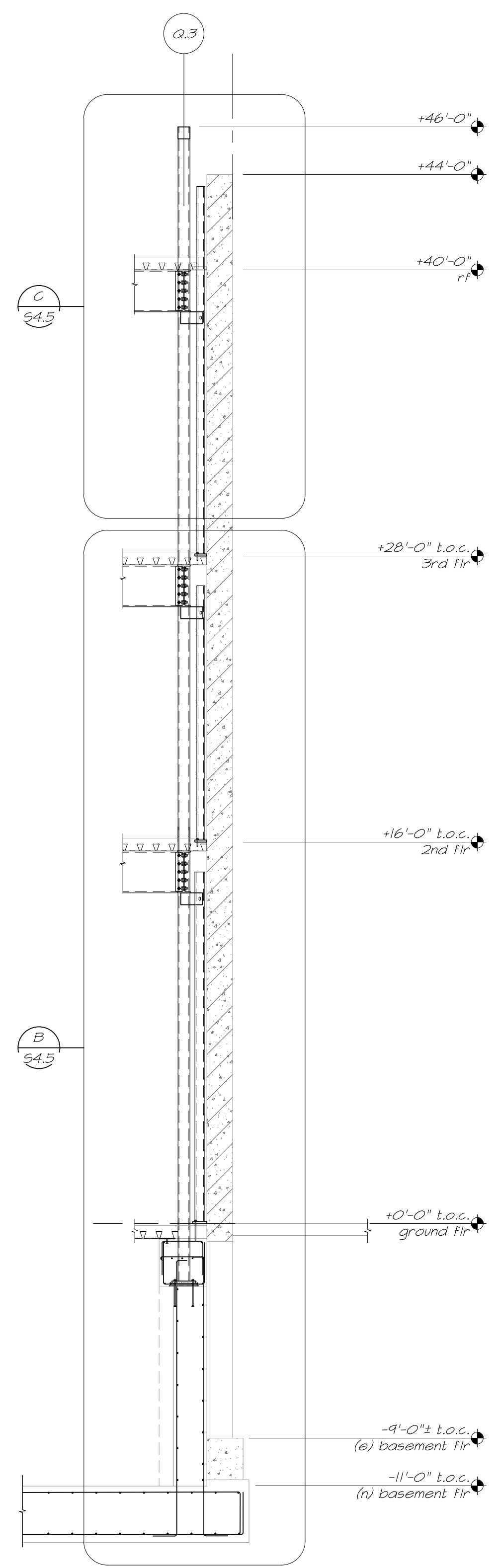


**B** Section  
 54.4 1/2"=1'-0"

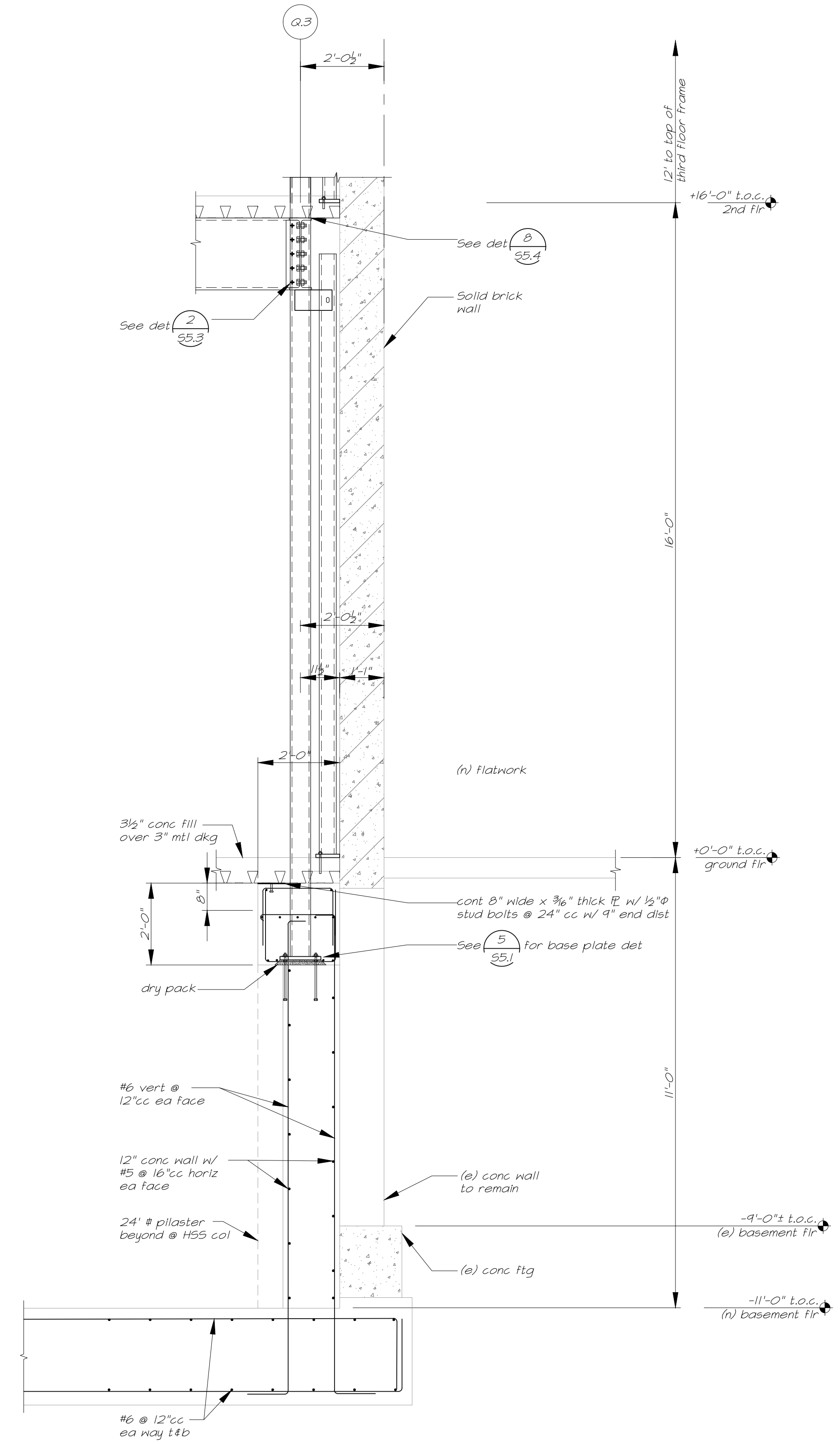


**C** Section  
 54.4 1/2"=1'-0"

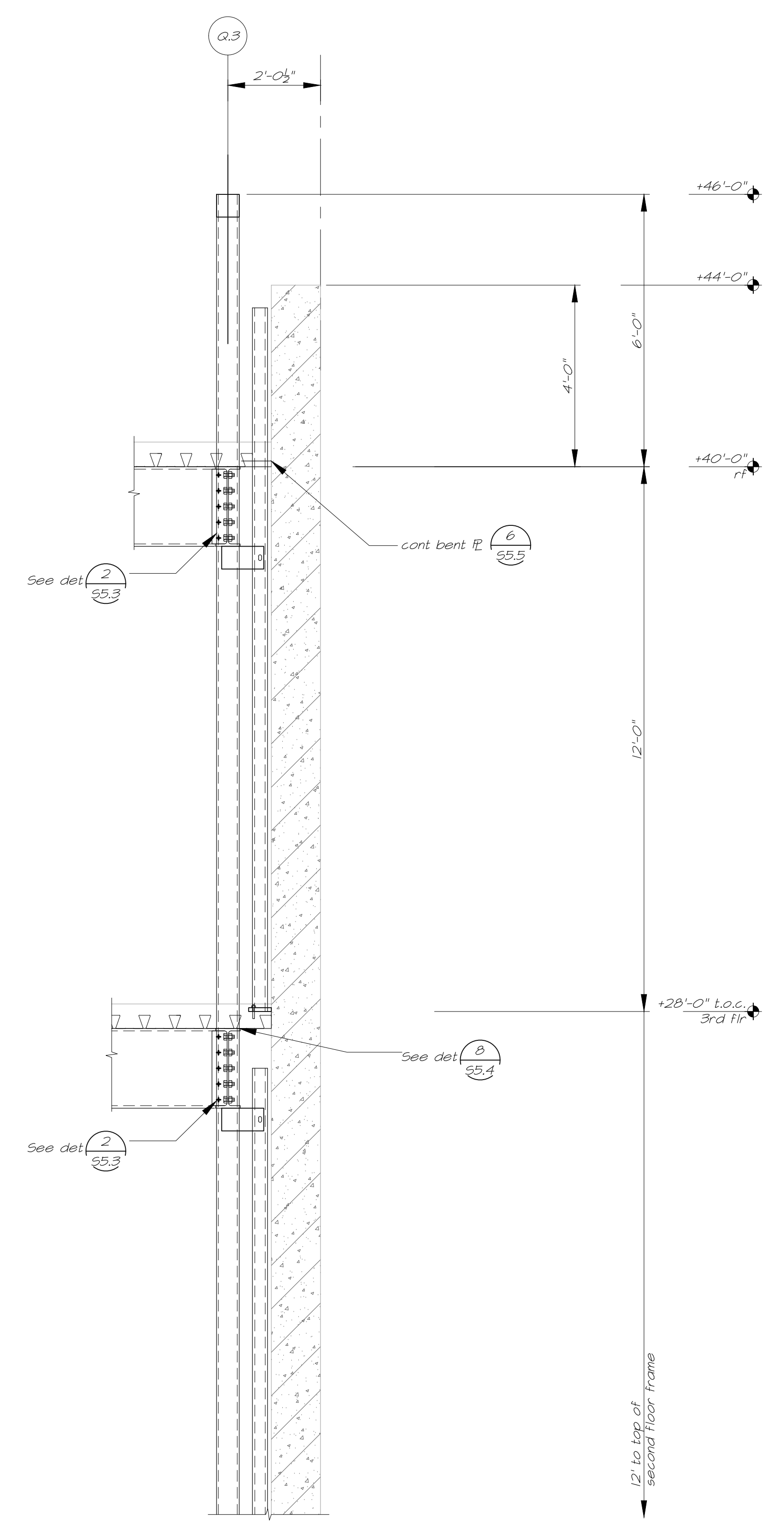




**A** Section @GL Q.3 from 3-9  
 S4.5 1/2"=1'-0"



**B** Section  
 S4.5 1/2"=1'-0"

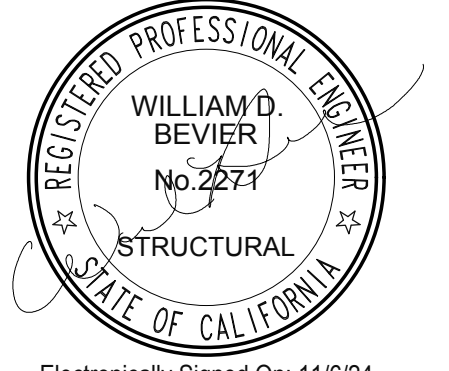


**C** Section  
 S4.5 1/2"=1'-0"

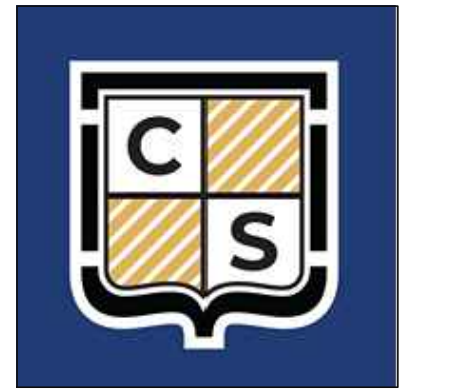


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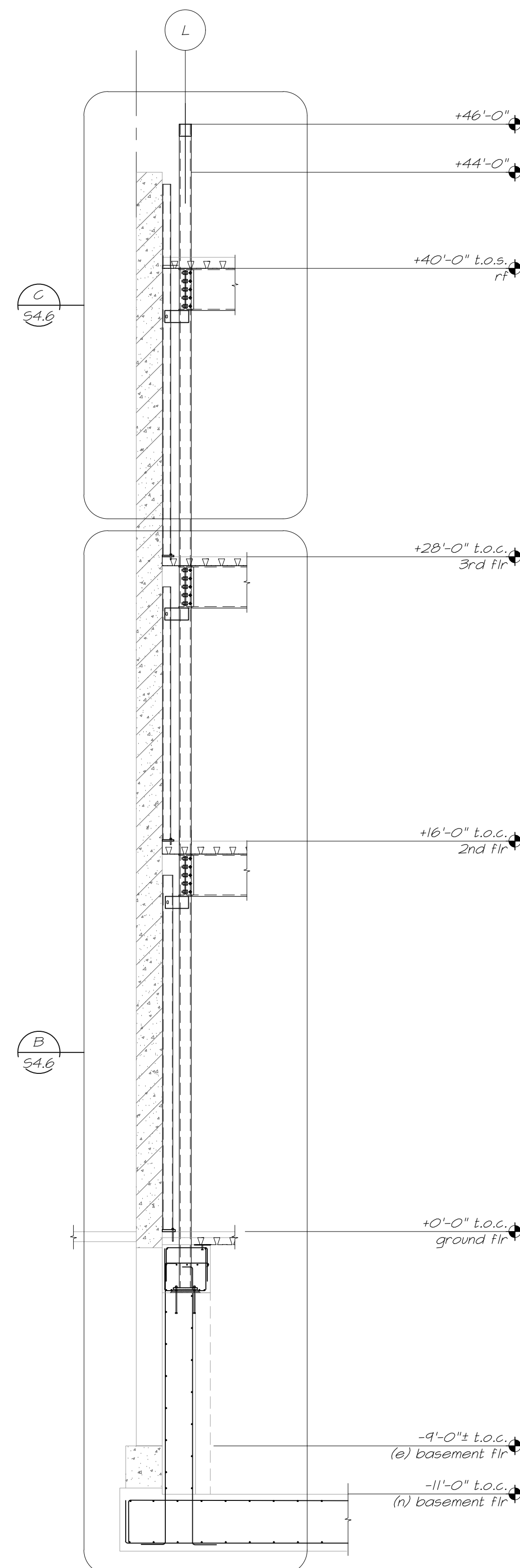
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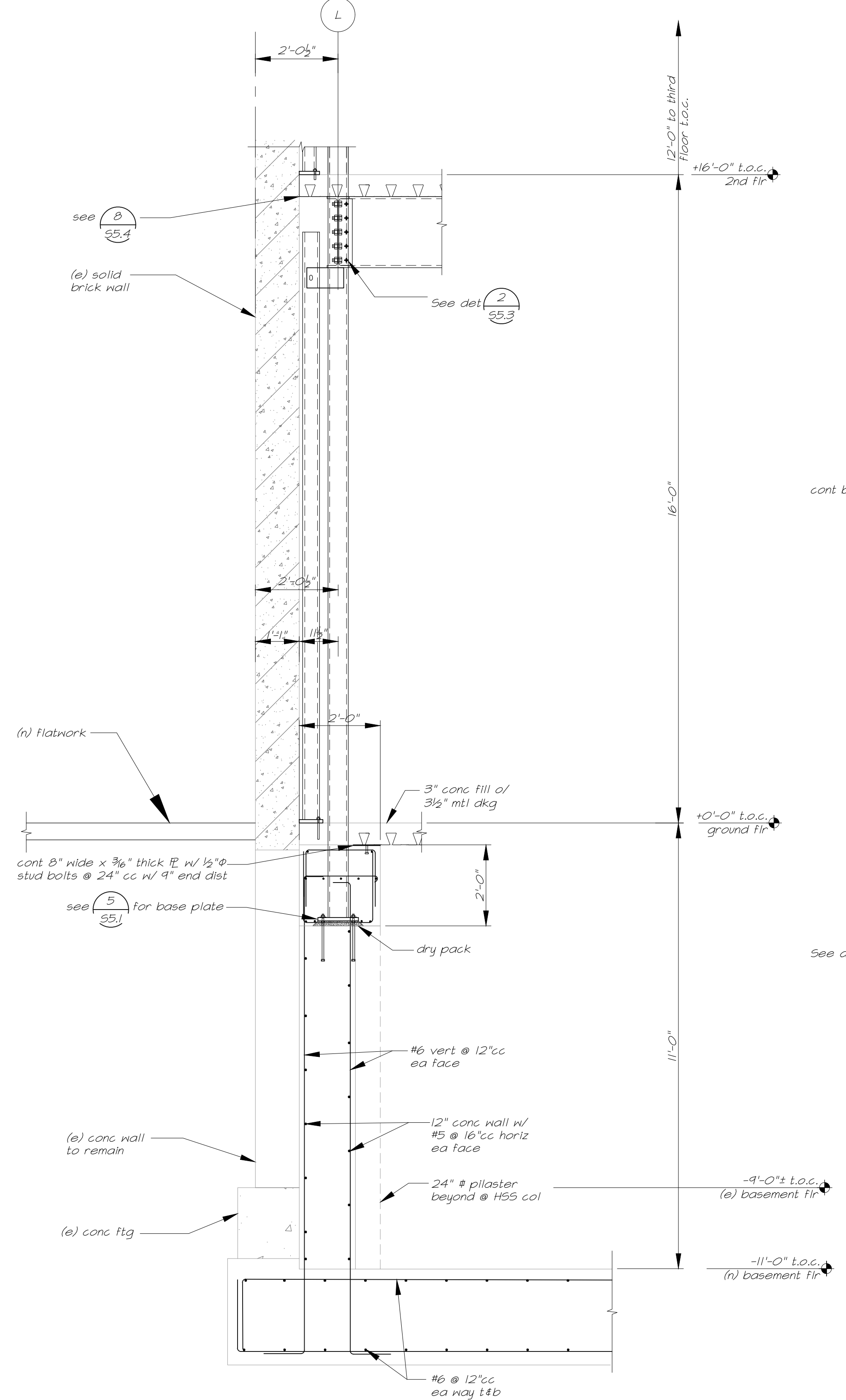
**SECTIONS**

**S4.5**

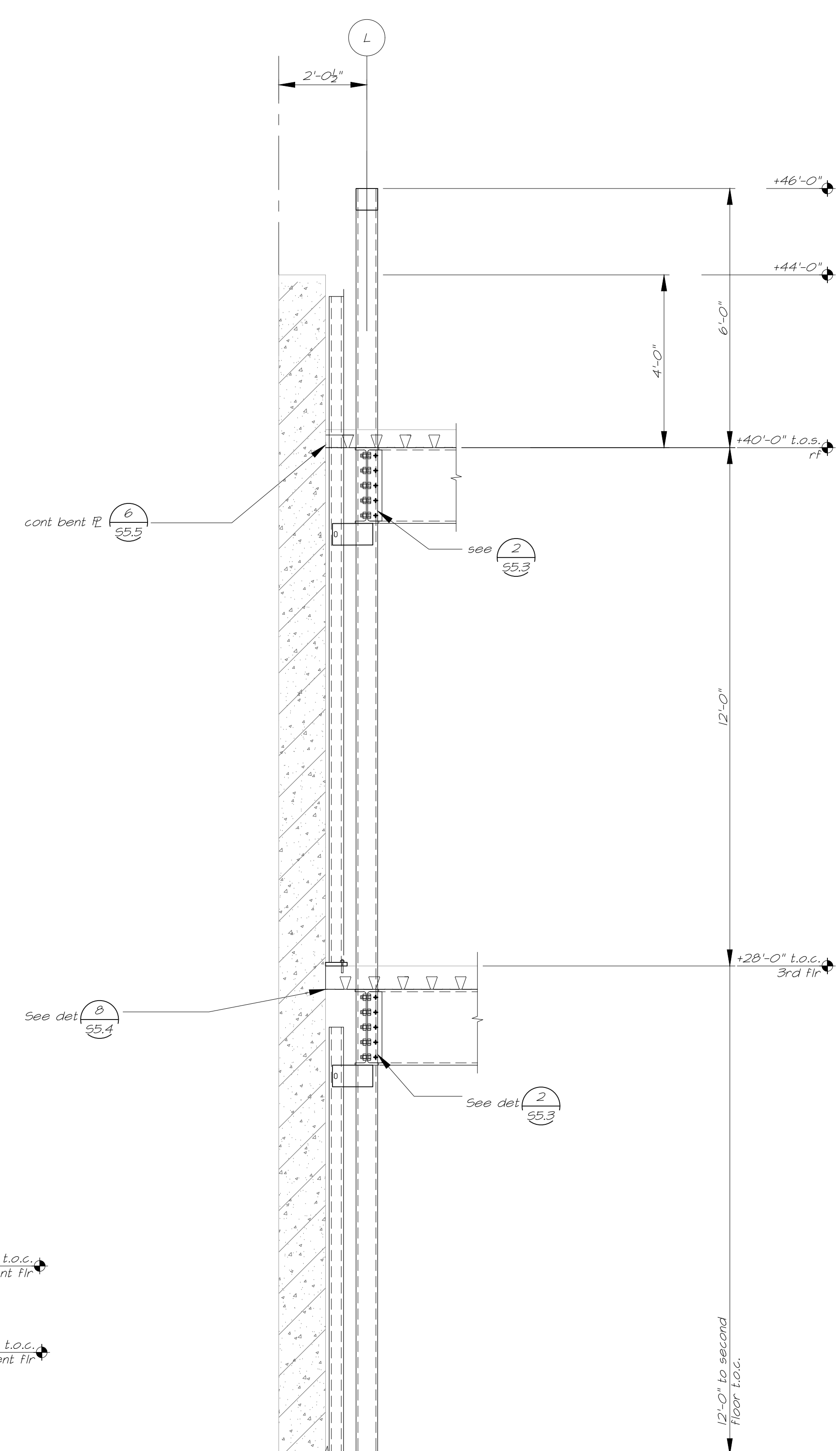




**A** Section  
54.6  
1/4"=1'-0"



**B** Section  
54.6  
1/2"=1'-0"

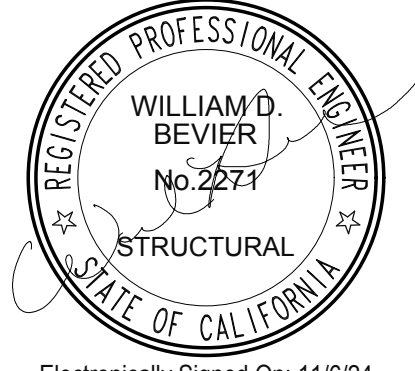


**C** Section  
54.6  
1/2"=1'-0"



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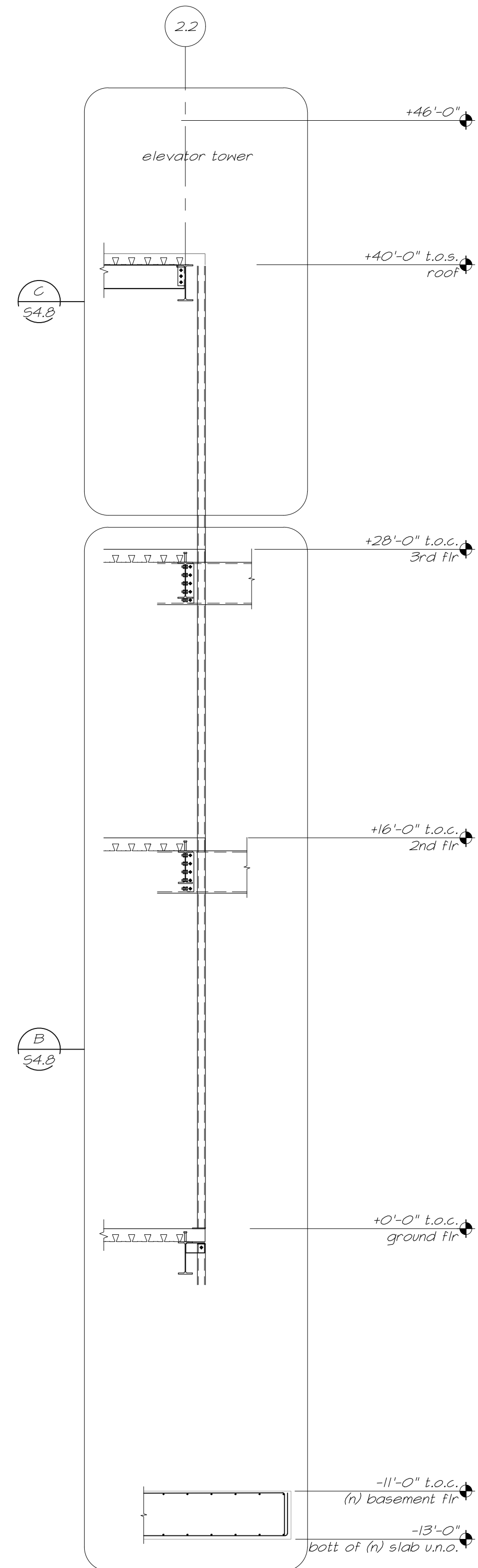
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**SECTIONS**





**A** Section - grid 2.2 at elevator  
 S4.8 1/4"=1'-0" grid 3 sim



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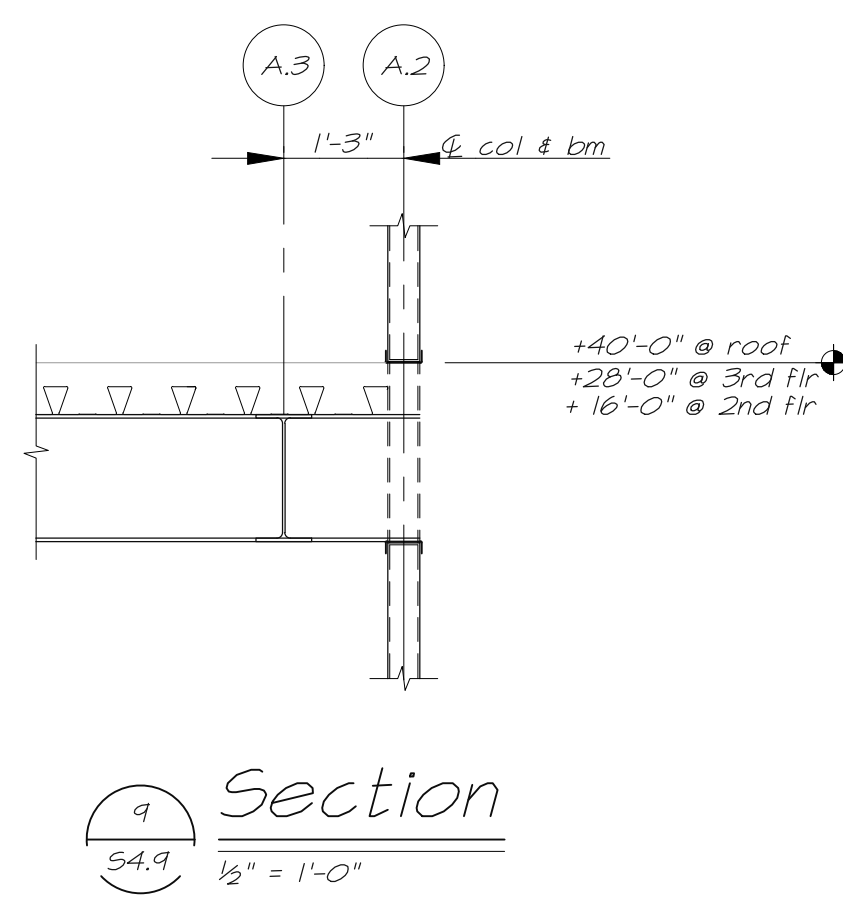
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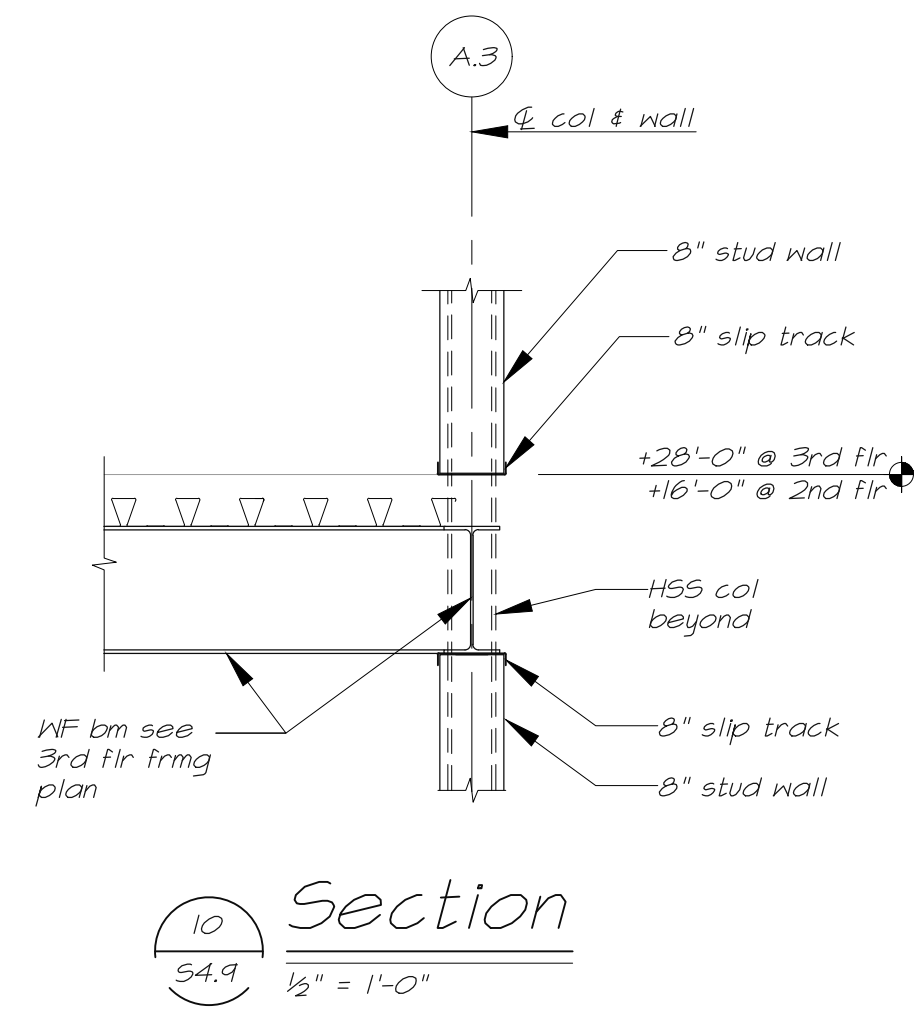
**ELEVATOR  
 SECTIONS**

**S4.8**

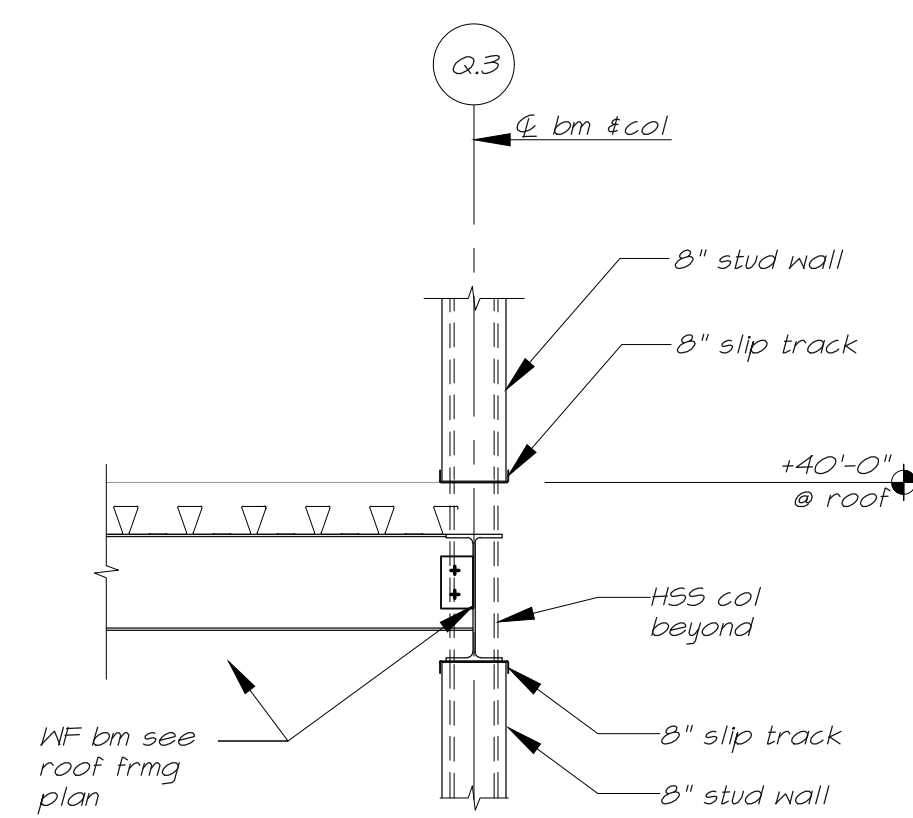




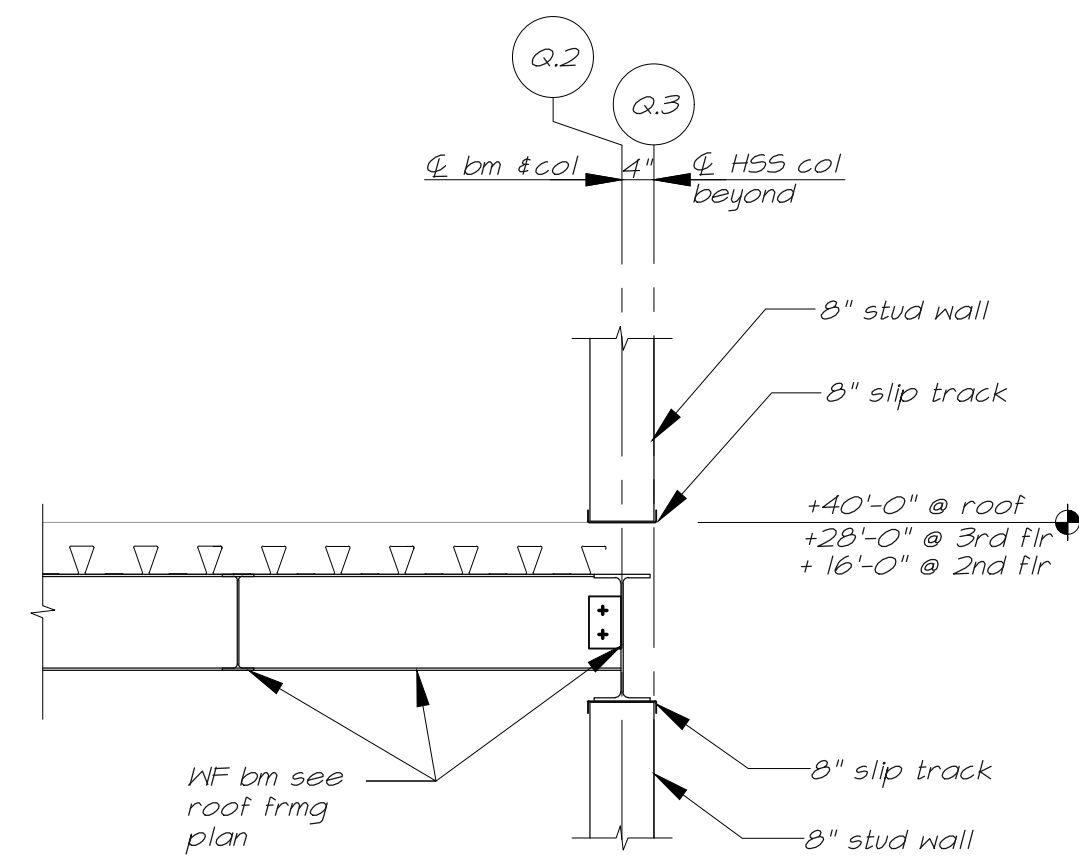
9 Section  
S4.9 1/2" = 1'-0"



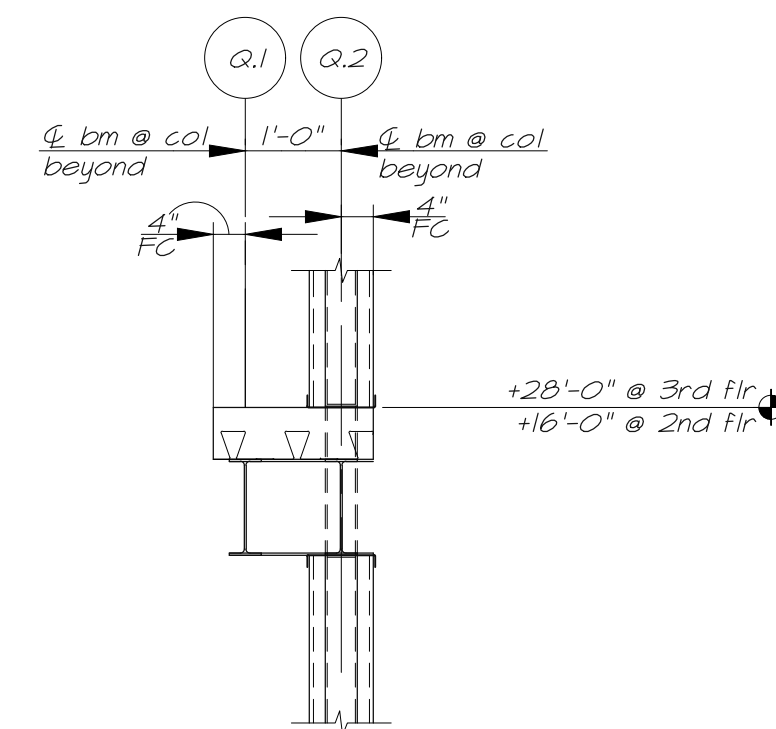
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S4.9 1/2" = 1'-0"



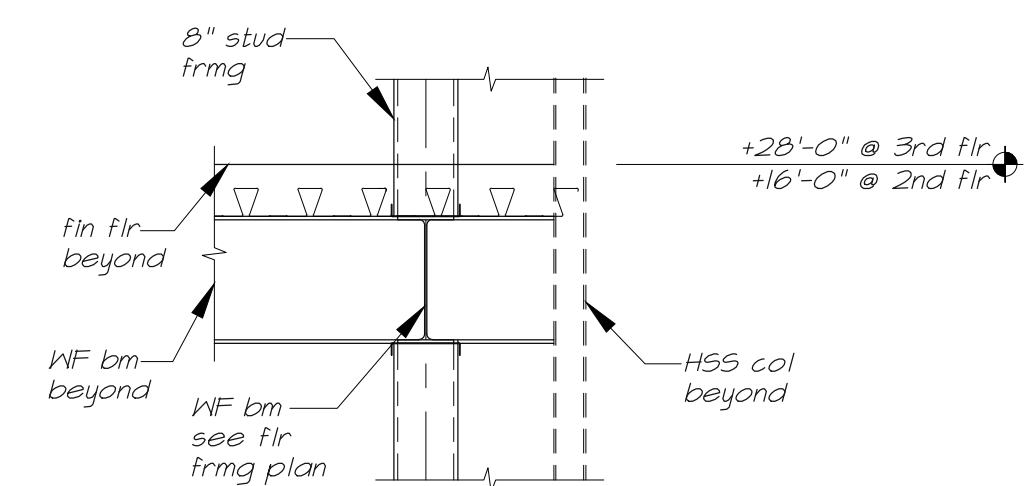
5 Section  
S4.9 1/2" = 1'-0"



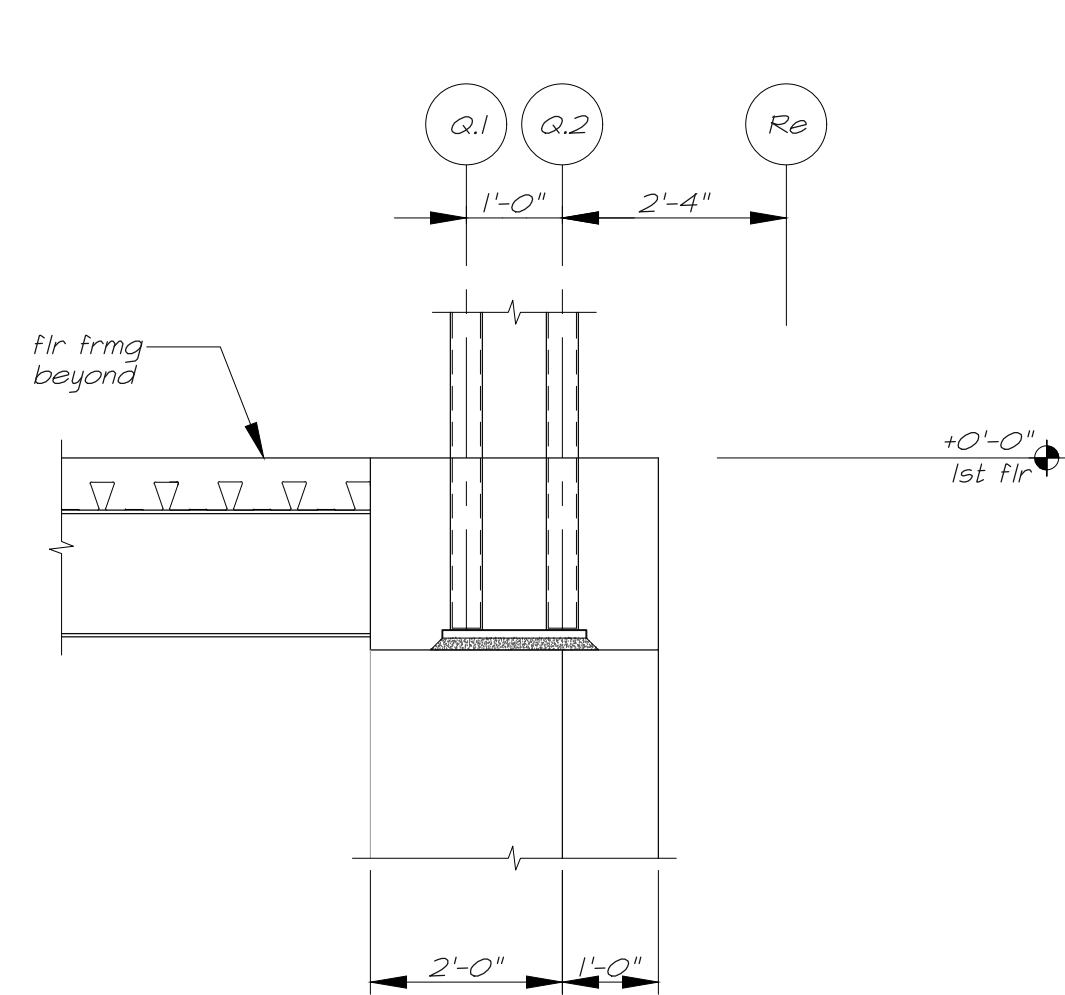
6 Section  
S4.9 1/2" = 1'-0"



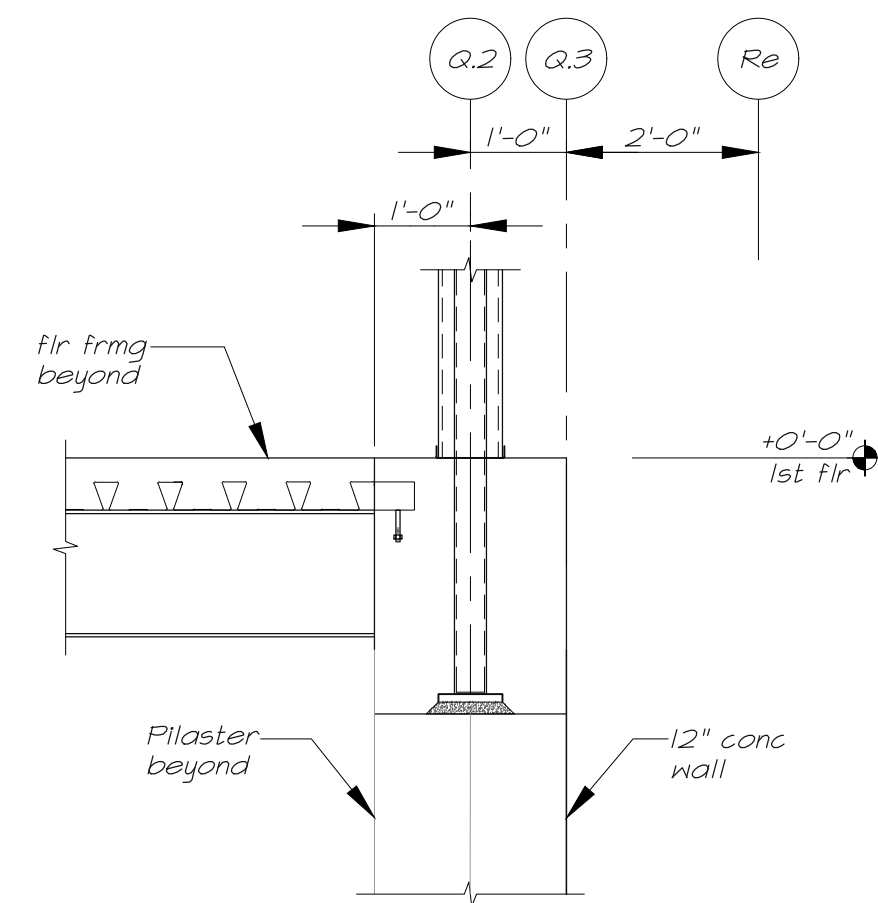
7 Section  
S4.9 1/2" = 1'-0"



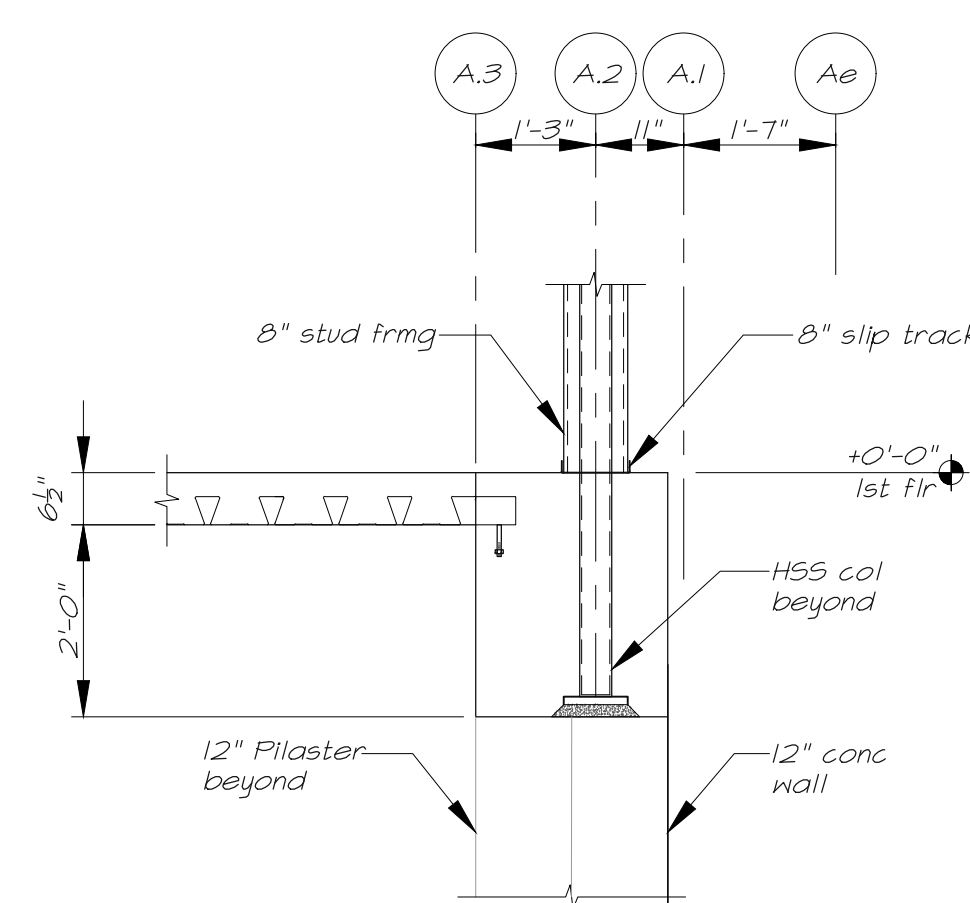
8 Section  
S4.9 1/2" = 1'-0"



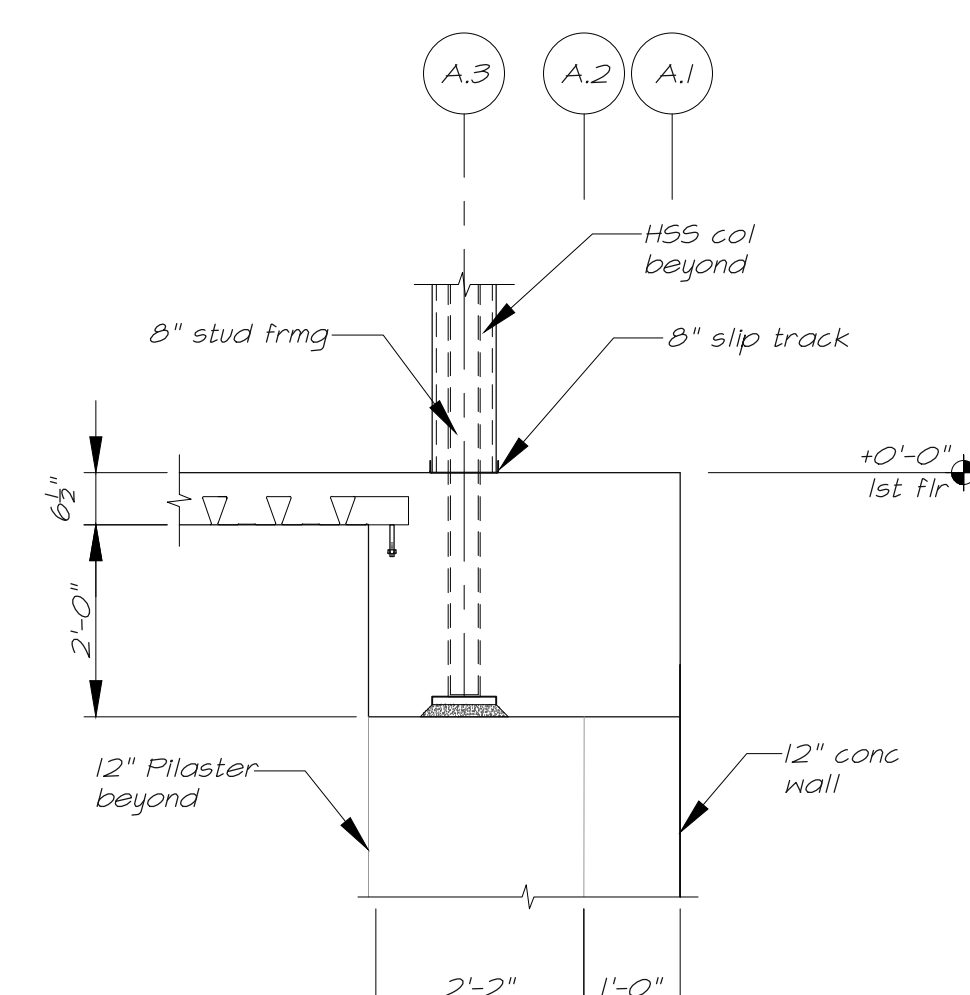
1 Section  
S4.9 1/2" = 1'-0"



2 Section  
S4.9 1/2" = 1'-0"



4 Section  
S4.9 1/2" = 1'-0"



5 Section  
S4.9 1/2" = 1'-0"



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S4.9

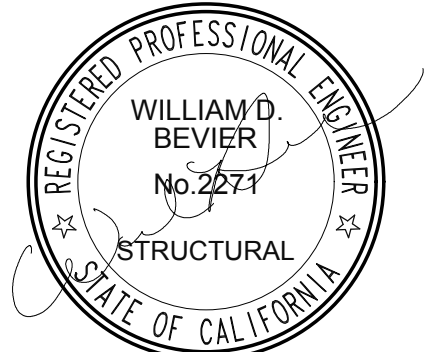




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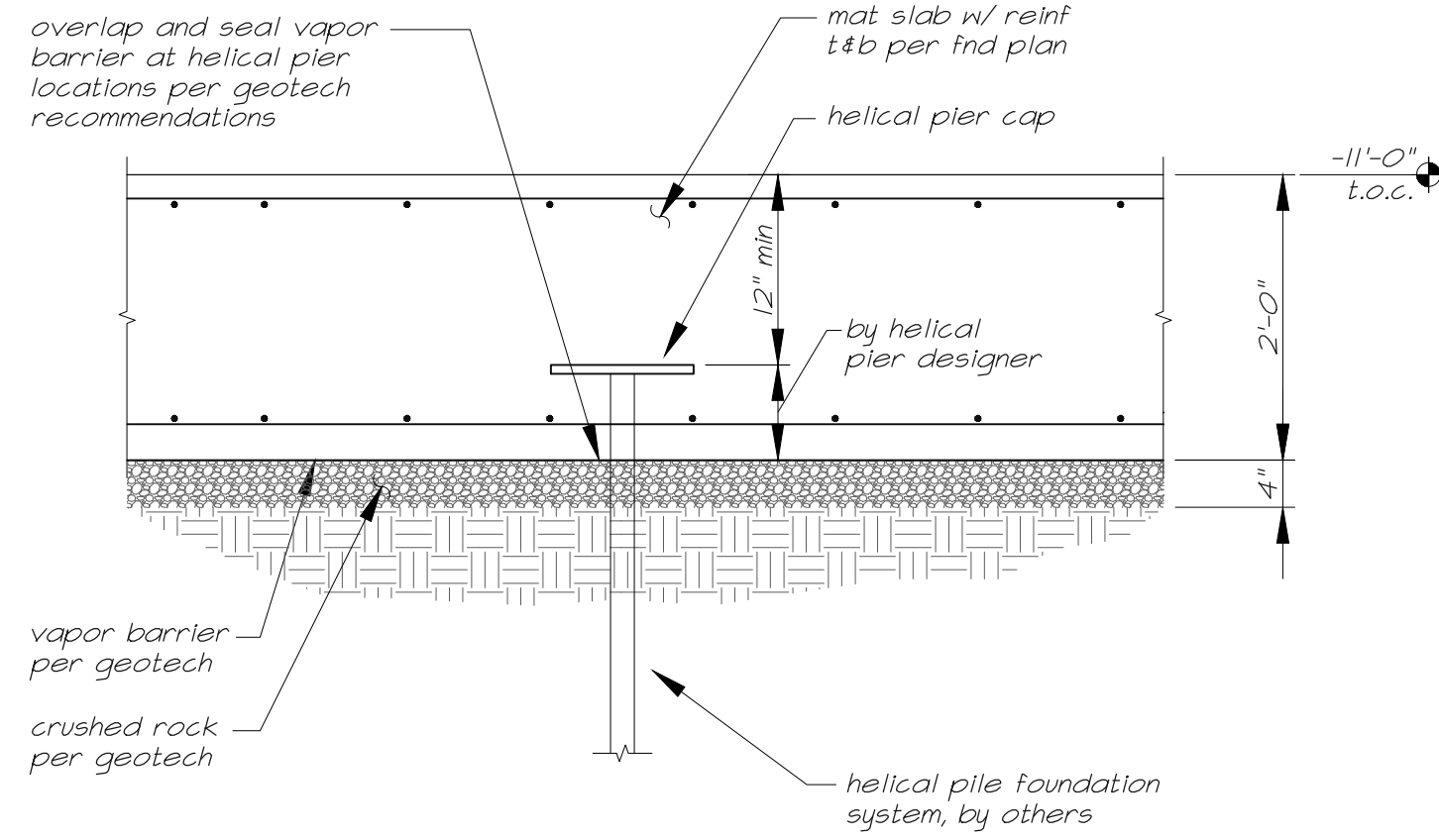
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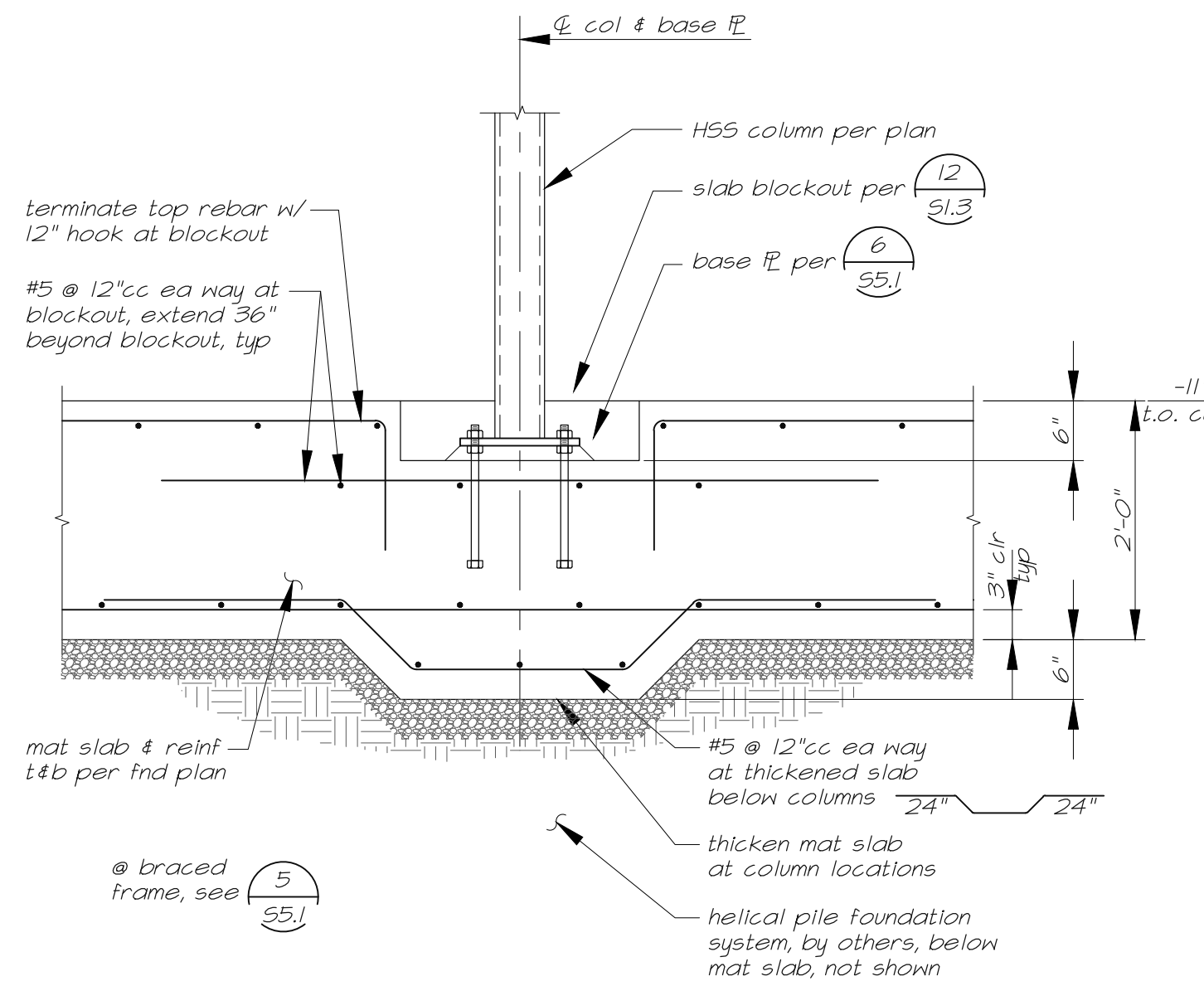
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**DETAILS**

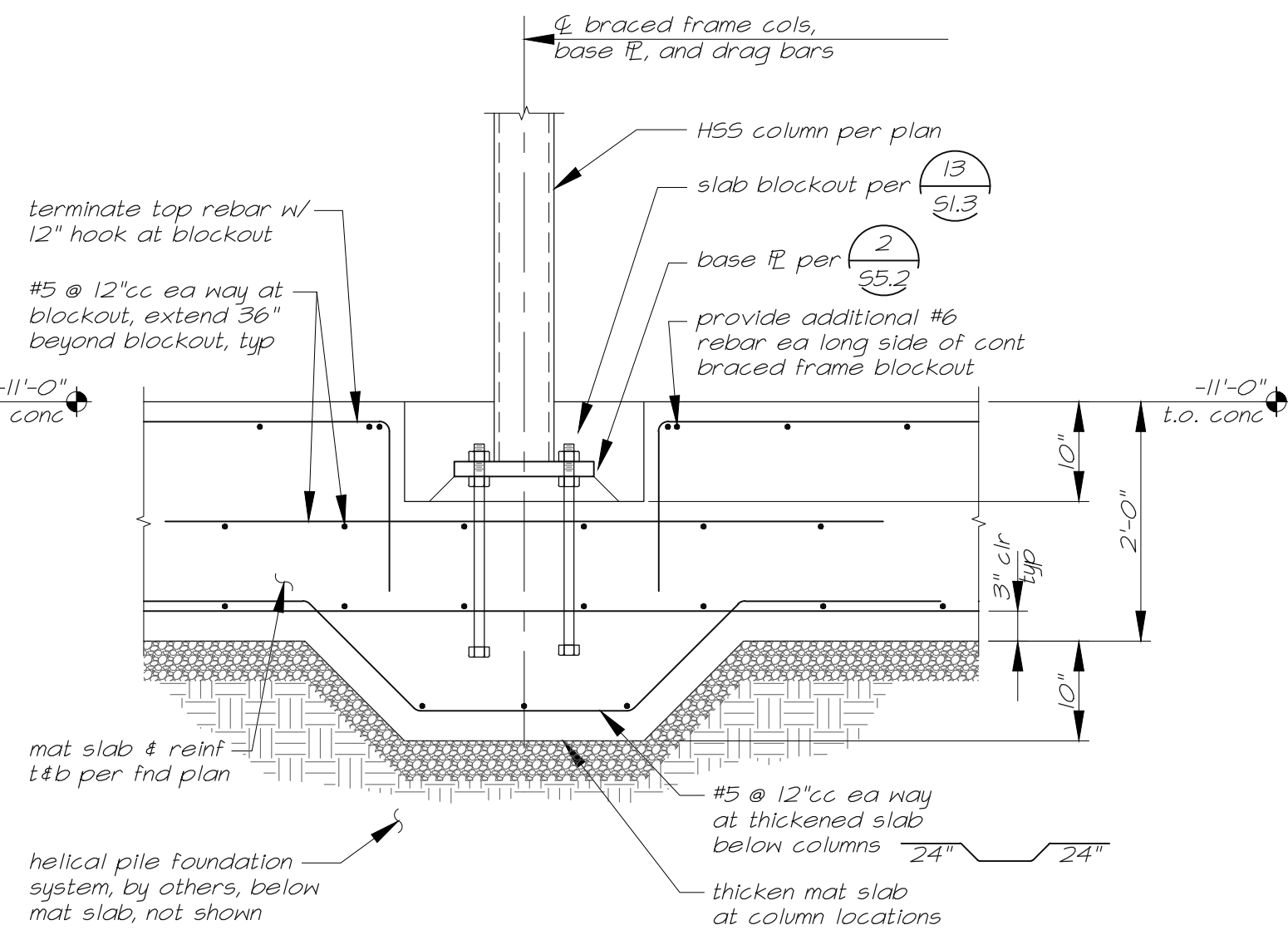
**S5.1**



**7** Detail  
 S5.1 3/4"=1'-0"



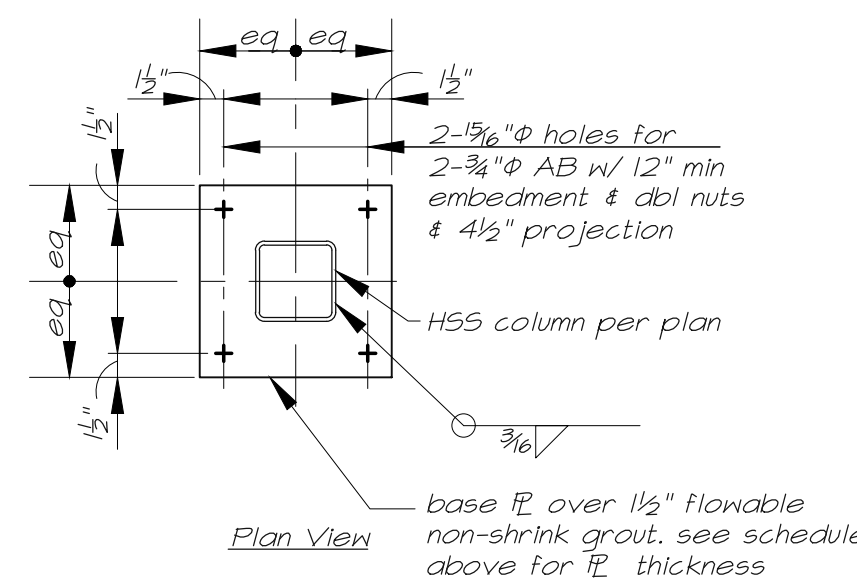
**4** Detail  
 S5.1 3/4"=1'-0" @ typ interior H55 col



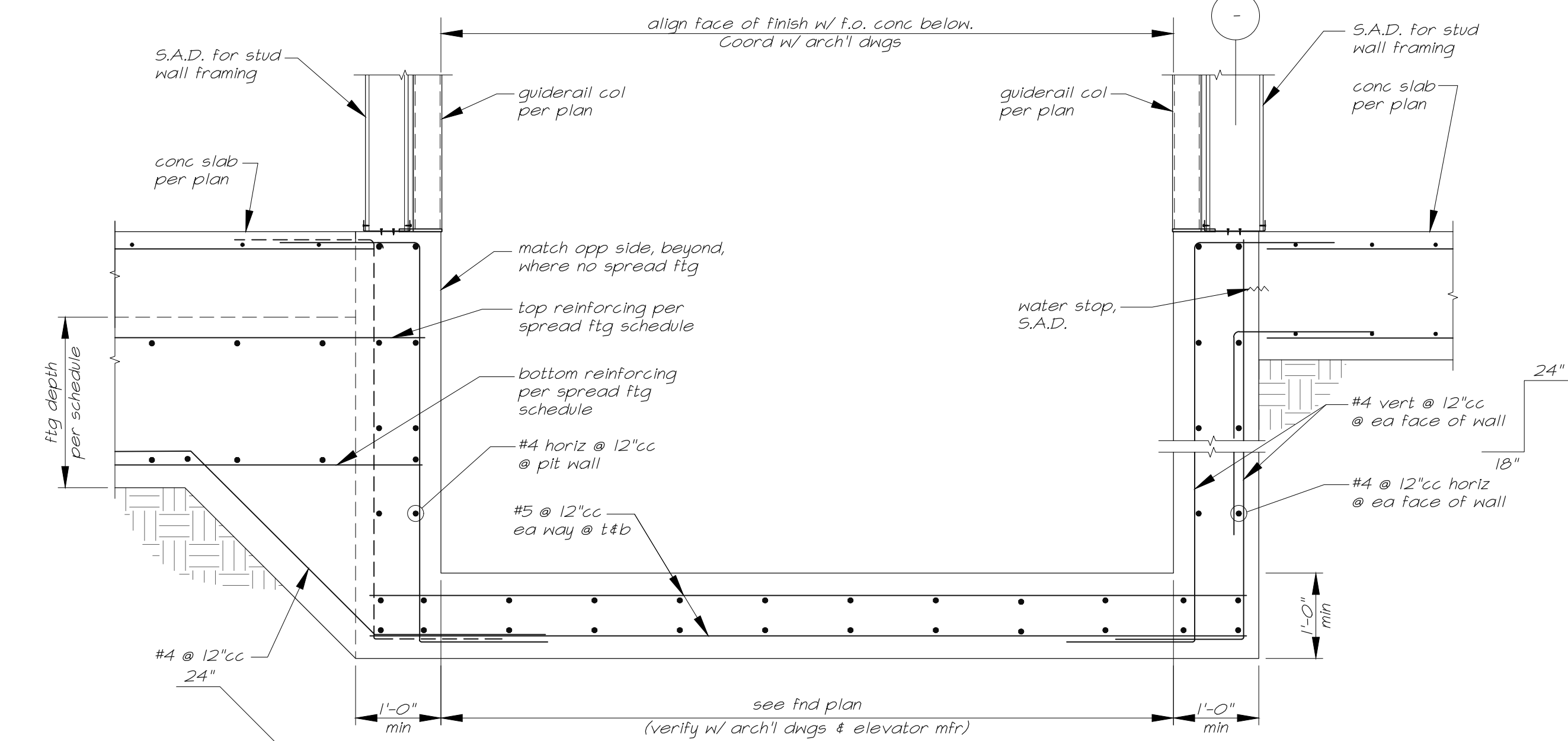
**5** Detail  
 S5.1 3/4"=1'-0" @ intr braced frame H55 col

H55 col base IR schedule		
Column Size	Base Plate Thickness	Base Plate Size
H555x5x12	1"	12" sq
H555x5x14	3/4"	12" sq
H554x4x14	3/4"	10" sq

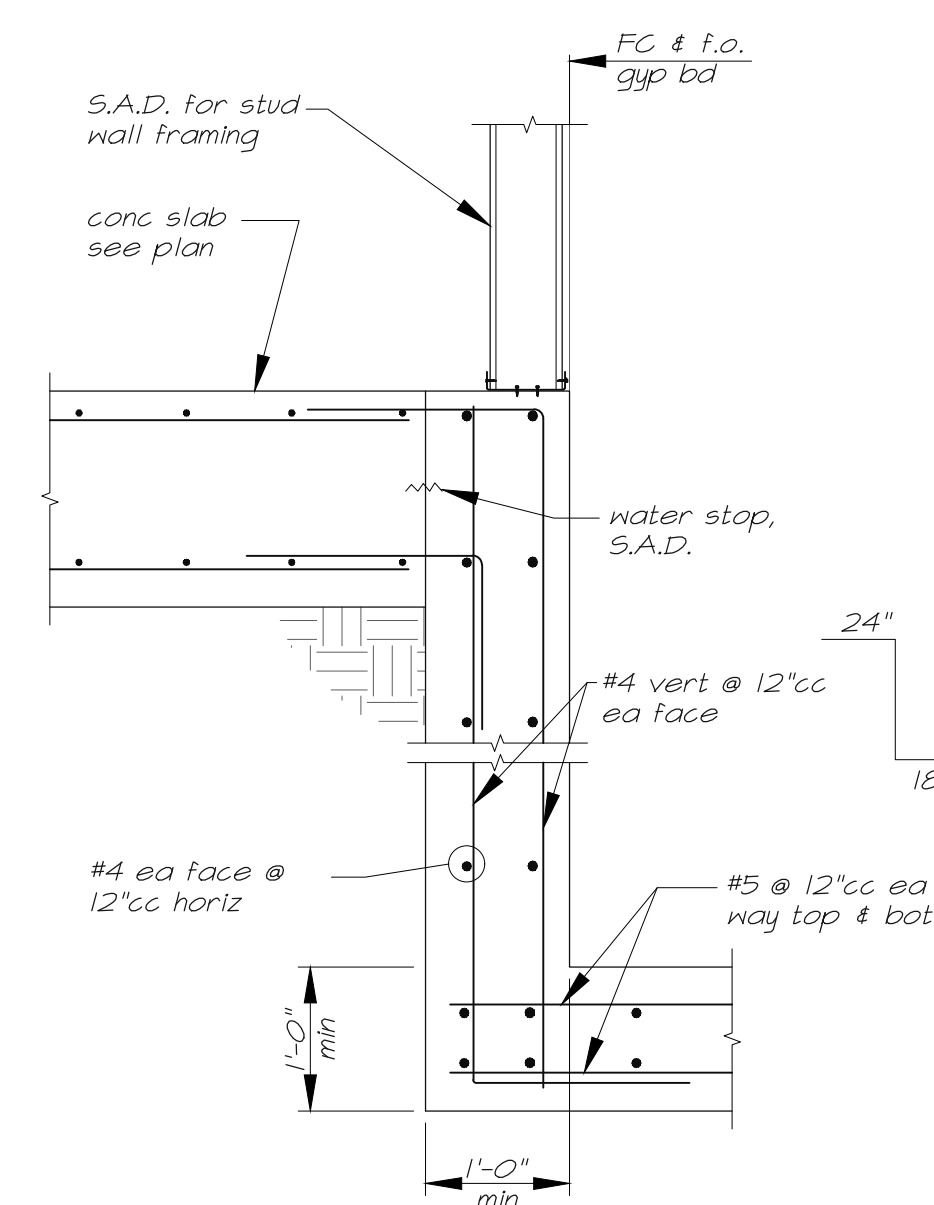
1. See sheet S5.2 for braced frame column base plates



**6** Detail  
 S5.1 1"=1'-0" typ. base IR, v.n.o.

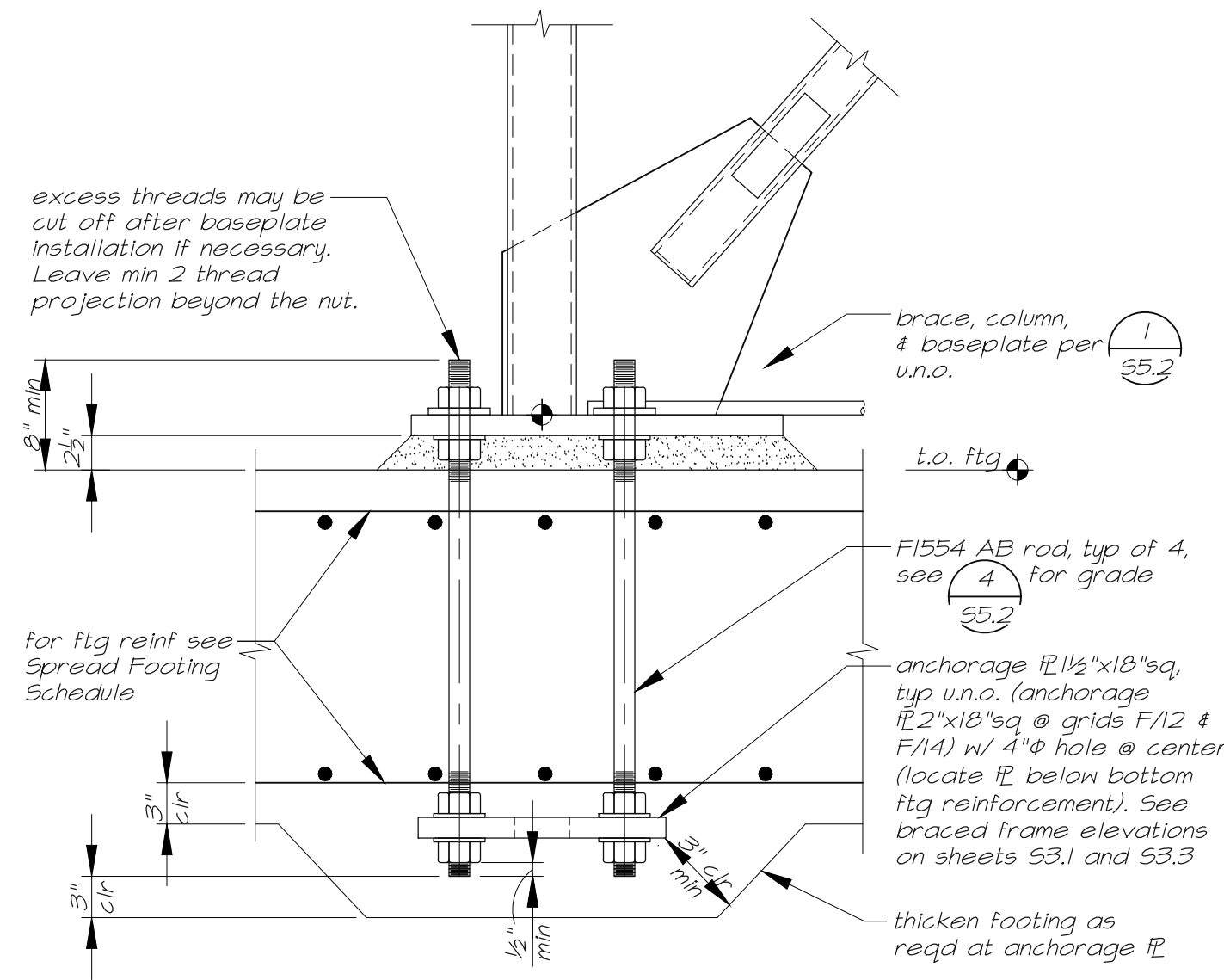


**1** Detail  
 S5.1 3/4"=1'-0" elevator pit

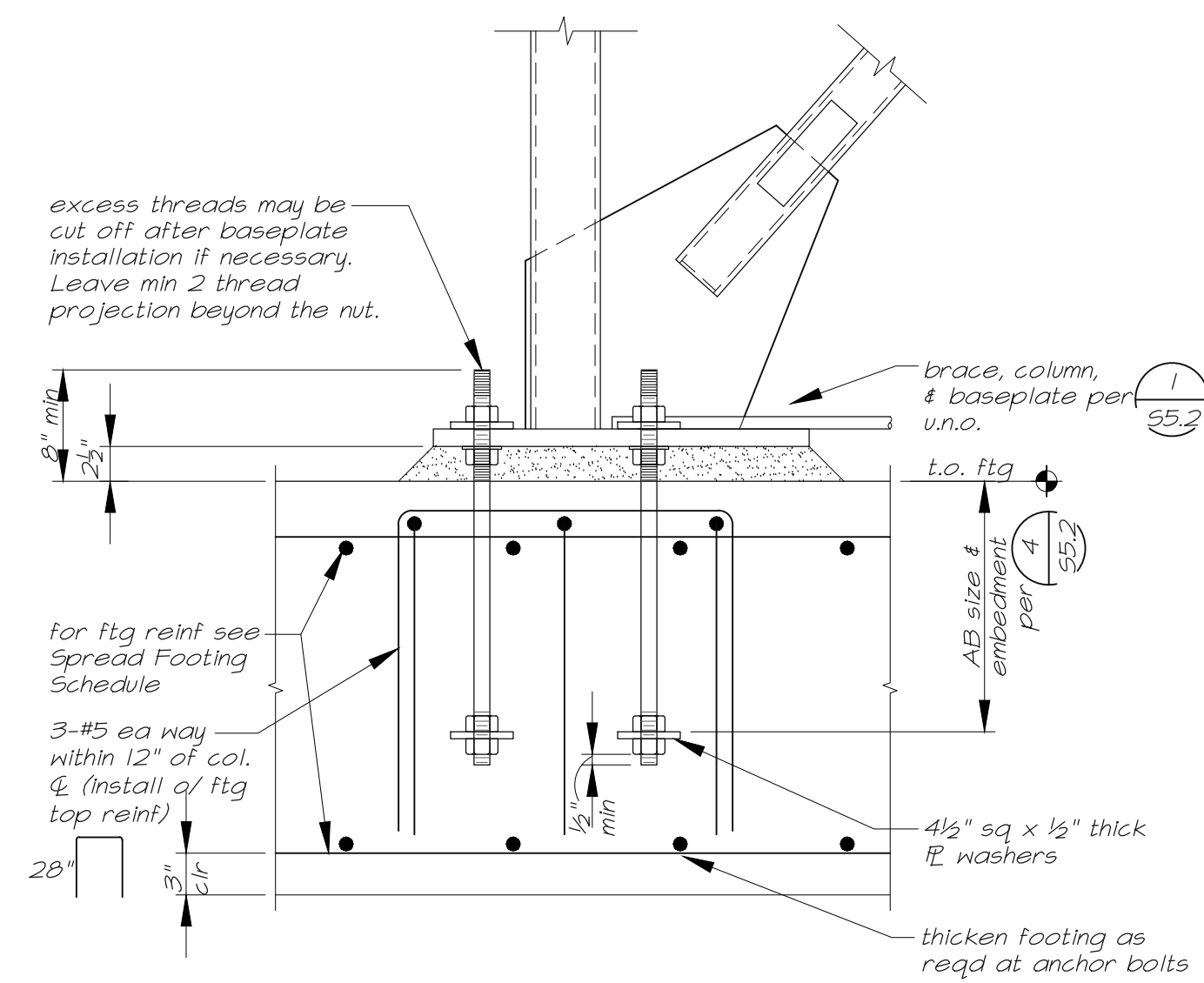


**2** Detail  
 S5.1 3/4"=1'-0" elevator pit wall

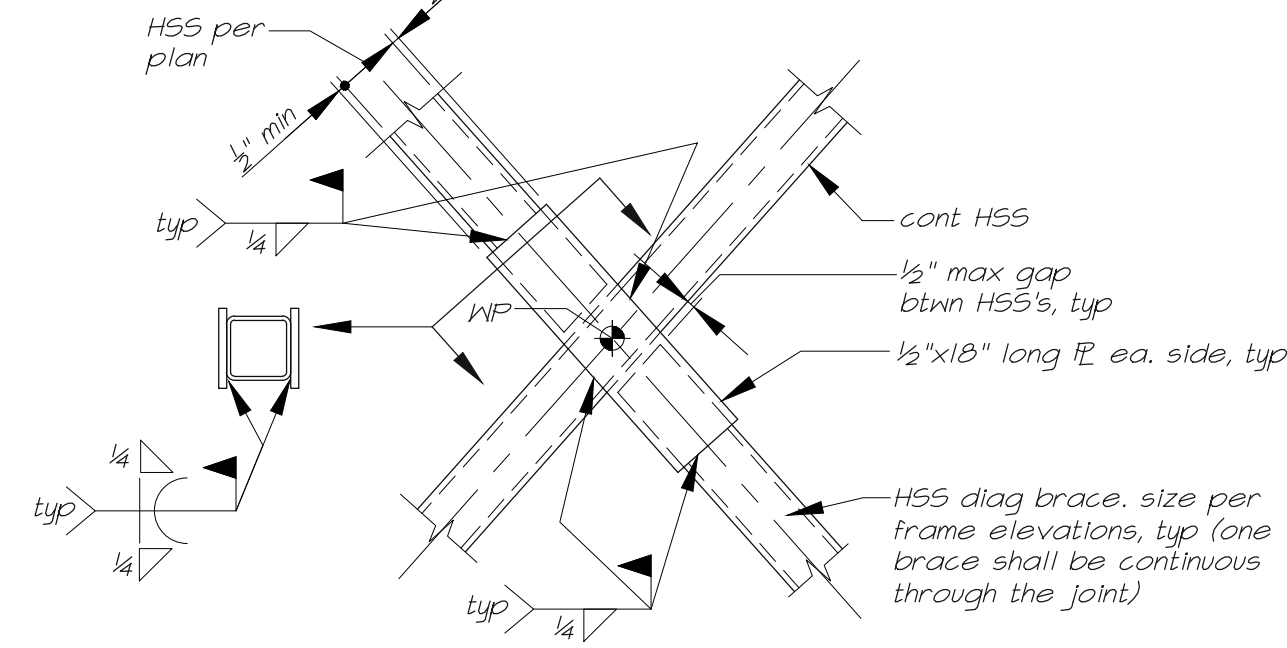




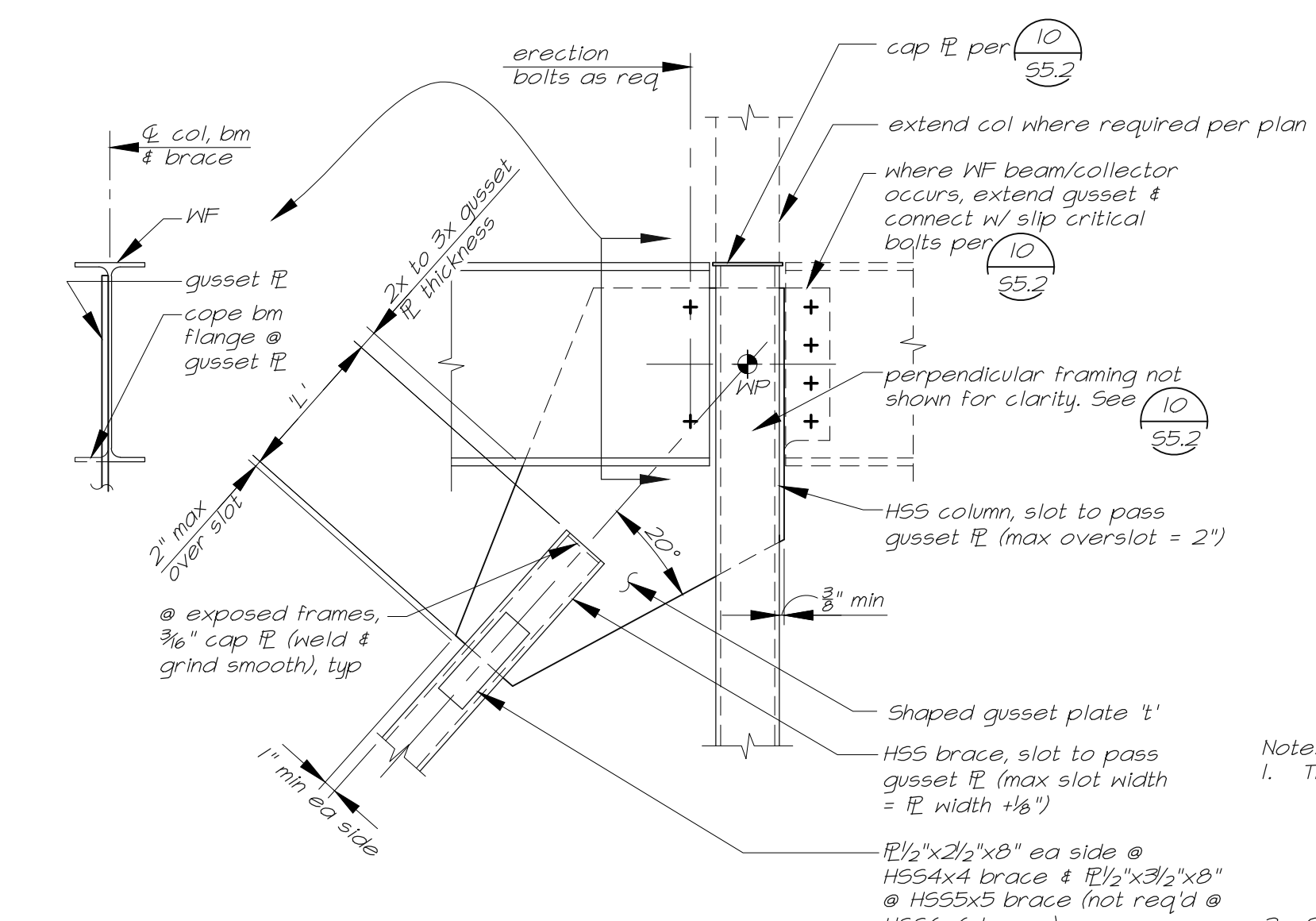
**7** Braced Frame Anchor PL  
55.2 1"=1'-0" H556x6 brace frame columns



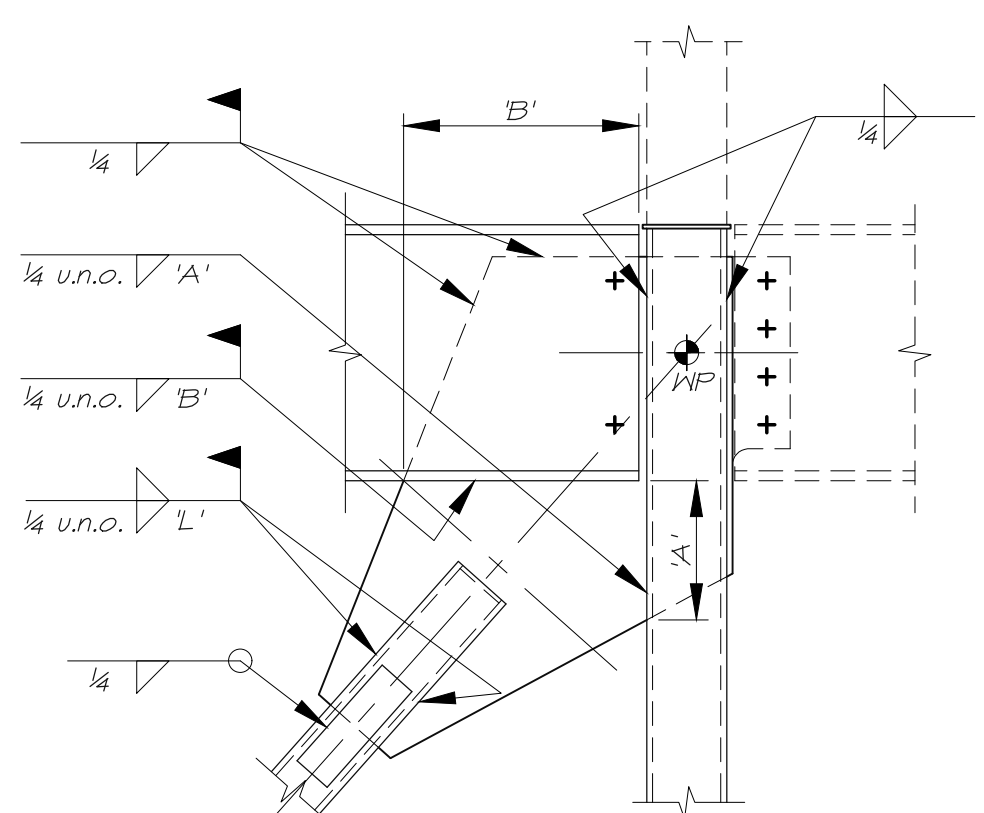
**8** Braced Frame Anchor PL  
55.2 1"=1'-0" H555x5 brace frame columns



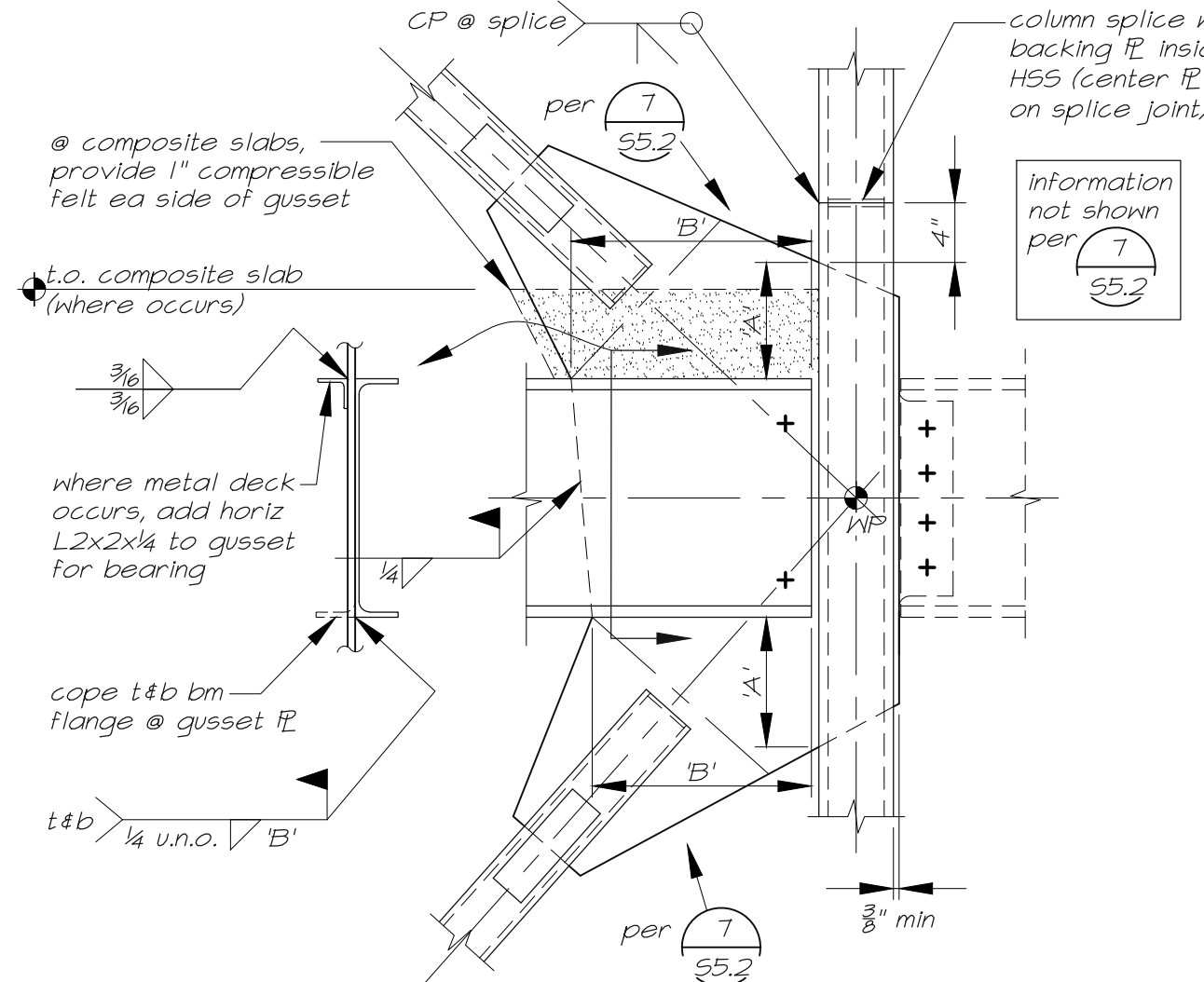
**9** Typ X-Brace Connection  
55.2 1"=1'-0"



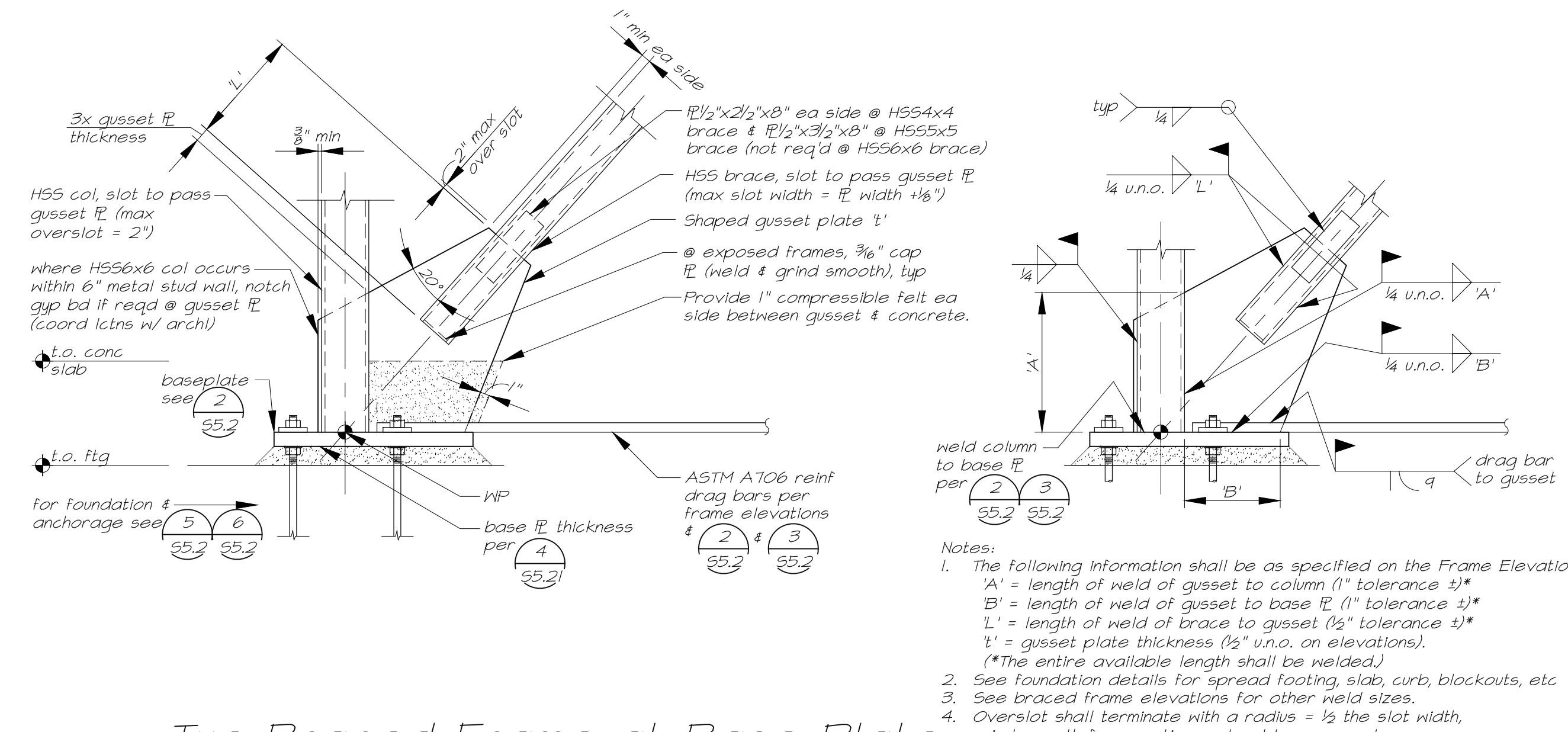
**4** Typ Braced Frame @ Column/Beam  
55.2 1"=1'-0"



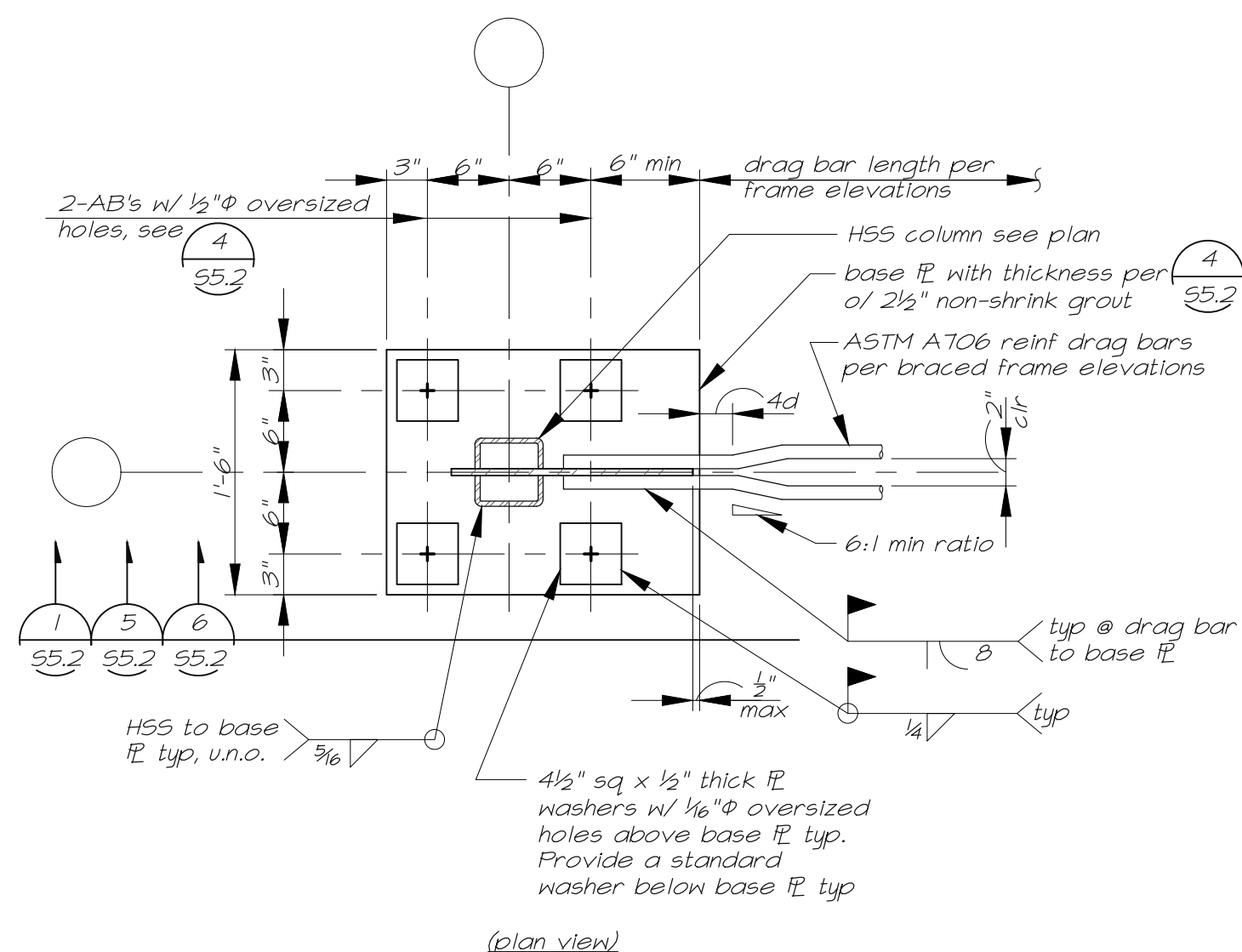
**5** Typ Brace Top & Bott @ Col  
55.2 1"=1'-0"



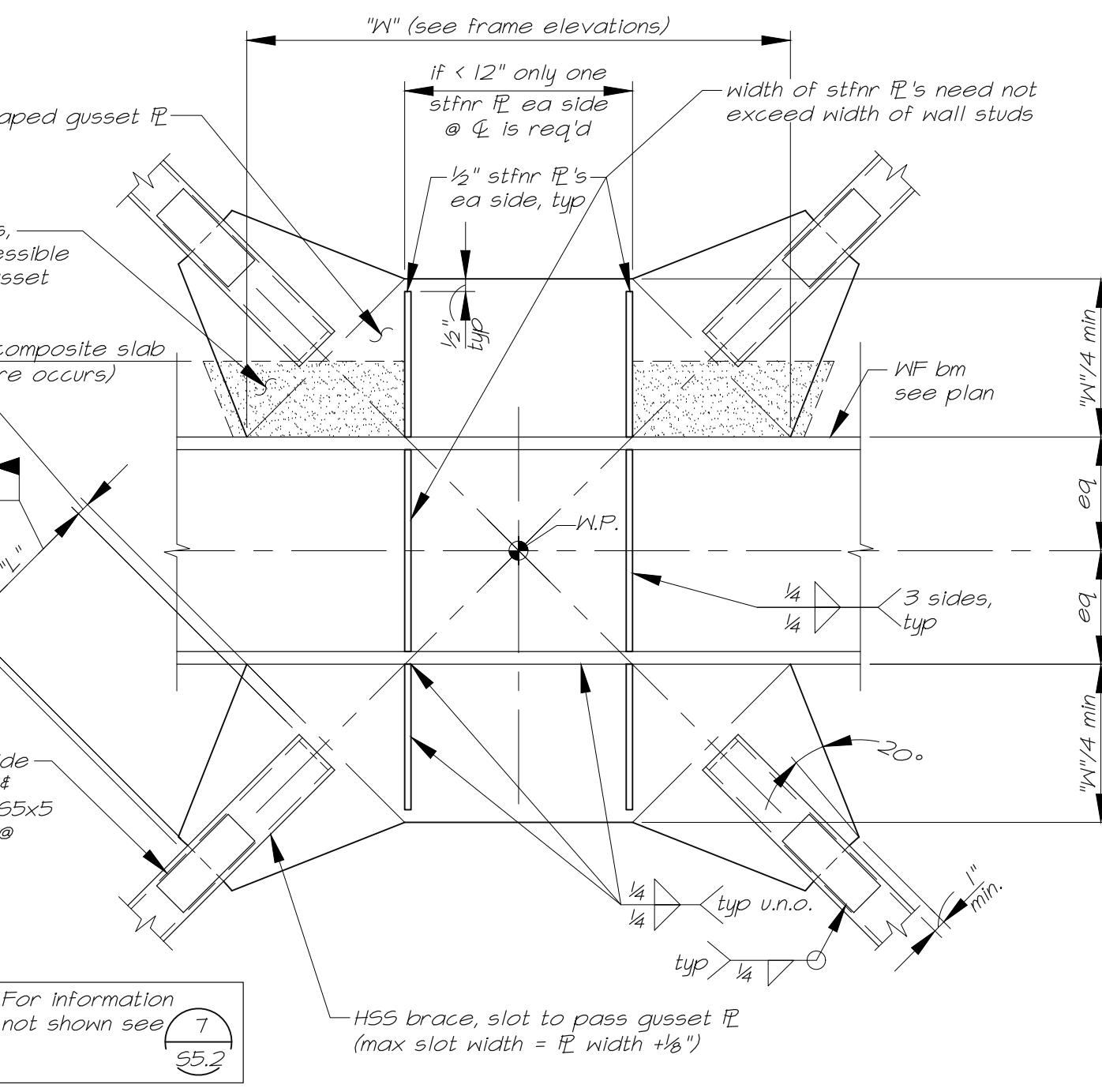
**6** Typ Midspan Conn @ Beam  
55.2 1"=1'-0"



**1** Typ Braced Frame at Base Plate  
55.2 1"=1'-0"



**2** Typ Base @ Braced Frame  
55.2 1"=1'-0"



HSS brace Base Plate Connection Schedule				
mark	column size	base PL thickness	AB size	ref. detail
1	H555x5x1/2 typ u.n.o.	1 1/2"	(4) 1 1/2"φ x 24" embedment FI554, gr 36	6/55.2
2	H556x6x3/8 typ u.n.o.	2"	(4) 1 1/2"φ FI554, gr 36	5/55.2
3	H556x6x1/2 at grids F/12 & F/14	2"	(4) 1 1/2"φ FI554, gr 55	5/55.2



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F: (209) 952-2442  
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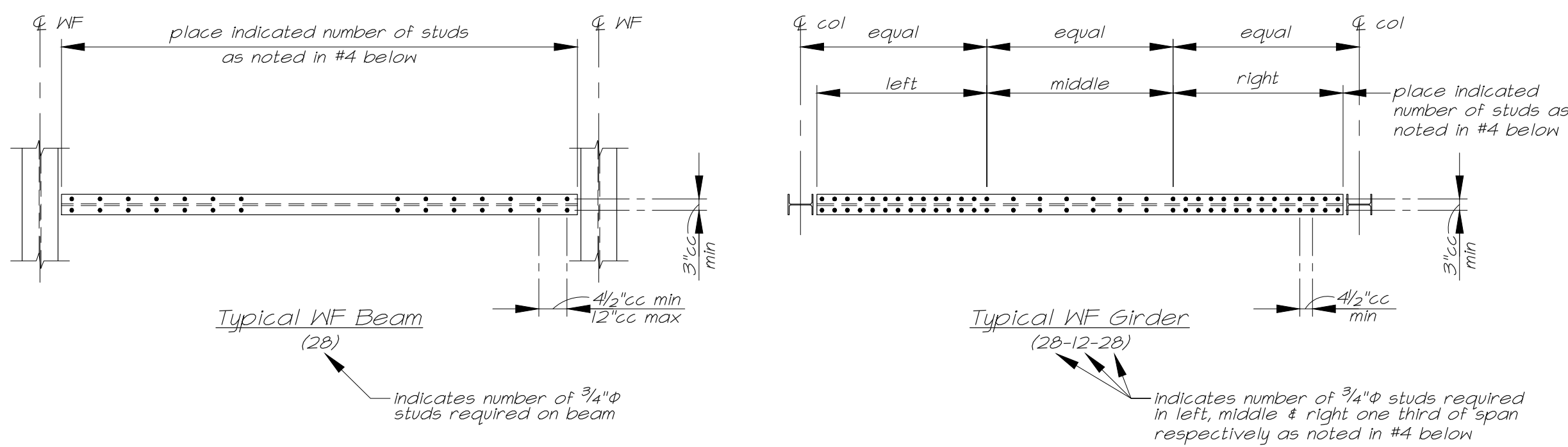
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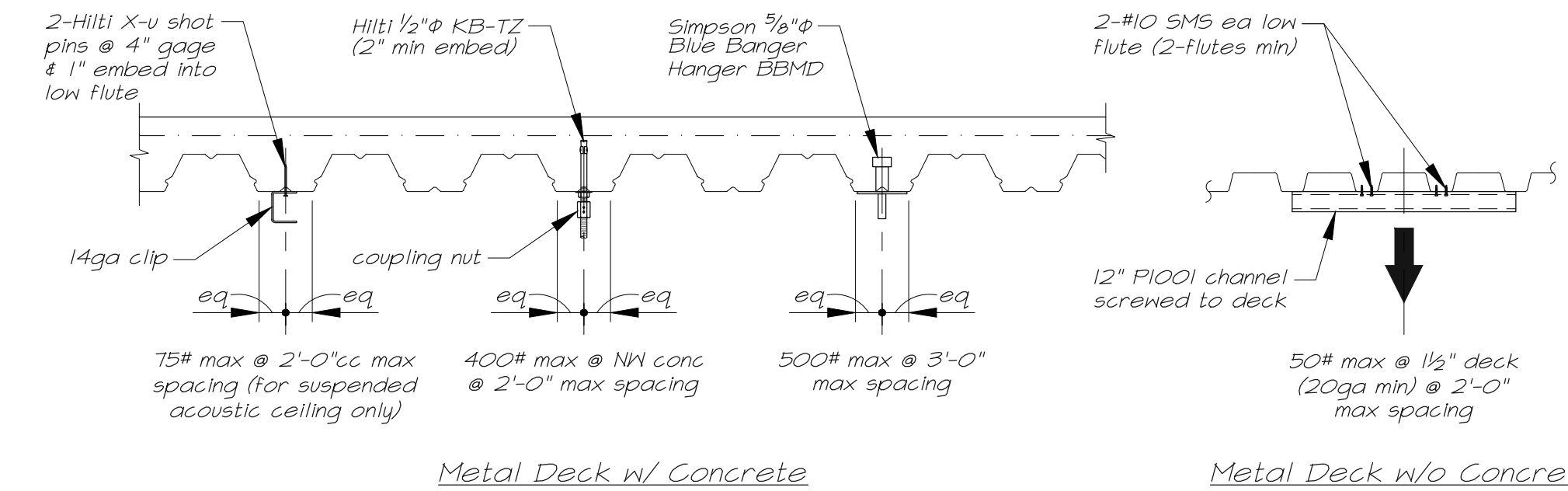
**DETAILS**





- Notes:**
1. Shear connectors for composite beams shall be automatic end welded high strength 3/4" dia headed anchors. See "Automatic End Welded Stud Notes" on typical notes sheet.
  2. All welding of shear connectors shall conform to the latest edition for "Recommended Practices for Stud Welding" and "Structural Welding Code" published by the American Welding Society.
  3. Shear connectors shall project a minimum of 1/2" above the top of the metal deck and shall be held a minimum of 3/4" clear from top of the concrete slab. Shear connectors shall be uniformly spaced. Use no more than one stud per rib where the number of shear connectors required is less than or equal to the number of ribs available. Where the number of shear connectors exceeds the number of ribs available, place one shear connector per rib beginning at the supports at each end on moving toward mid-span until the required number of shear connectors are supplied.
  4. Where not indicated otherwise, provide 3/4" dia shear connectors @ 8" cc maximum spacing @ all beams receiving metal deck and concrete fill.
  5. Contractor shall submit shop drawings for review prior to install.

8 Shear Connector Layout  
S5.3 1/4"=1'-0"



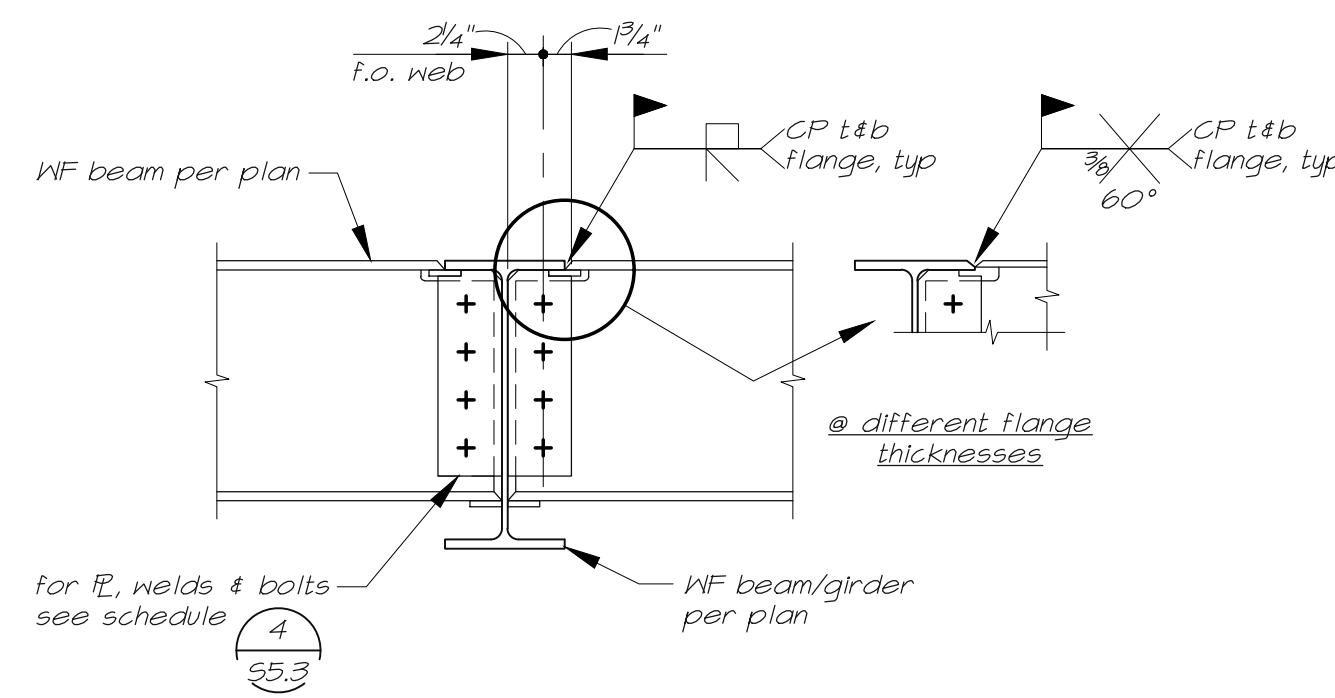
Metal Deck w/ Concrete  
Metal Deck w/o Concrete

9 Detail  
S5.3 1"=1'-0" hangers @ metal deck

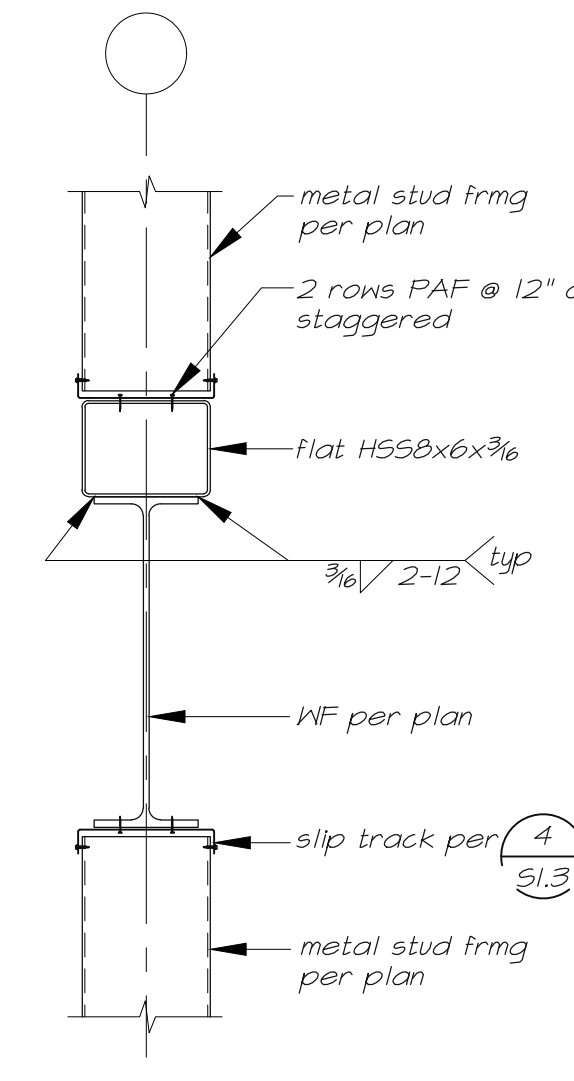
bm size	No. & Dia. A325-N Bolts per row, u.n.o.	shear PL thickness	W
CB, CB & C10	2 - 3/8" dia	1/4"	1/4"
WB & W10	2 - 3/8" dia	1/4"	1/4"
W12 & W14	3 - 3/8" dia	3/16"	1/4"
W16	4 - 3/8" dia	3/16"	1/4"
W18	5 - 3/8" dia	3/16"	1/4"
W21	5 - 3/8" dia	3/16"	5/16"
W24	6 - 3/8" dia	1/2"	3/16"
W27	7 - 3/8" dia	1/2"	3/16"
W30	7 - 3/8" dia	1/2"	3/16"
W33	8 - 3/8" dia	1/2"	3/16"

- Note:**
1. Use A325-N bolts at connections, typ. A325 SC group A bolts are to be used at specific locations as indicated on framing plans. Use multiple rows of no. & dia. shown in schedule to achieve total number of bolts specified on plans. All slip critical connections shall have full-depth shear plates.

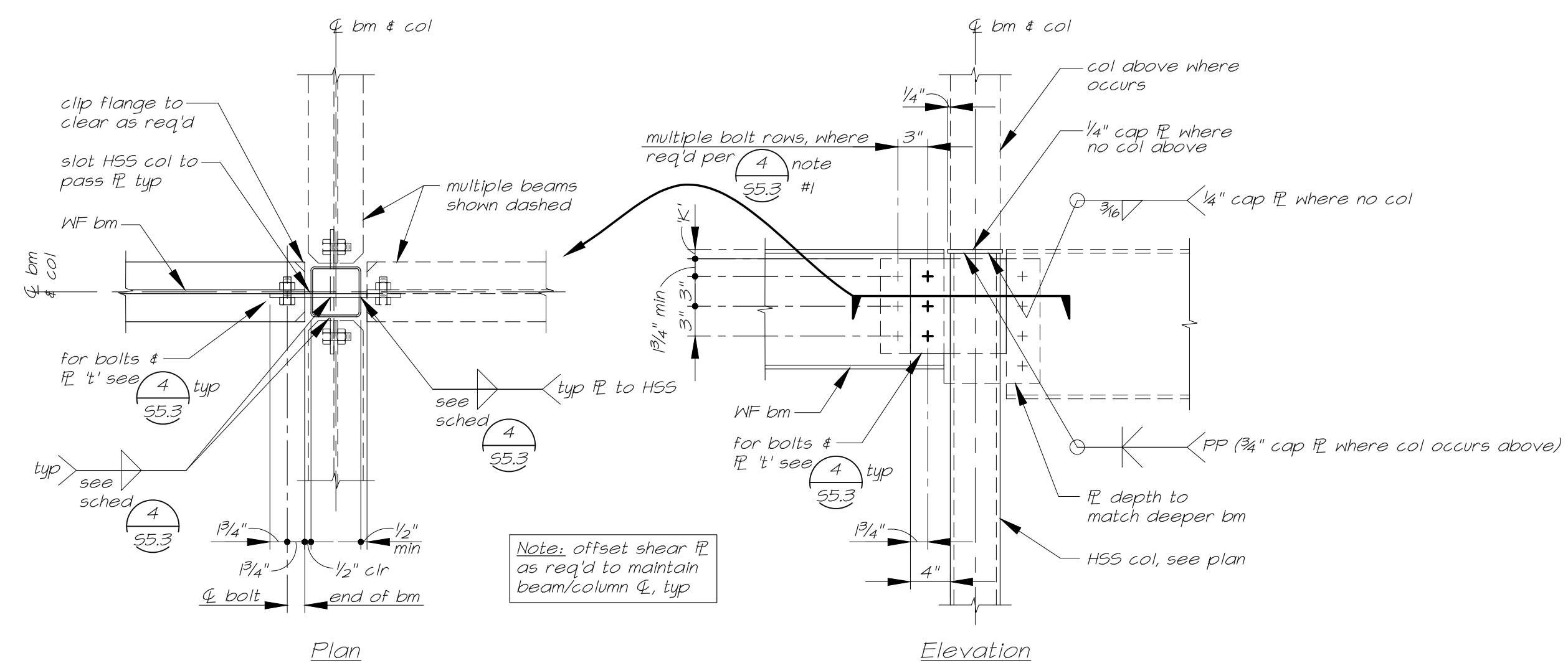
4 Connection Schedule  
S5.3 n.t.s.



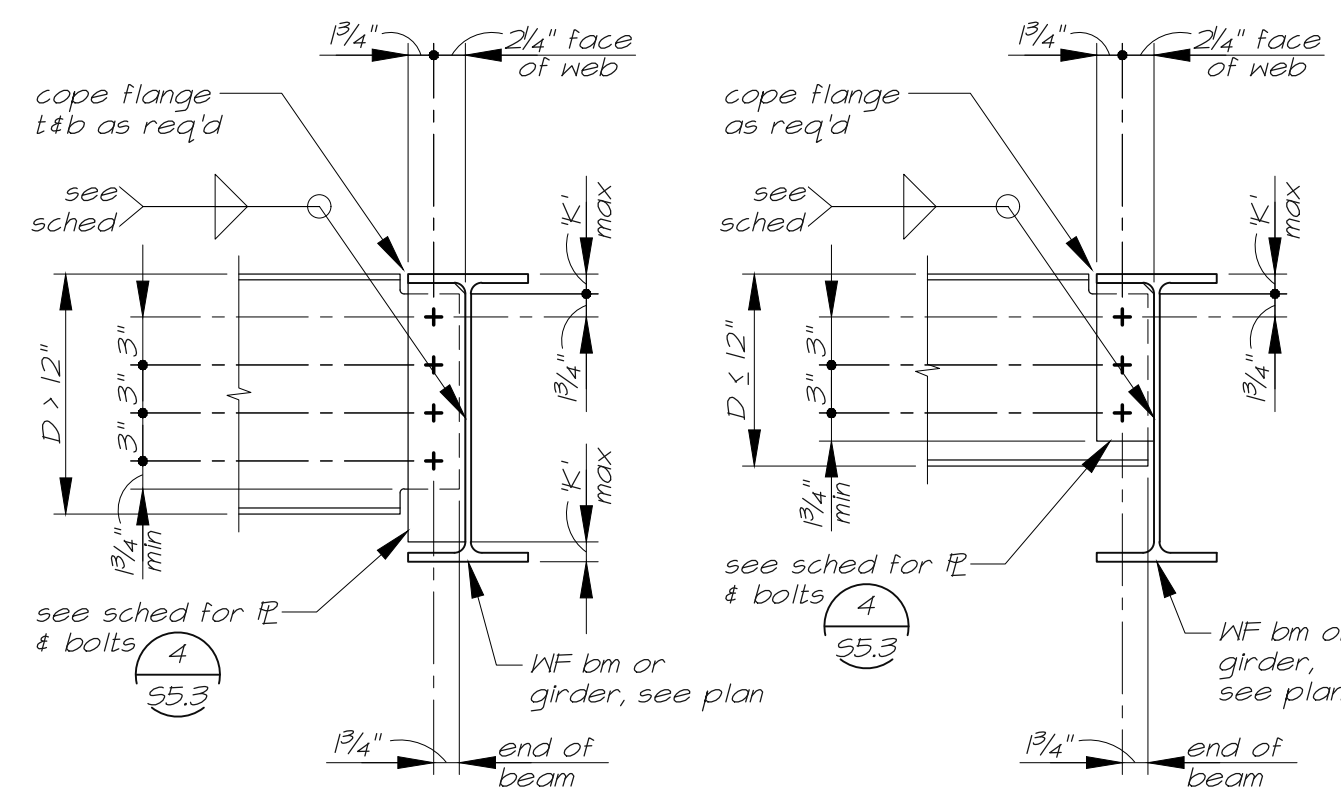
5 Detail  
S5.3 1"=1'-0" moment connection - beam to beam



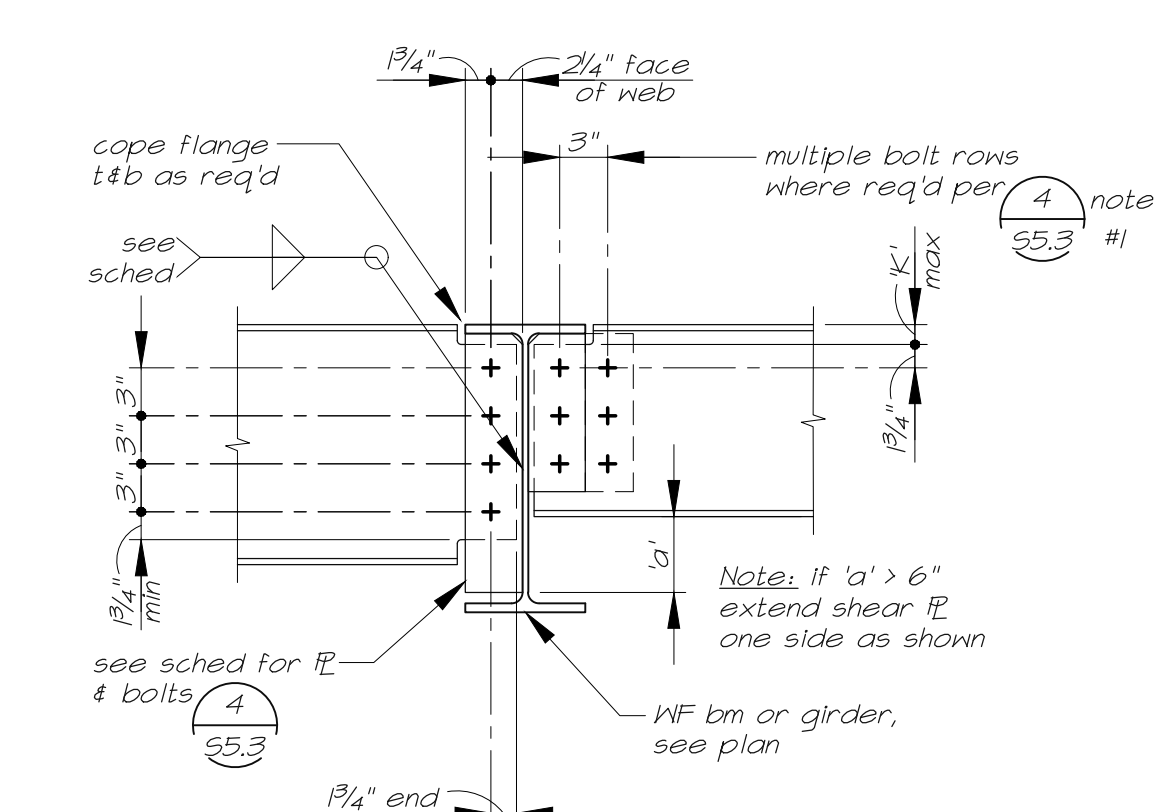
6 Detail  
S5.3 1"=1'-0" wind beam @ stairwell



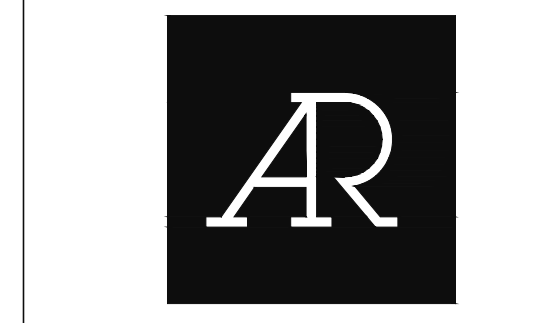
1 Beam to Column Connection  
S5.3 1"=1'-0" one or multiple sides



2 Beam to Beam Connection  
S5.4 1"=1'-0" one sided



3 Beam to Beam Connection  
S5.4 1"=1'-0" two sided



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**DETAILS**

**S5.3**



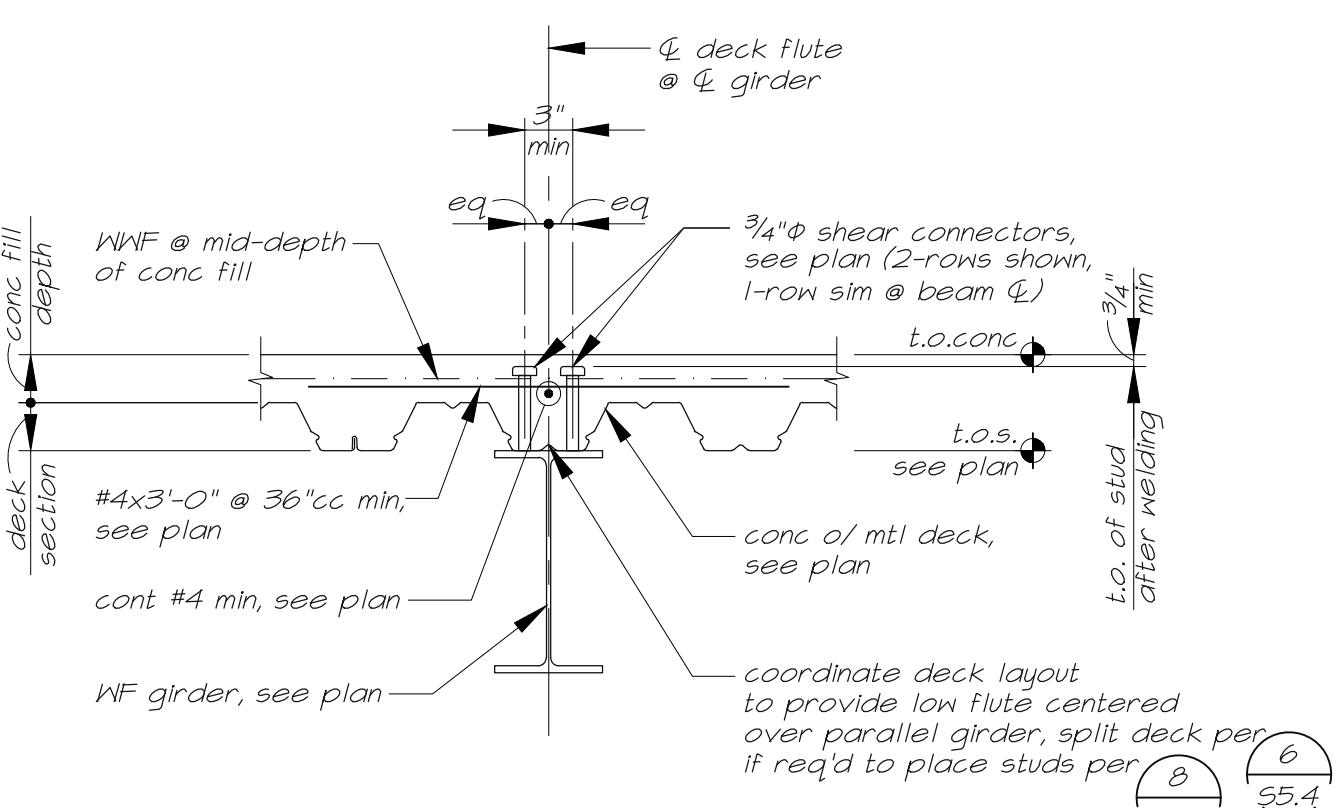
## 5 Deck Fastening Schedule

Deck Type	Depth & Gauge	Perpendicular Supports	Parallel Supports	Side Laps
3.5D-24 FormLok (floor)	3/2"x16 ga	4 P.N. per sheet	P.N. @ 12"cc	#12 @ 24"cc
3.5D-24 deck (roof)	3/2"x18 ga	4 PAF's per sheet	PAF @ 12"cc	#12 @ 24"cc
B deck (stair landing)	1/2"x16 ga	4 PAF's per sheet	PAF @ 12"cc	VSG2 @ 24"cc

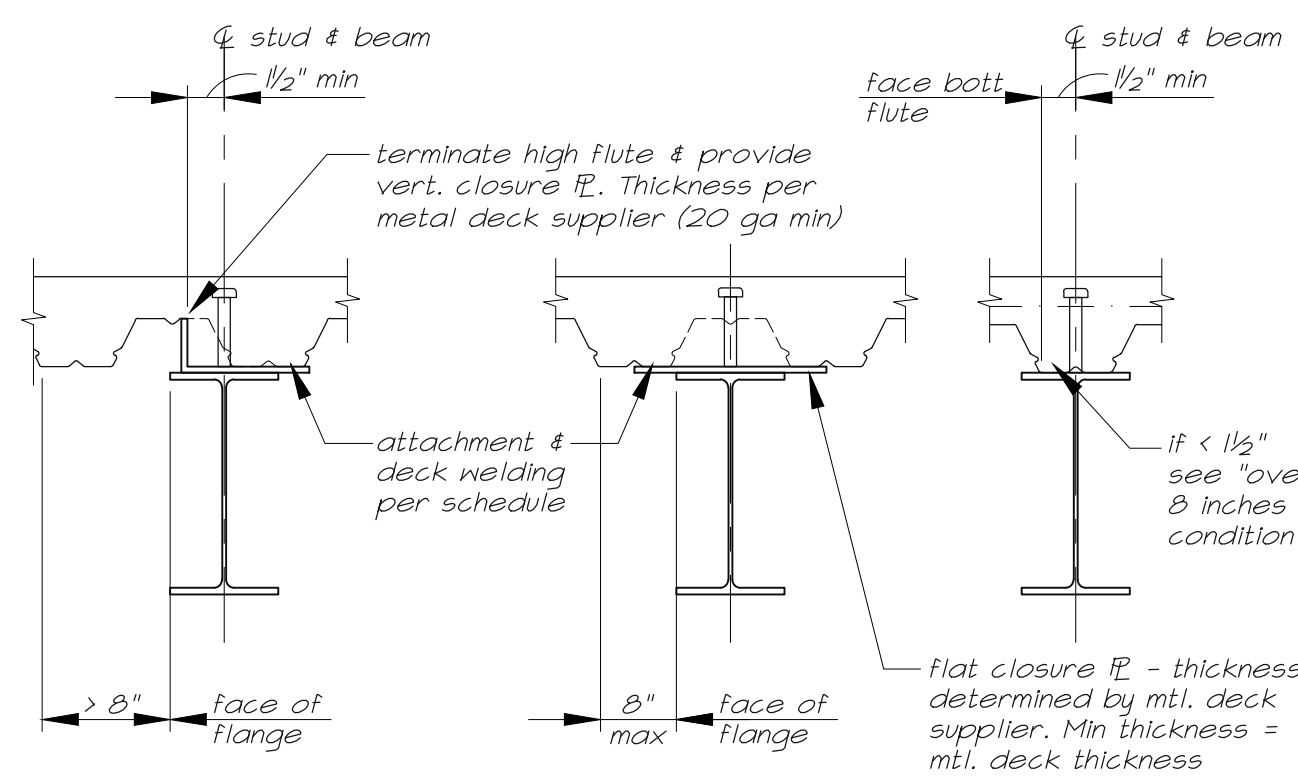
1. P.N. = 1"Ø (visible size) puddle weld
2. PAF = Hilti Powder Actuated Fastener. See PAF notes on sheet S1.1
3. VSG2 = side lap connection w/ Verco PunchLok II system, (or ASC Delta Grip system, or approved equal).
4. Metal decking shall be Verco or ASC, of type and gauge shown on plans and fastened as shown above.
5. See below for typical fastener patterns.
6. No conduit or non-structural items may be placed in concrete over metal deck.
7. At composite floor decks, 3/4"Ø shear studs may be substituted for P.N.
8. 2" minimum bearing required @ all perpendicular supports.

## Metal Deck Fastener Patterns

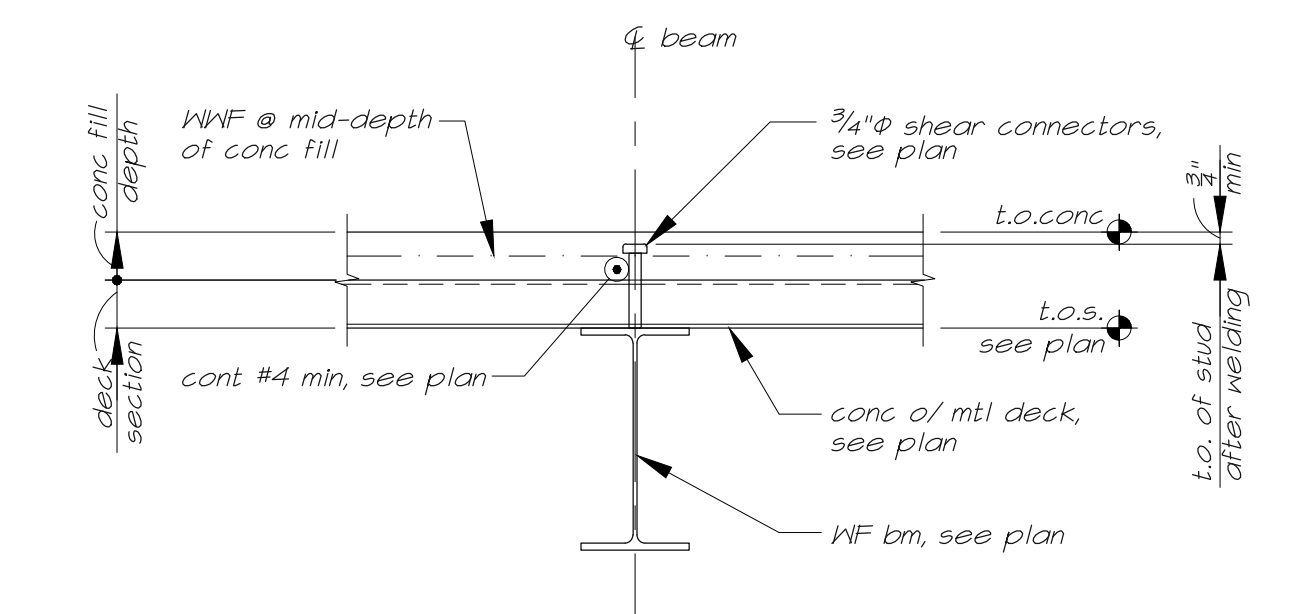
Deck Type	Profile	No. fasteners per sheet
3.5D-24 deck (floor & roof)		4
B deck (stair landing)		4



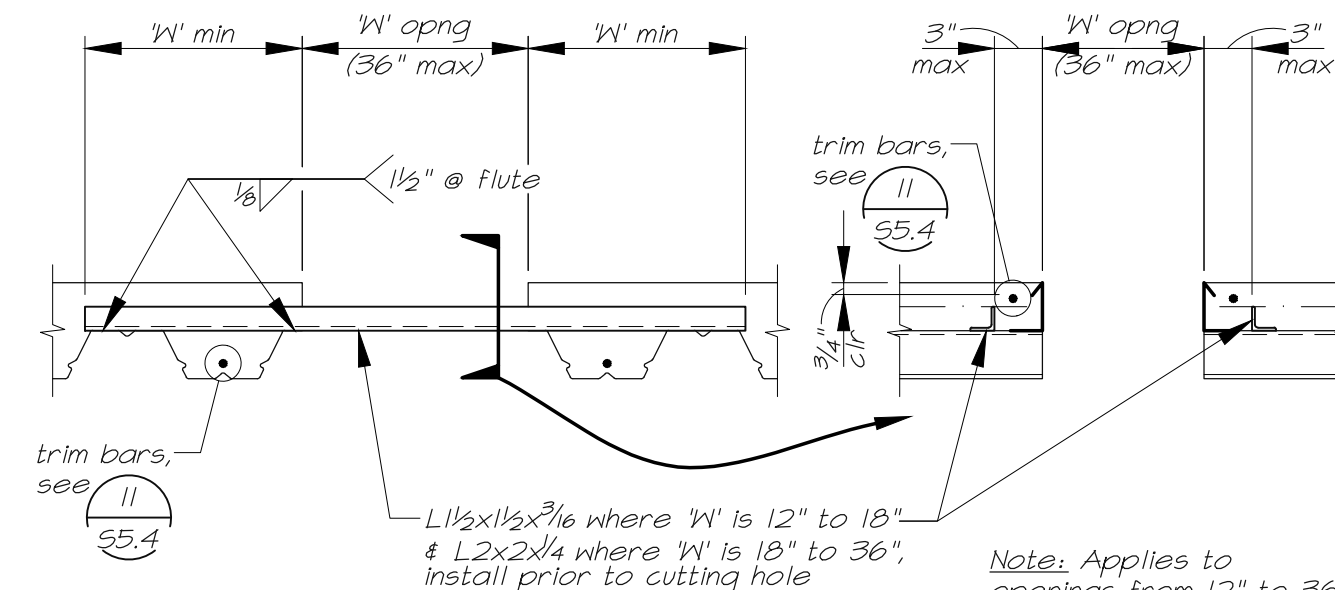
1 Floor @ Girder Bearing  
S5.4 1"=1'-0"



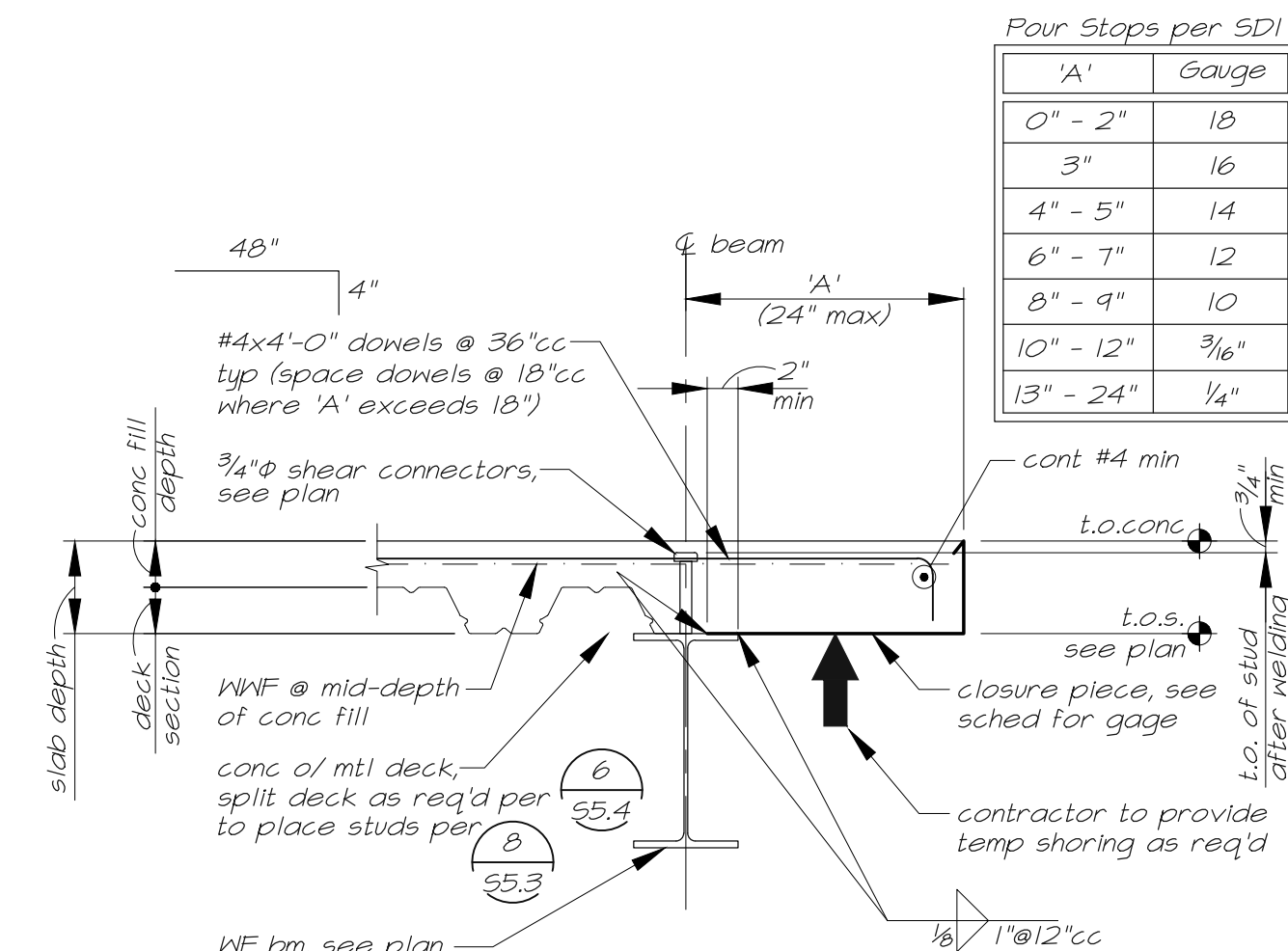
6 Metal Deck Parallel to Beam  
S5.4 1"=1'-0"



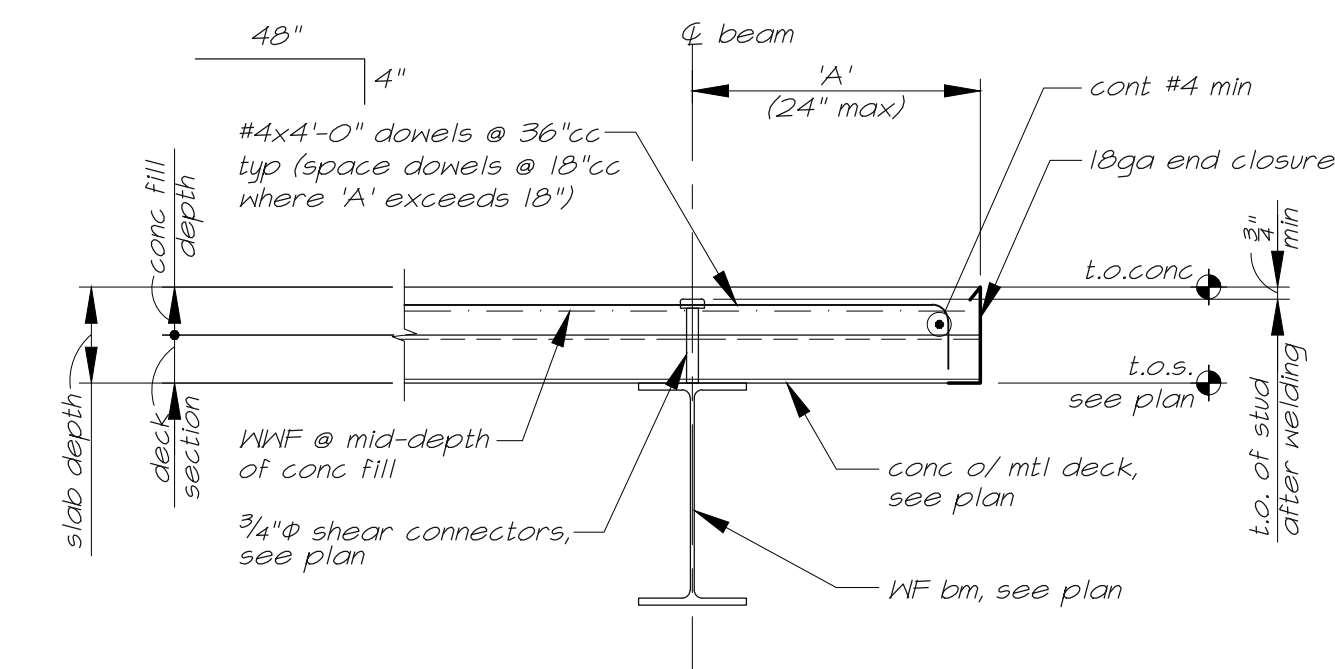
2 Floor @ Beam Bearing  
S5.4 1"=1'-0"



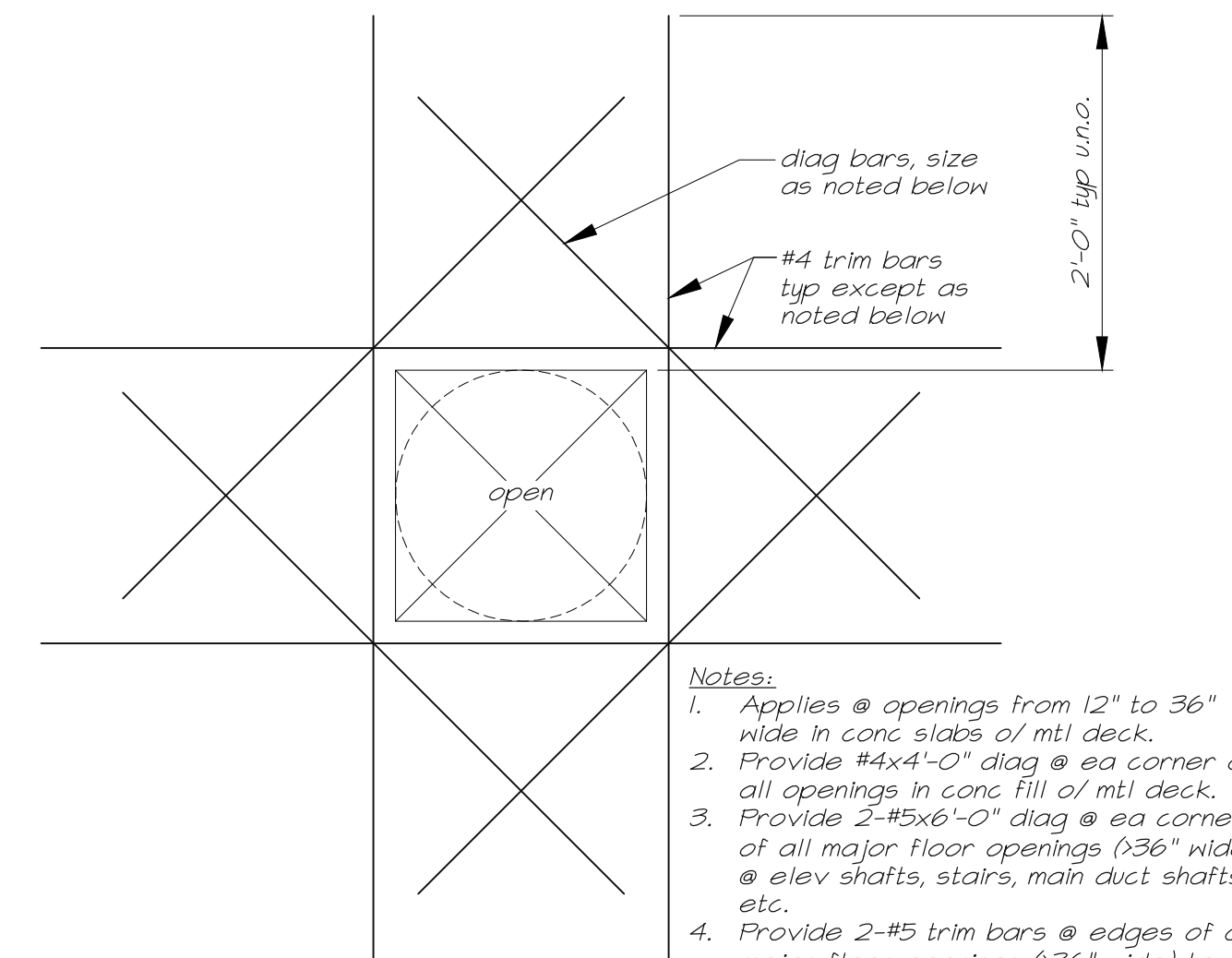
10 Opening in Metal Deck  
S5.4 1"=1'-0" w/ conc fill (opng from 12" to 36")



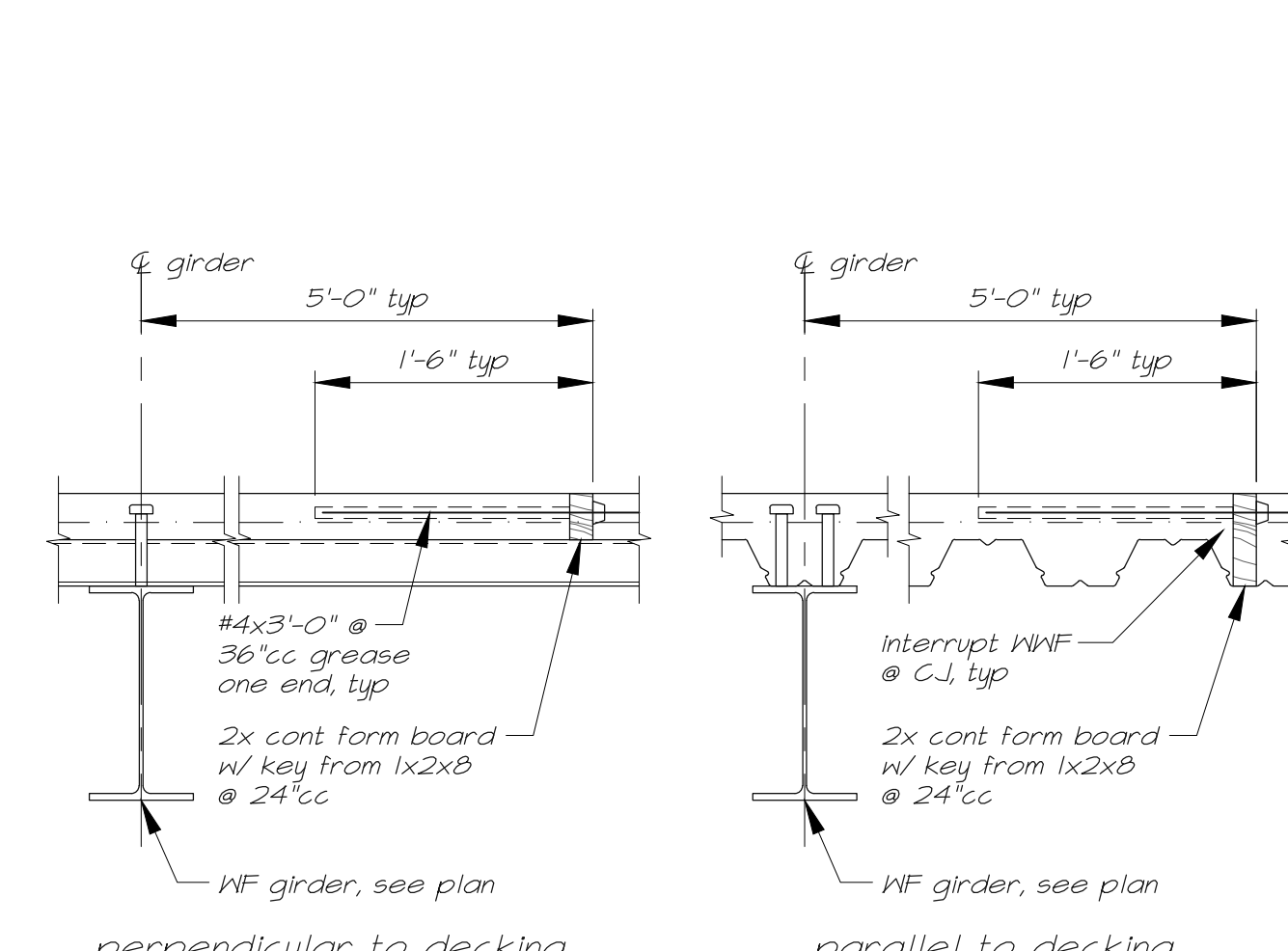
7 Floor @ Beam Bearing  
S5.4 1"=1'-0" parallel to slab edge



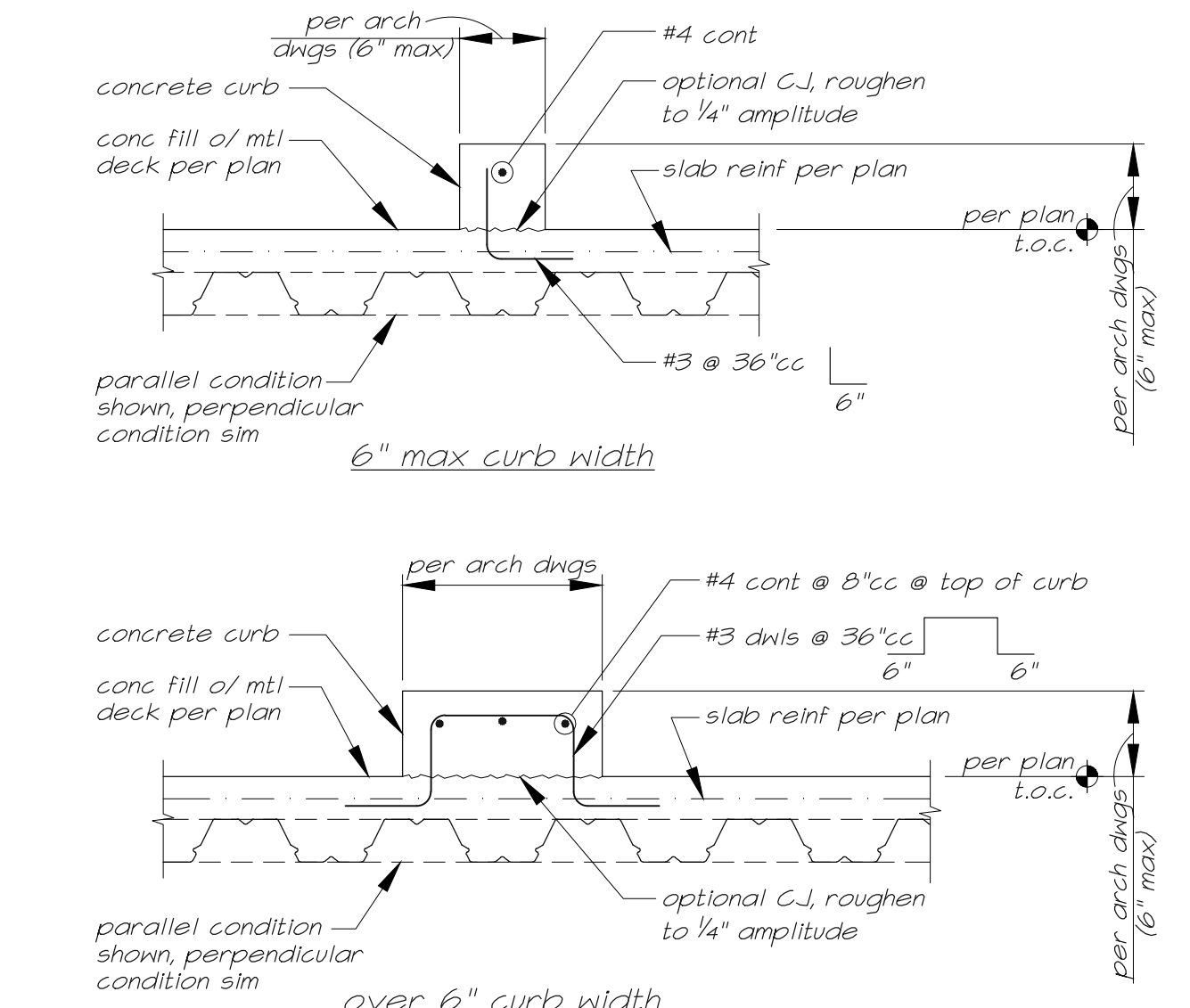
3 Floor @ Beam Bearing  
S5.4 1"=1'-0" perpendicular to slab edge



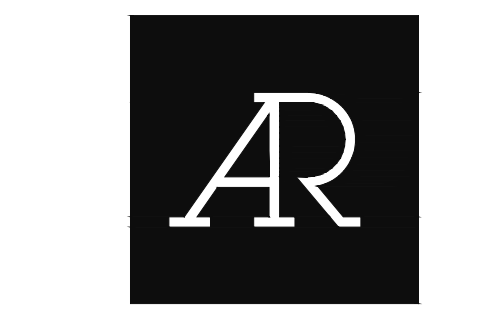
11 Opening in Metal Deck  
S5.4 1"=1'-0" w/ conc fill (opng from 12" to 36")



8 Construction Joint  
S5.4 1"=1'-0"



4 Detail  
S5.4 1"=1'-0" conc curb @ mtl deck



555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@archtechnica.net  
www.archtechnica.net

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2479 Sunrise Blvd.  
Gold River, CA 95670  
Tel: (916) 631-3030  
Fax: (916) 631-8996  
Web: www.bevier.net  
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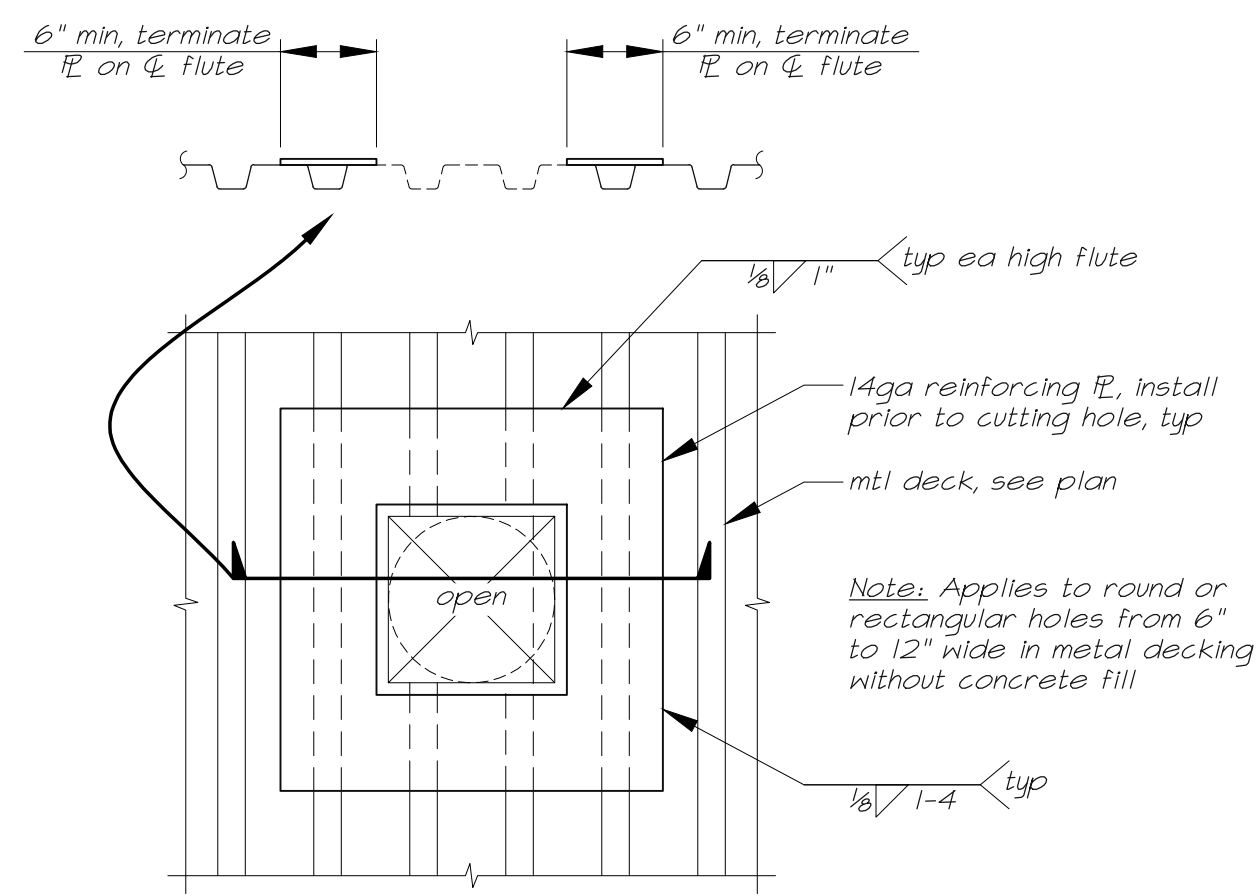
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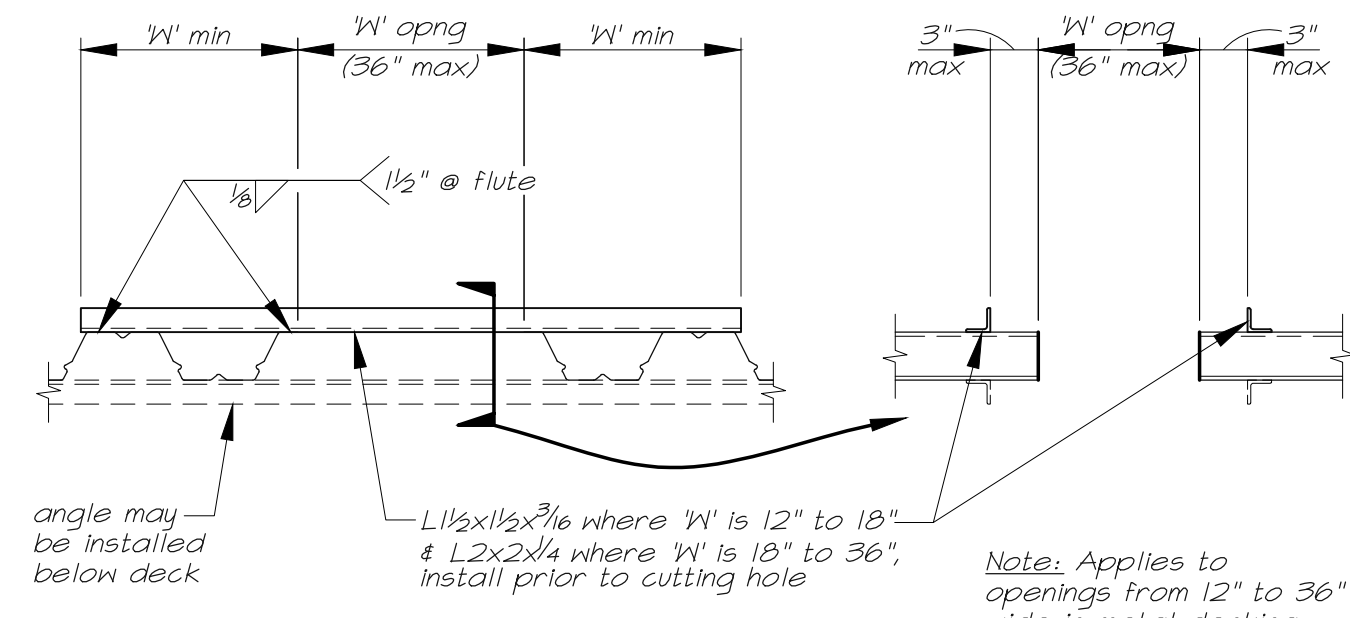
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S5.4

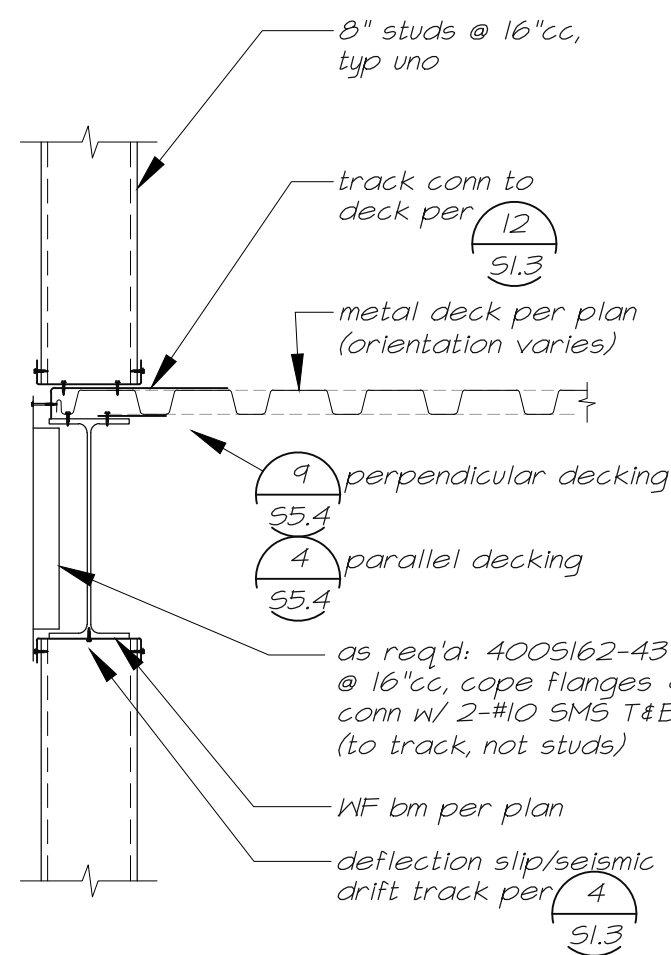




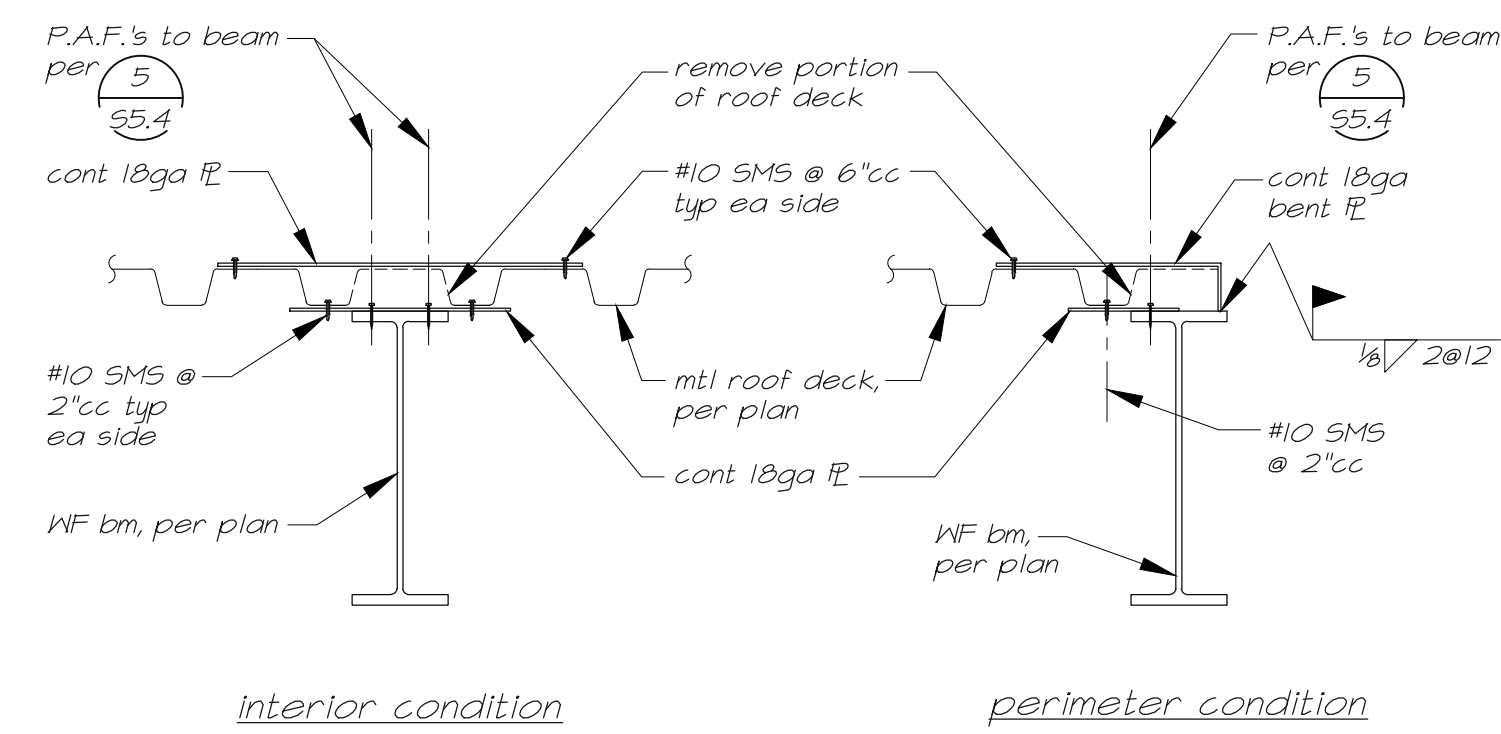
8 Opening in Metal Deck  
S5.5 1"=1'-0" w/o conc fill (openings from 6" to 12")



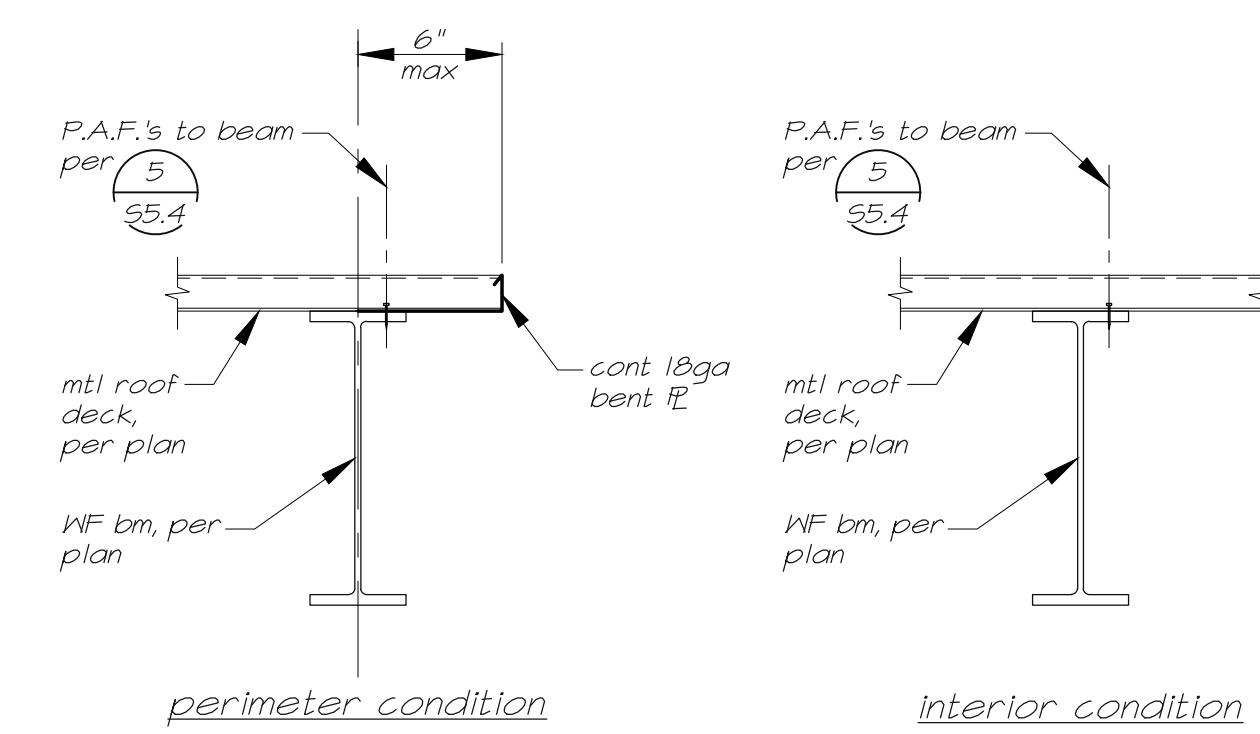
9 Opening in Metal Deck  
S5.5 1"=1'-0" w/o conc fill (opng from 12" to 36")



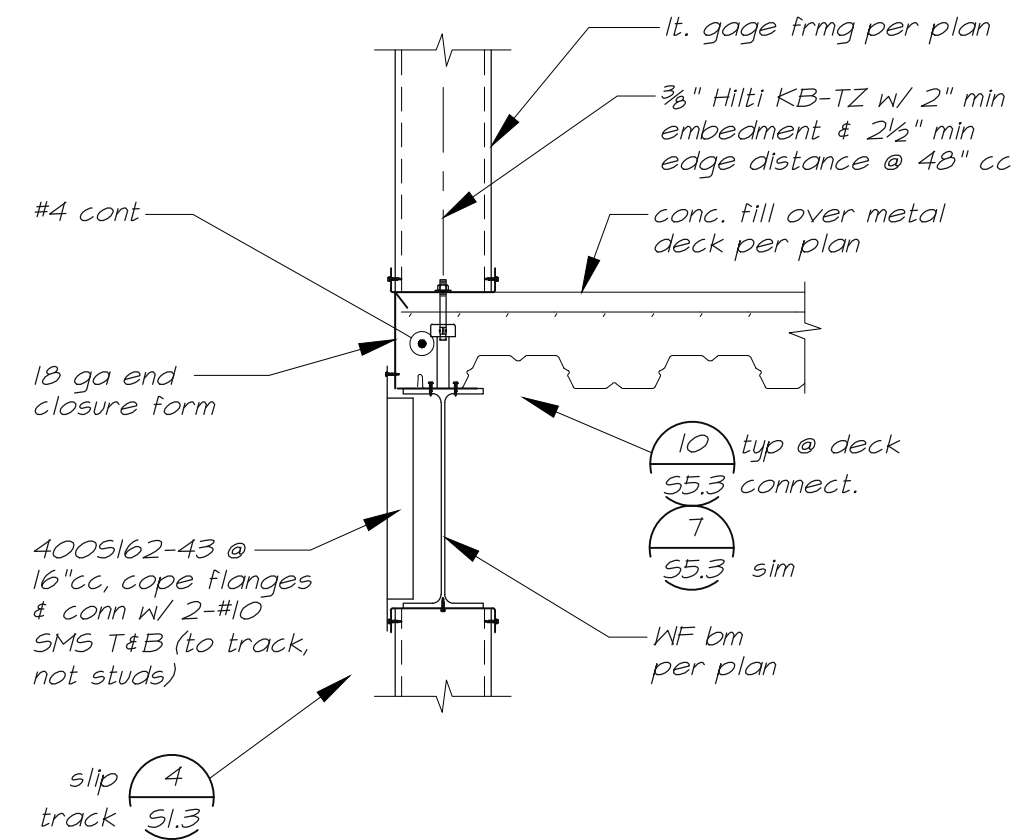
5 Detail  
S5.5 1"=1'-0" stud to structure at roof



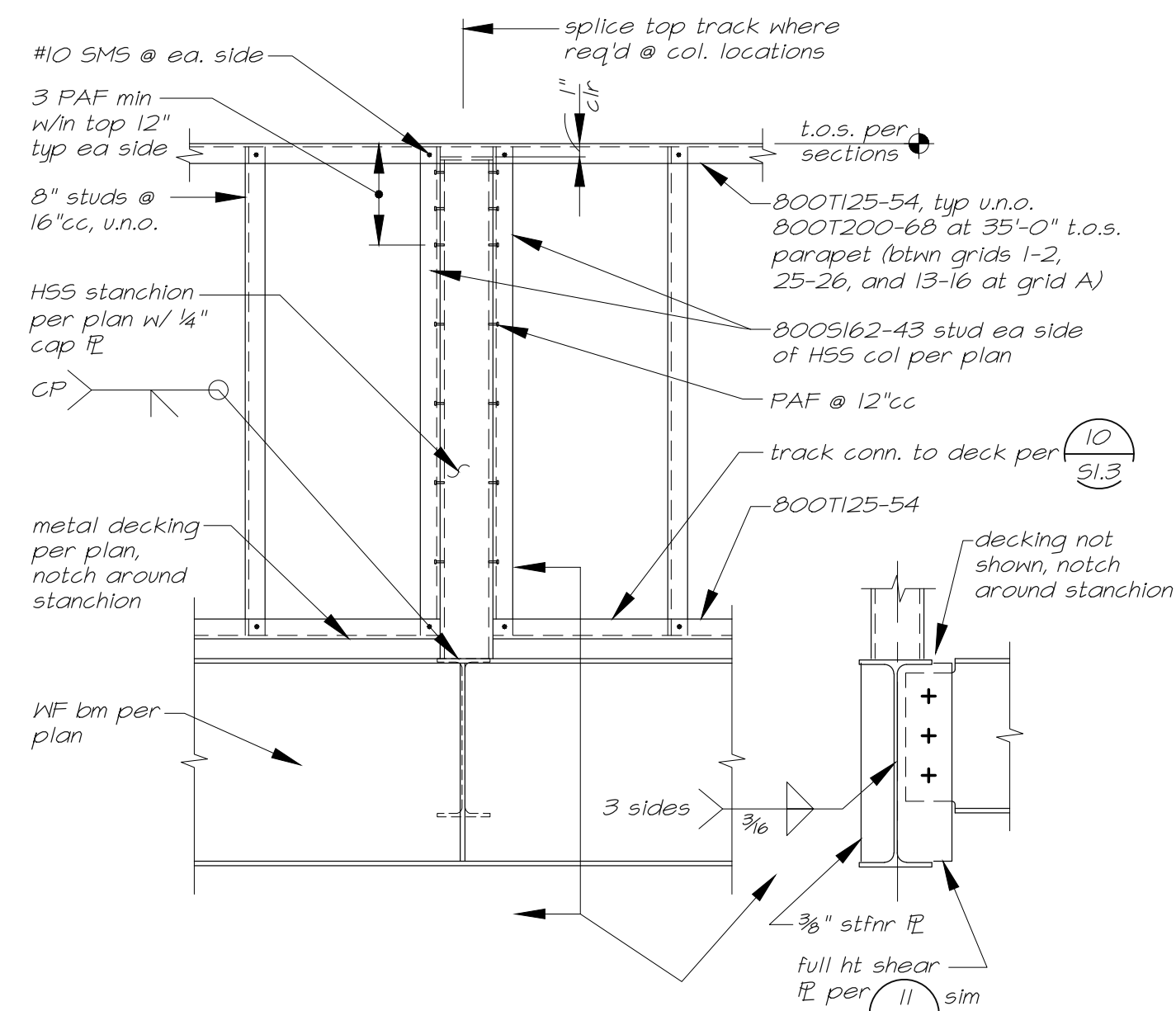
6 Detail  
S5.5 1/2"=1'-0" roof deck parallel to beam



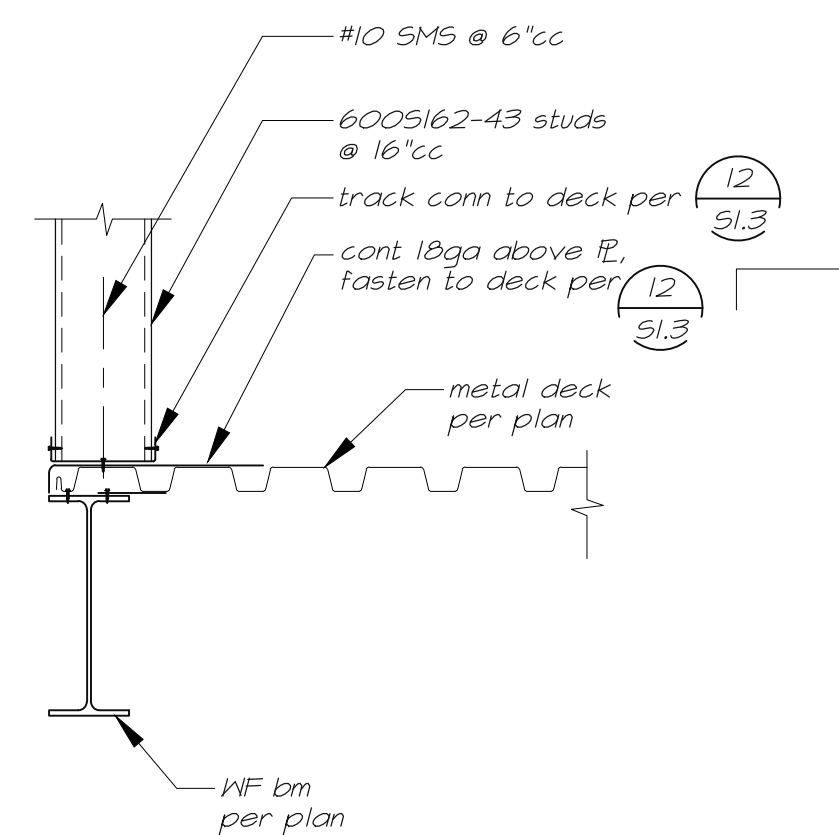
7 Detail  
S5.5 1/2"=1'-0" roof deck perpendicular to beam



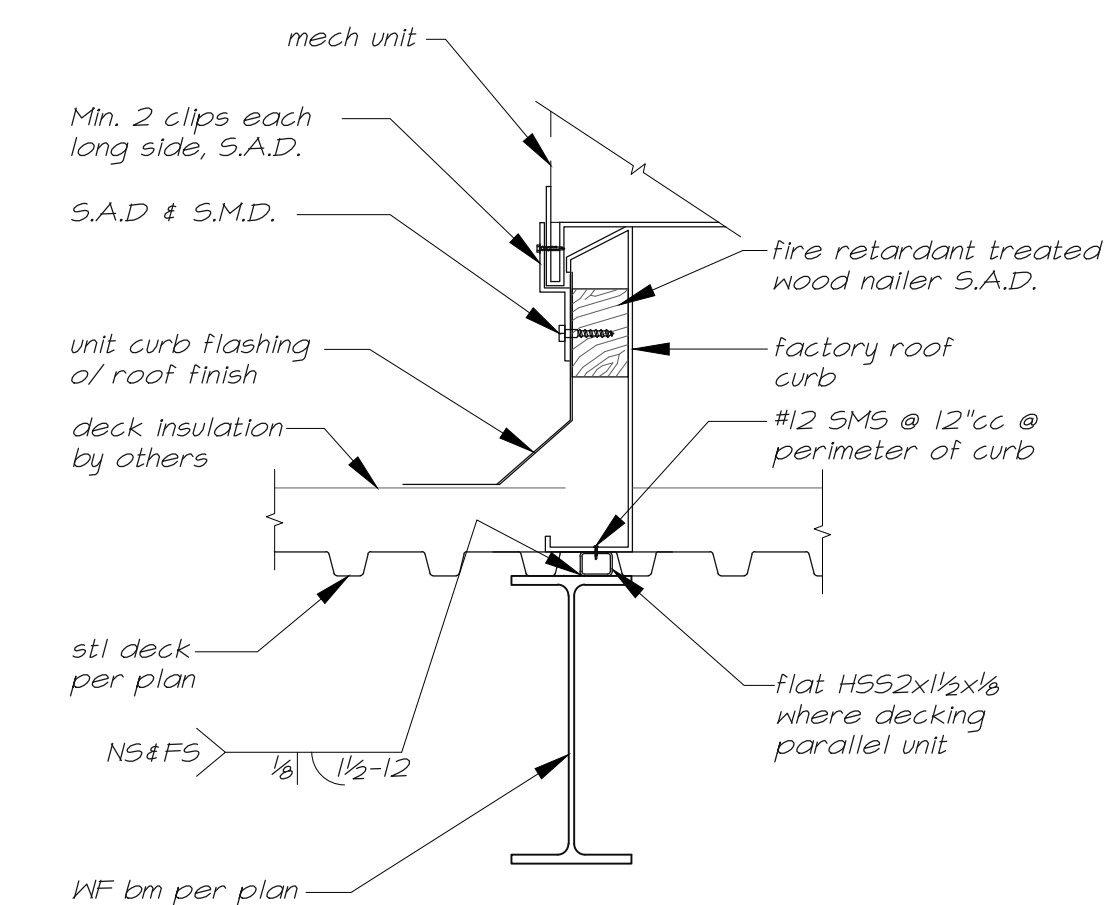
1 Detail  
S5.5 1"=1'-0" stud to structure @ floor



2 Detail  
S5.5 1"=1'-0" parapet stanchion



3 Detail  
S5.5 1"=1'-0"



4 Detail  
S5.5 1"=1'-0" mechanical curb



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Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architecturica.net  
www.architecturica.net

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2479 Sunrise Blvd.  
Gold River, CA 95670  
Tel: (916) 631-3030  
Fax: (916) 631-8996  
Web: www.bevier.net  
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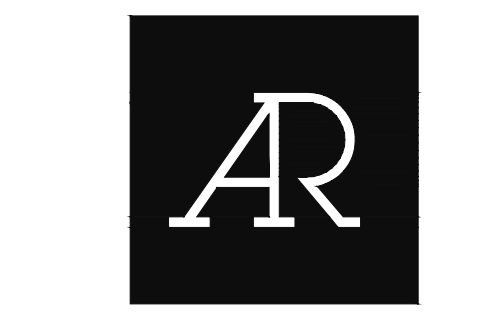
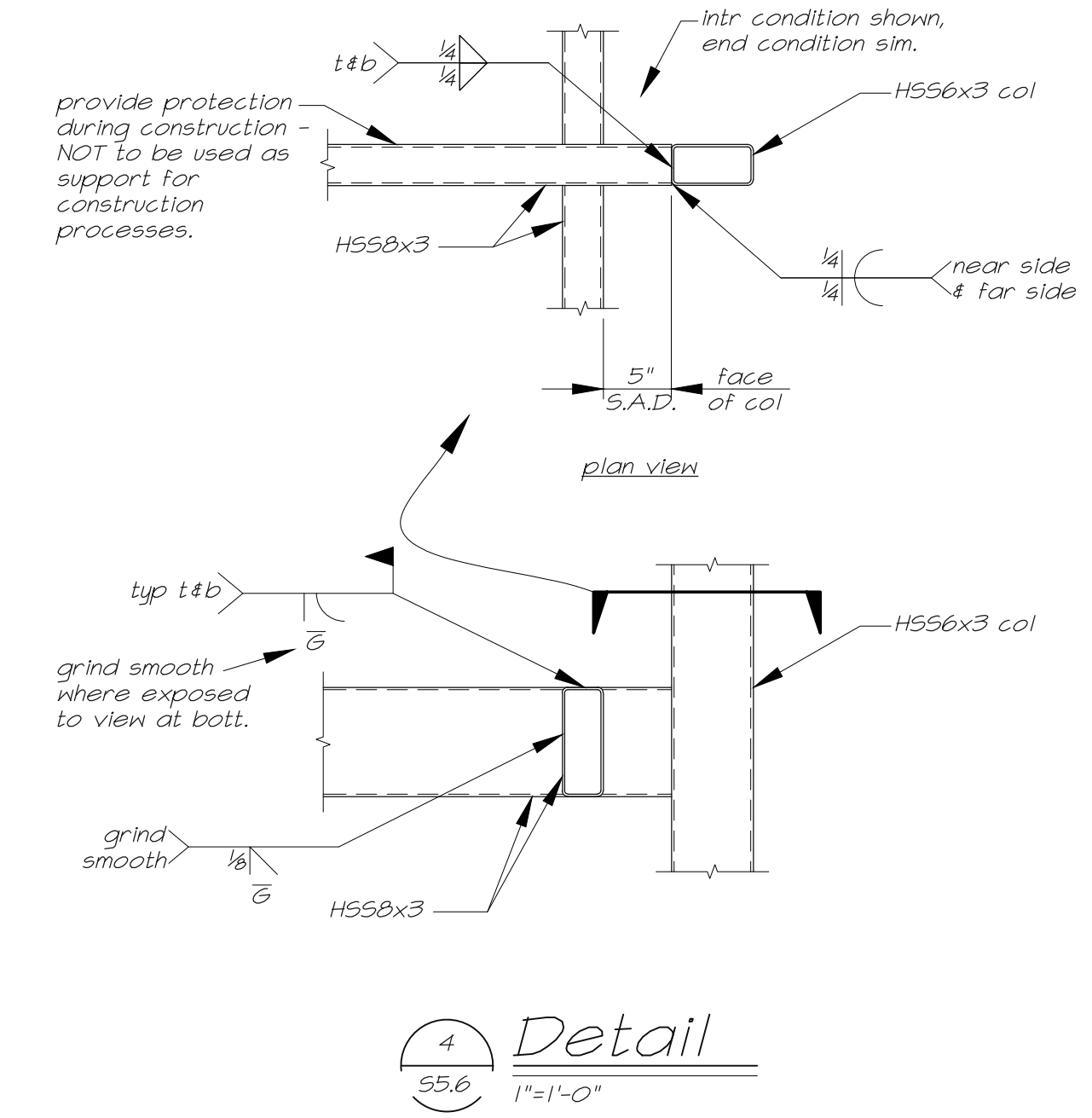
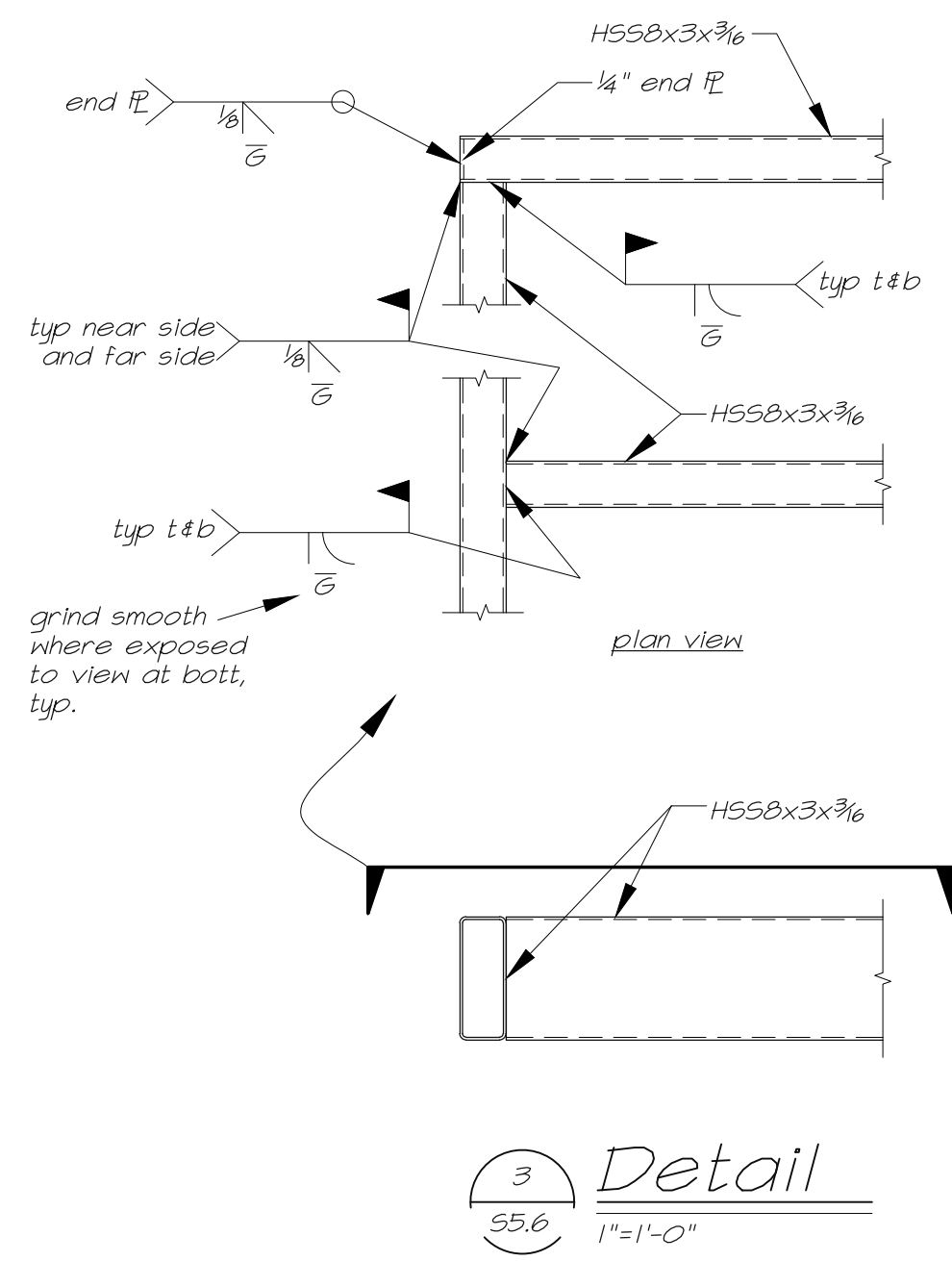
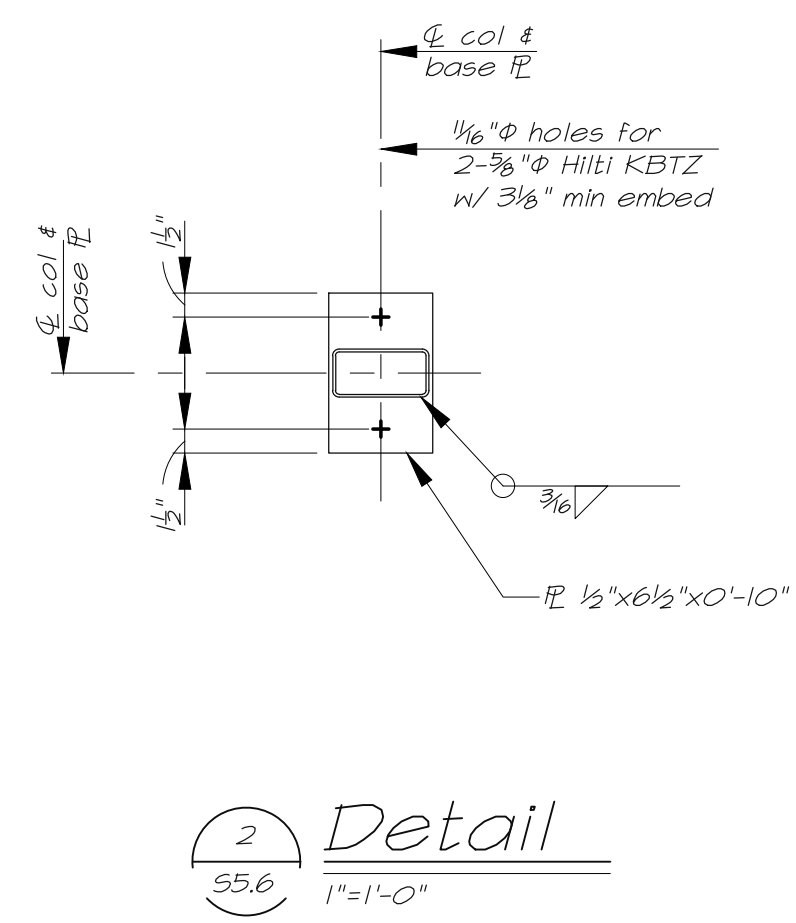
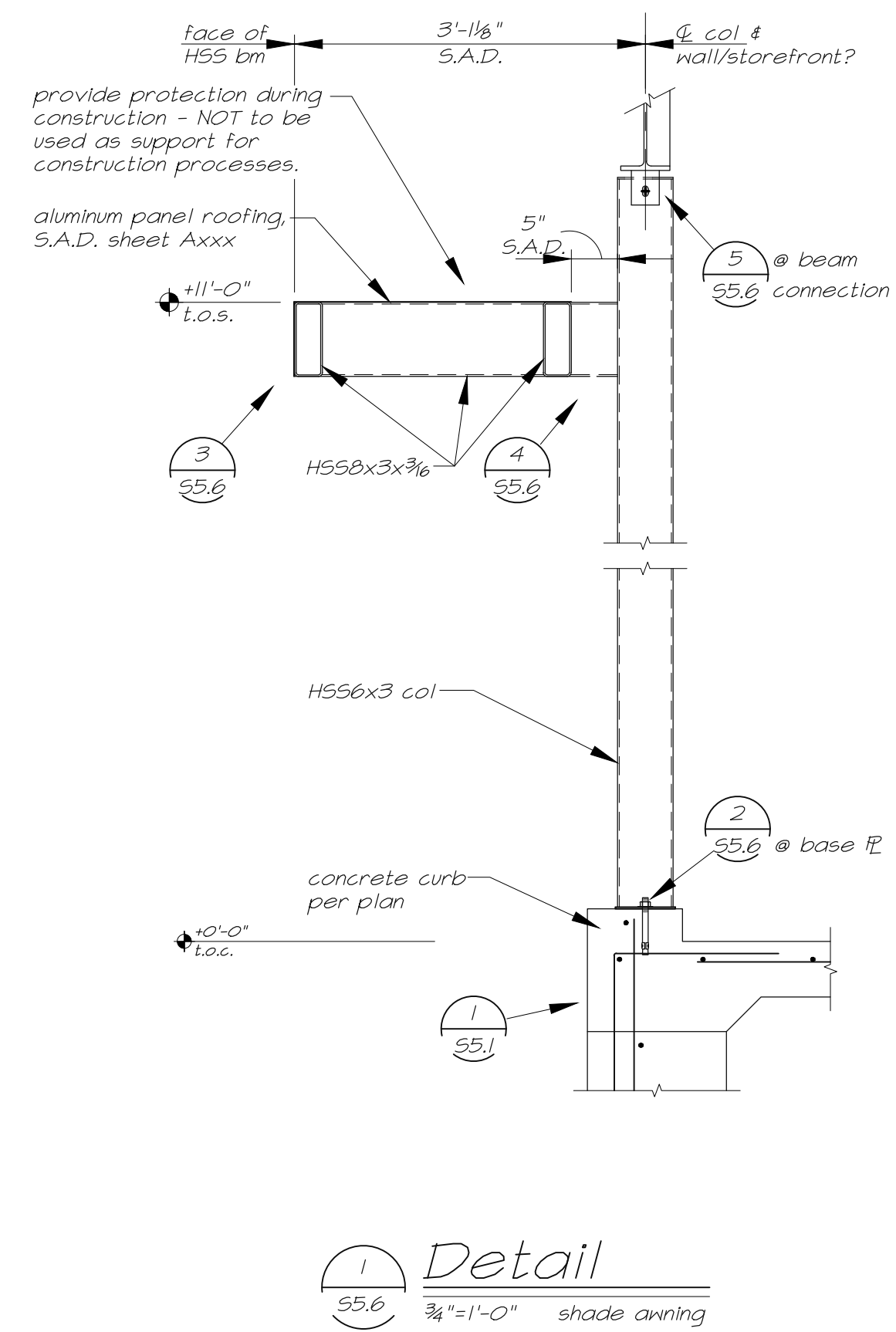
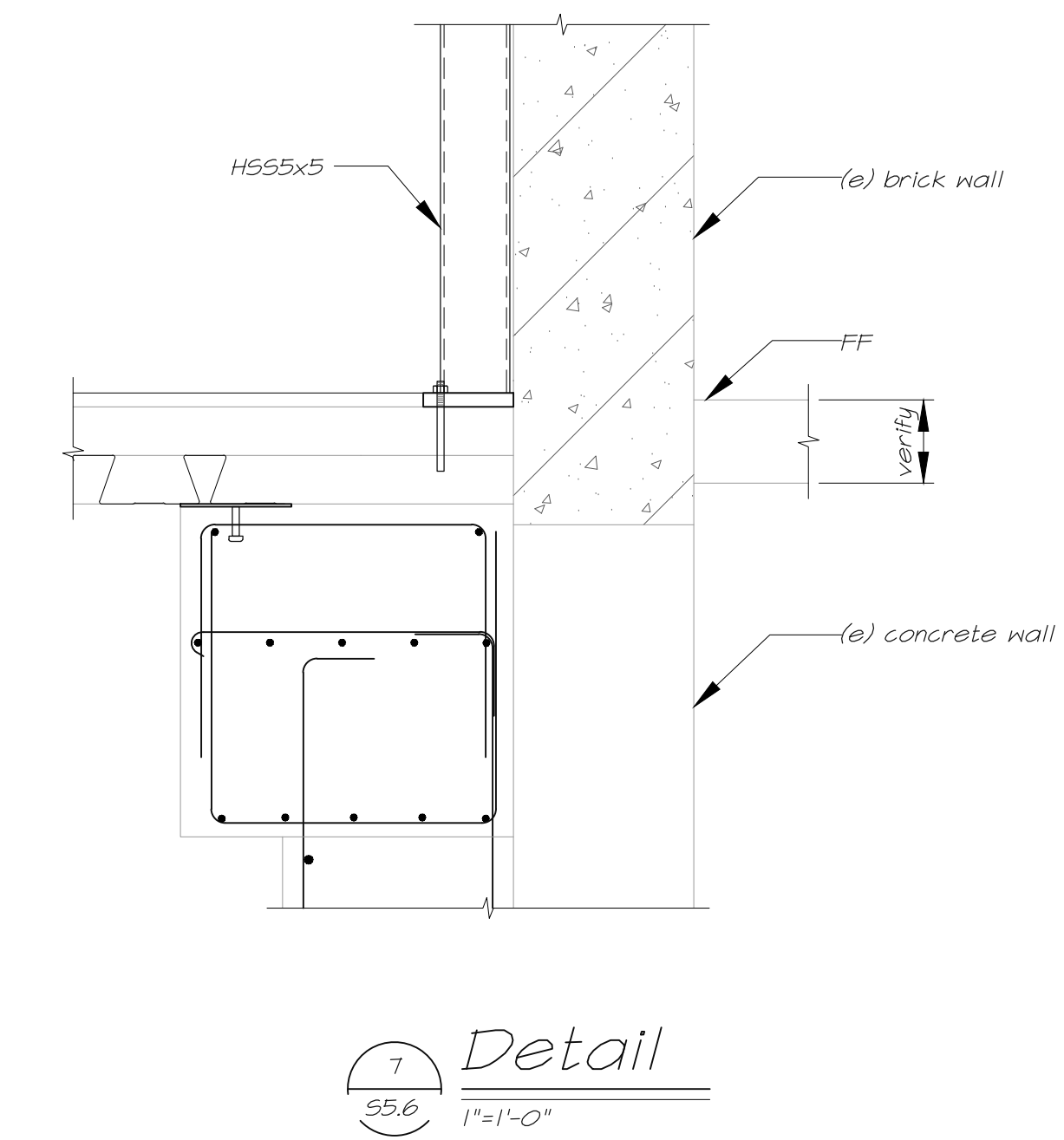
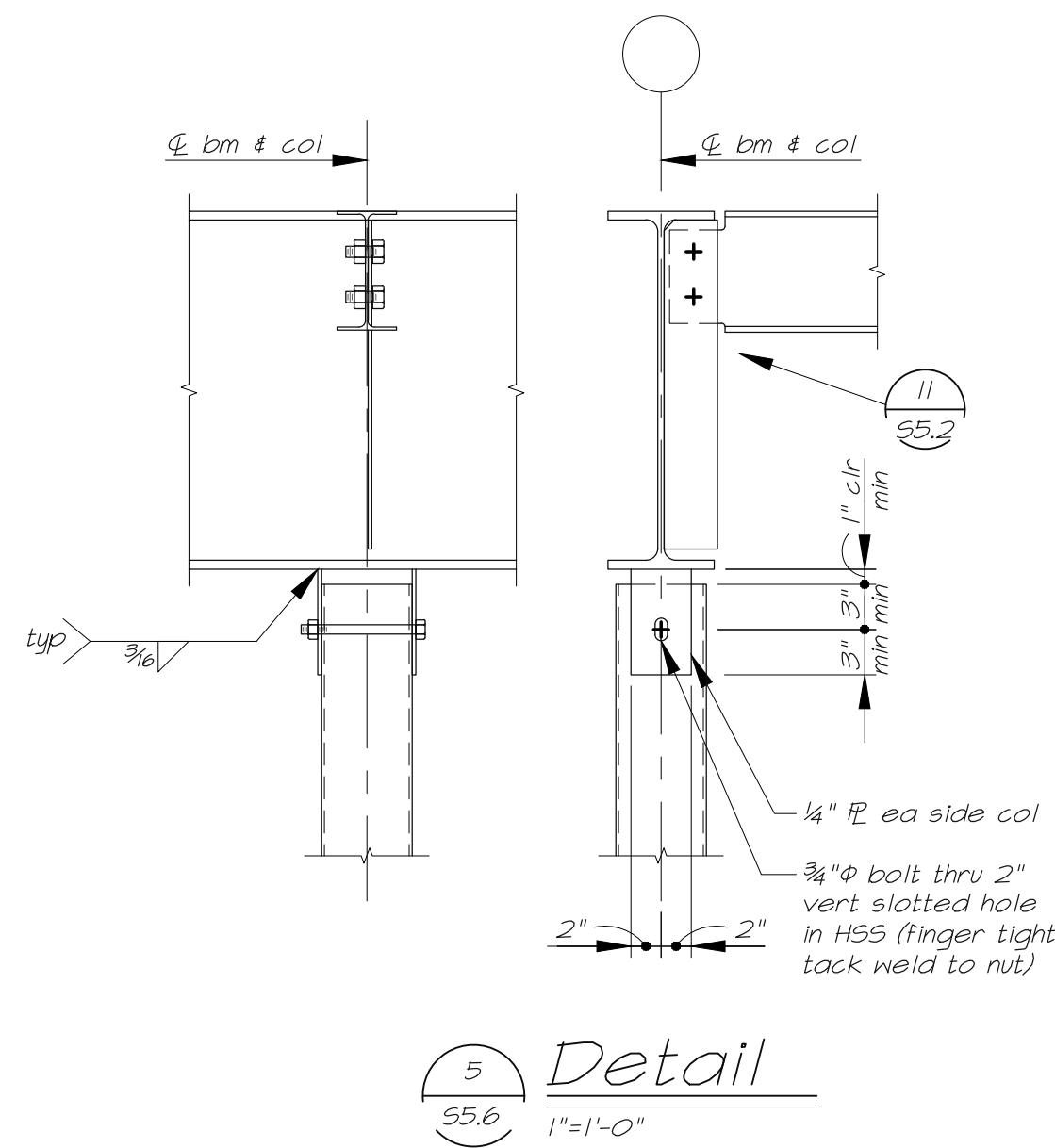
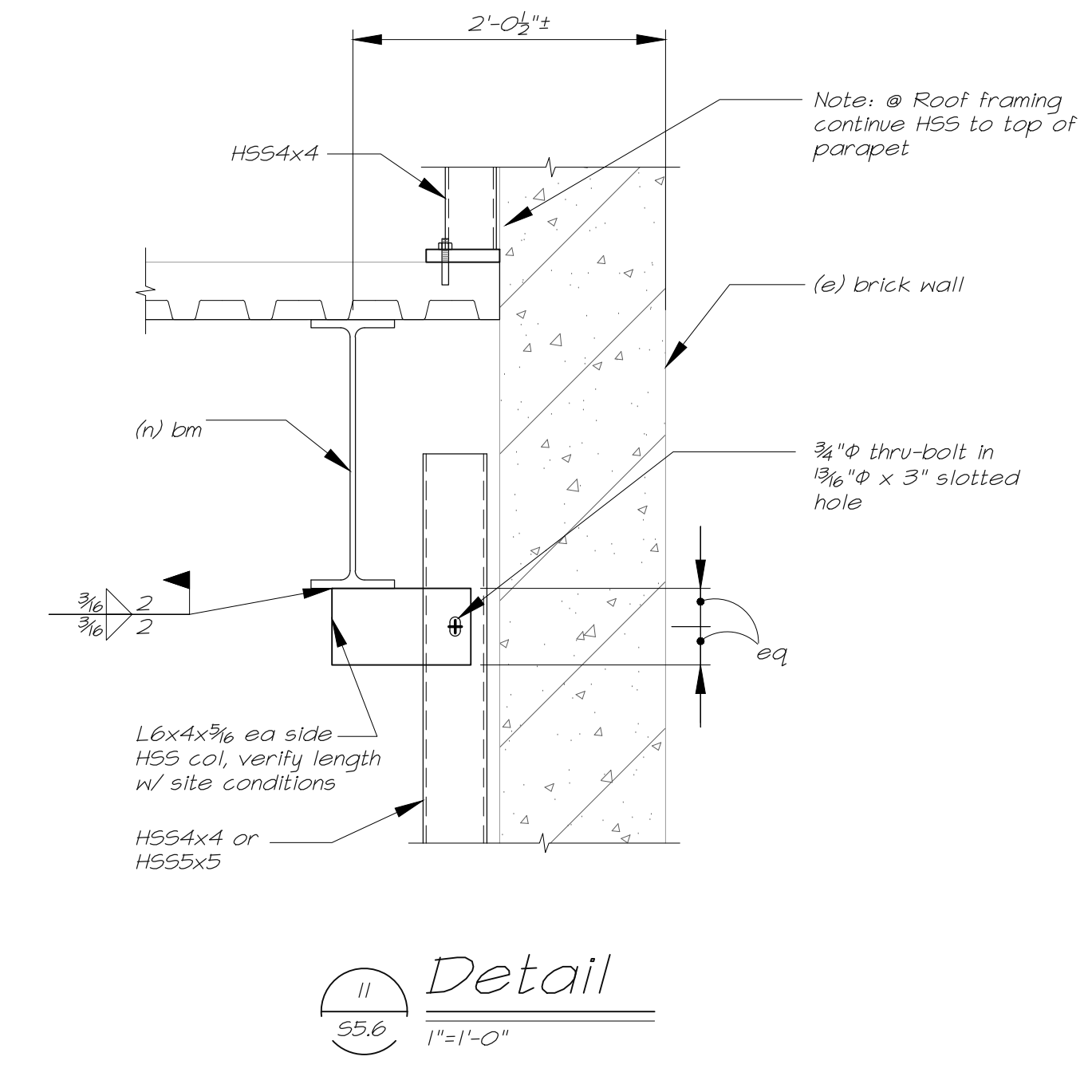
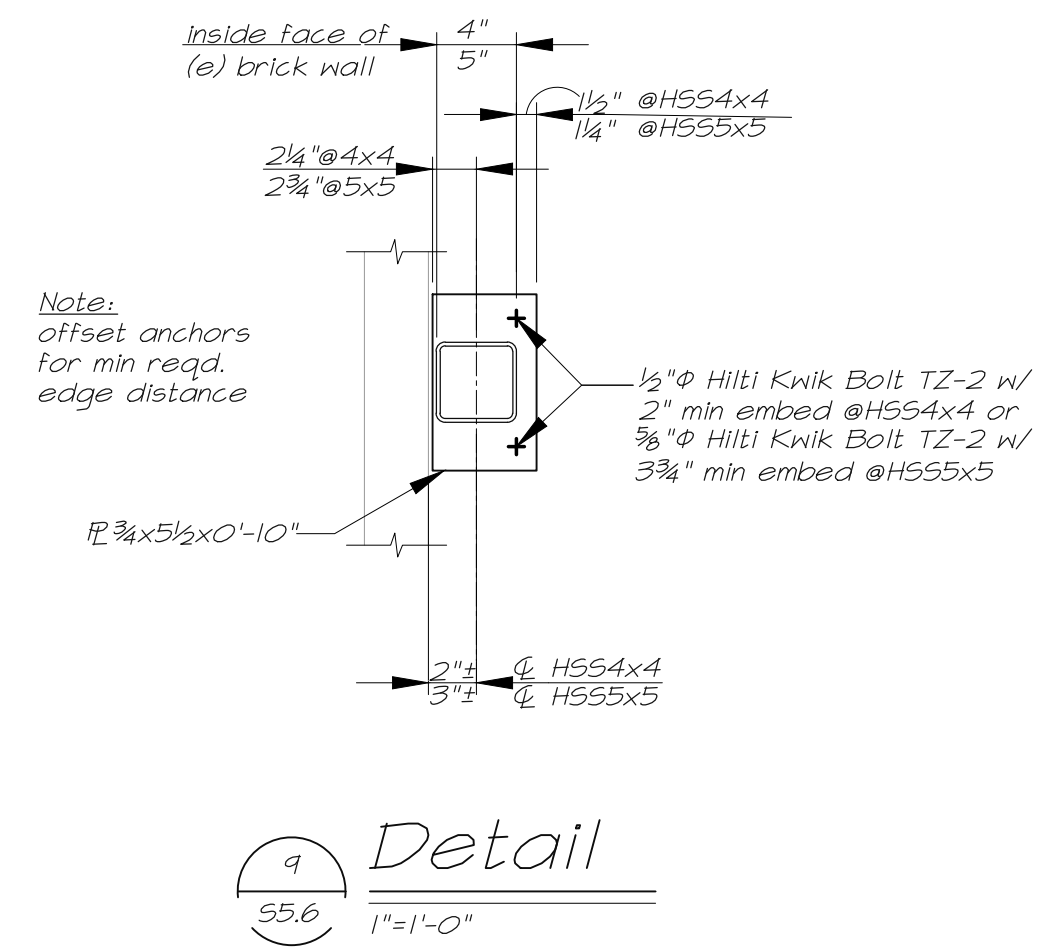
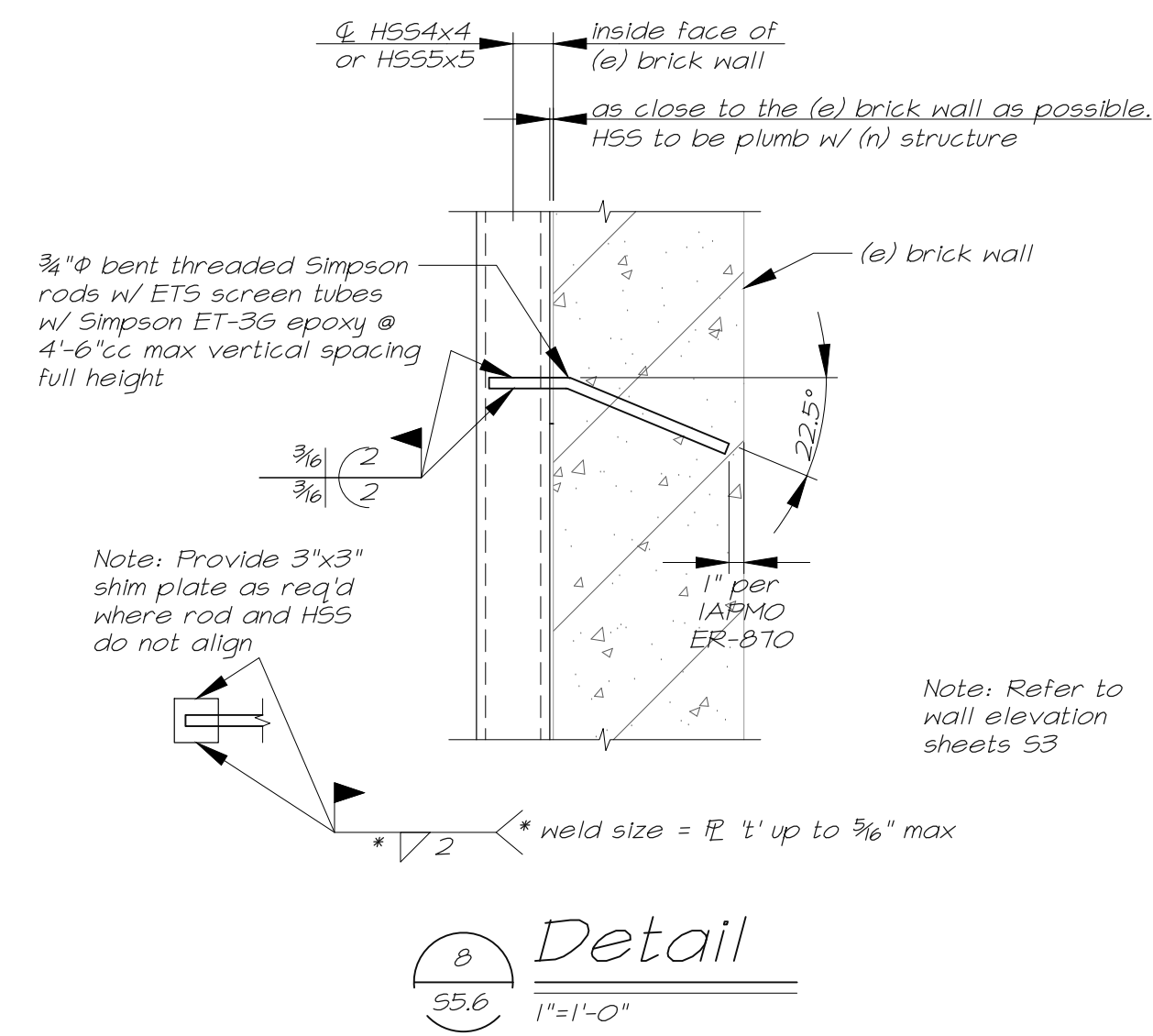
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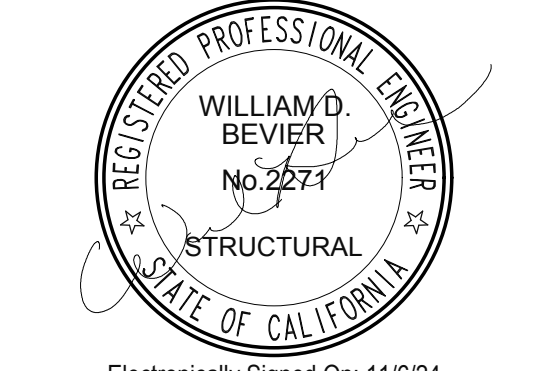
**S5.5**





555 West Benjamin Holt Drive, Suite 423  
Stockton, California 95207  
P: (209) 952-5850  
F: (209) 952-2442  
E: hello@architechnica.net  
www.architechnica.net

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Gold River, CA 95670  
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